

Memorandum



EXPERIENCE | Transportation

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Project Title:	STA-62-24.05	Date:	7-19-2018
Project No:	P403160049	Client Project No:	100824
Subject:	STA 62 Pavement Design Calculations		To: ODOT District 4
		From:	Walid Antonios, PE

Pavement Design Approach

Pavement calculations were performed for the US 62 mainline and the US 62 ramps (ramps A and B). It was found that the US 62 mainline has significant levels of truck traffic to warrant the use of superpave items. Ramps A and B were not found to require the use of superpave items, however, due to the short length of the ramps in comparison to the overall project area, it is recommended that the same pavement mixes be used for the ramps that will be used for the mainline. Pavement calculations were not performed for the side roads of the project, due to a lack of certified traffic information on these roads for the proposed project condition. For the stage I submittal, the mainline pavement buildup has been shown for the side roads. This pavement buildup is a place holder only, and the side road pavement design will be finalized in subsequent submissions after design assumptions can be discussed and agreed upon between TranSystems and the district. The CBR value used for all roads on the project is 9, per the geotechnical investigation.

Pavement Design - US 62 Mainline

CBR = 9 (from geotechnical investigation)

2025 ADT = 43,120

midyear traffic (2035) = $43,120 + (45,280 - 43,120)(.5) = 44,200$

2045 ADT = 45,280

24 hour truck % = 4%

principal arterial - urban

$M_r = 1200 \times 9 = 10,800 \text{ psi}$

Per 202-1:

D = 50%

Lane Factor = 80%

B:C = 2:1

ESAL for B trucks = 1.04

ESAL for C trucks = 0.41

reliability = 90%

overall standard deviation = 0.49

design serviceability loss = 2.0

B trucks: $(44,200)(0.04)(0.50)(0.80)(2/3)(1.04) = 490$

C trucks: $(44,200)(0.04)(0.50)(0.80)(1/3)(0.41) = 97$

Total = $490 + 97 = 587$

20 year design period ESAL: $(587)(365.25 \text{ day/yr})(20 \text{ yr}) = 4,288,035 \rightarrow 4.3 \times 10^6$

See pages 2 & 3 for graphical determinations $\rightarrow SN = 3.83$

opening day truck traffic: $(43,120)(0.04) = 1,725 \rightarrow$ greater than 1500, use item 442, 12.5 mm

442 AC surface course: min. 1.5" @ 0.43 = 0.65

442 AC int. course: min. 1.75" @ 0.43 = 0.75

302 AC base course: min. 4" @ 0.36 = 1.44

304 agg. base: 6" @ 0.14 = 0.84

$3.68 \rightarrow$ increase 302 layer to 6"
+ 0.72 $2" @ 0.36 = 0.72$

4.40 \checkmark greater than SN of 3.83

442 AC surface course: 1.5"

442 AC intermediate course: 1.75"

302 AC base course: 6"

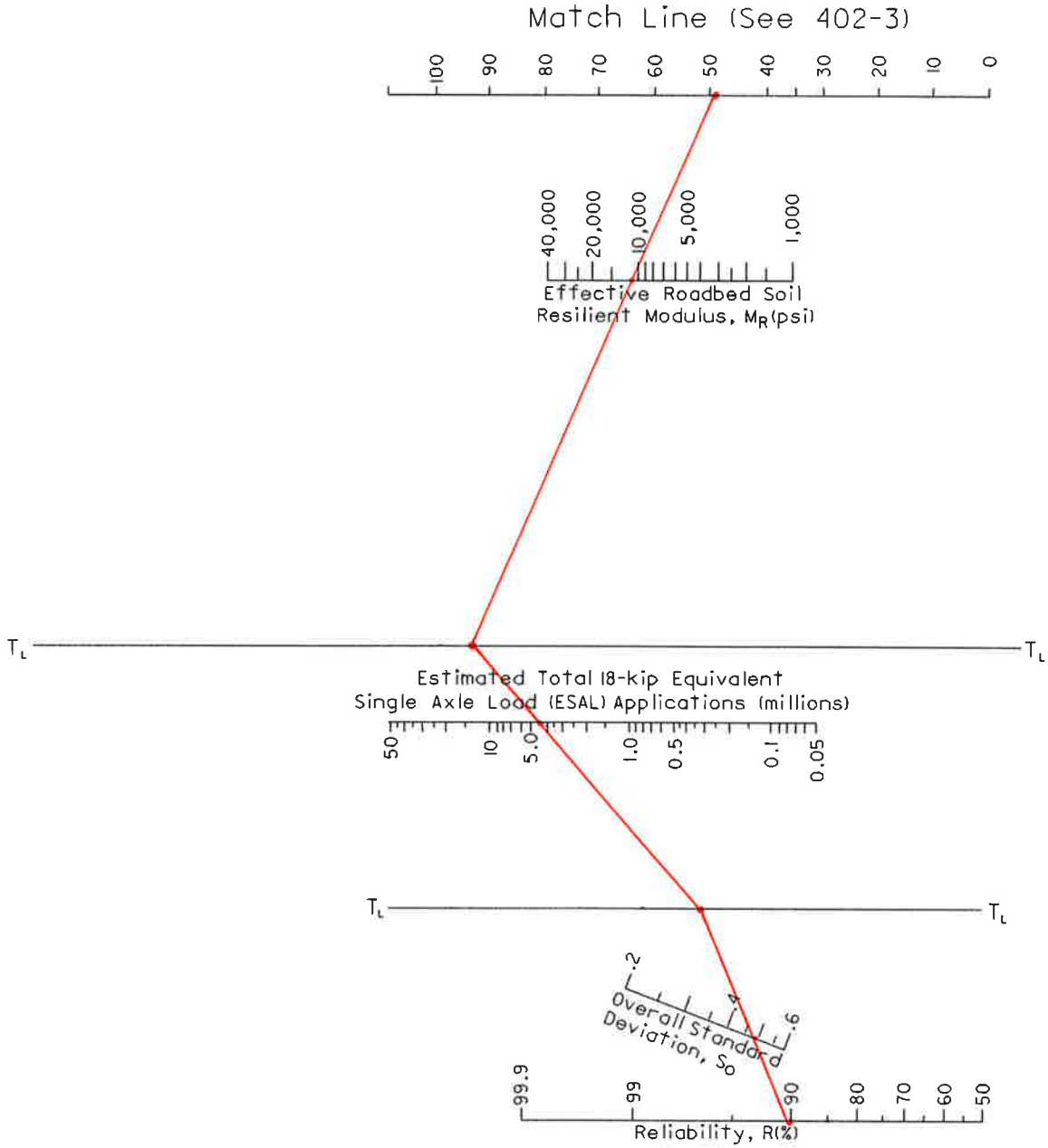
304 aggregate base: 6"

**Flexible Pavement Design Chart
Segment 1**

402-2

July 2008

**Reference Section & Figure
402, 402-1(step 3)**

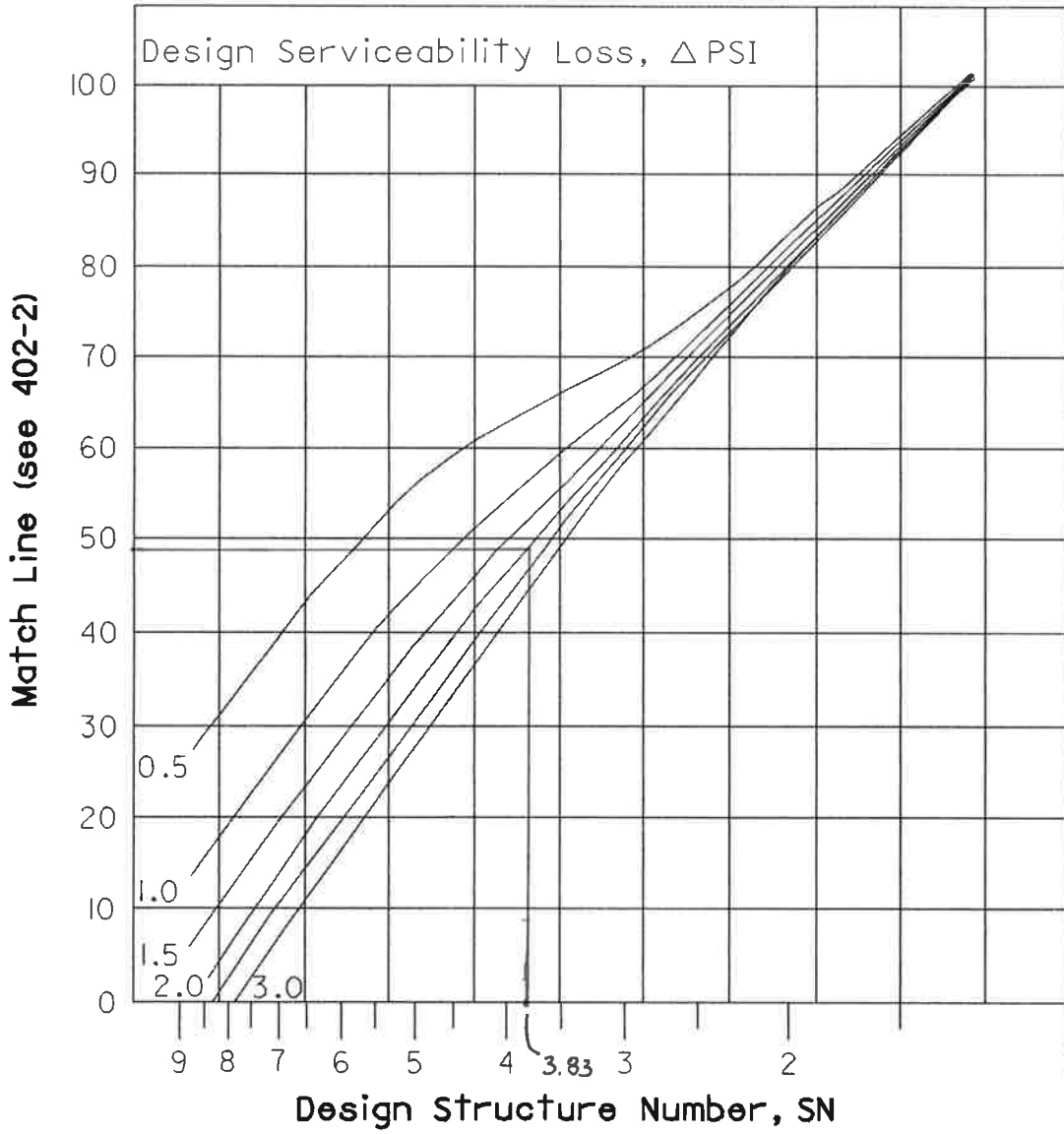


**Flexible Pavement Design Chart
Segment 2**

402-3

July 2008

Reference Section & Figure
402, 402-1(step 3)



US 62 Pavement Design - Ramps A & B

CBR = 9 (from geotechnical investigation)

2025 ADT (ramp A) = 5,240

Ramp A midyear traffic = $5,240 + (5,390 - 5,240)(.5) = 5,315$

2045 ADT (ramp A) = 5,390

2025 ADT (ramp B) = 1,260

Ramp B midyear traffic = $1,260 + (1,500 - 1,260)(.5) = 1,380$

2045 ADT (ramp B) = 1,500

24 hour truck % (A) = 0.09

24 hour truck % (B) = 0.02

$M_r = 1,200 \times 9 = 10,800$ psi

D = 100%

Lane factor = 100%

B:C = 2:1

ESAL for B trucks = 1.04

ESAL for C trucks = 0.41

reliability = 90%

overall standard deviation = 0.99

design serviceability loss = 2.0

Ramp A: B trucks = $5,315(0.09)(1)(1)(\frac{2}{3})(1.04) = 147.4 \rightarrow 147$

C trucks = $5,315(0.04)(1)(1)(\frac{1}{3})(0.41) = 29$

Total = $147 + 29 = 176$

Ramp B: B trucks = $1,380(0.02)(1)(1)(\frac{2}{3})(1.04) = 19.1 \rightarrow 19$

C trucks = $1,380(0.02)(1)(1)(\frac{1}{3})(0.41) = 4$

Total = $19 + 4 = 23$

ESAL (Ramp A): $(176)(365.25 \text{ day/yr})(20 \text{ yr}) = 1,285,680 = 1.3 \times 10^6$

ESAL (Ramp B): $(23)(365.25)(20) = 168,015 = 0.2 \times 10^6$

see pages 2 & 3 for graphical determinations \rightarrow SN(Ramp A) = 3.2
(4 & 5) SN(Ramp B) = 2.3

Ramp A opening day truck traffic: $5,240 \times 0.09 = 210$

less than 1,500 % no

Ramp B opening day truck traffic: $1,260 \times 0.02 = 25$

superpave required

Surface course: $441 @ 1" \rightarrow 0.43 \times 1 = 0.43$

@ 1.5" = 0.65

int. course: $441 @ 1" \rightarrow 0.43 \times 1 = 0.43$

@ 1.75" = 0.75

base course: $301 @ 3" \rightarrow 0.36 \times 3 = 1.08$

@ 4" = 1.44

agg. base: $6" \rightarrow 0.14 \times 6 = 0.84$

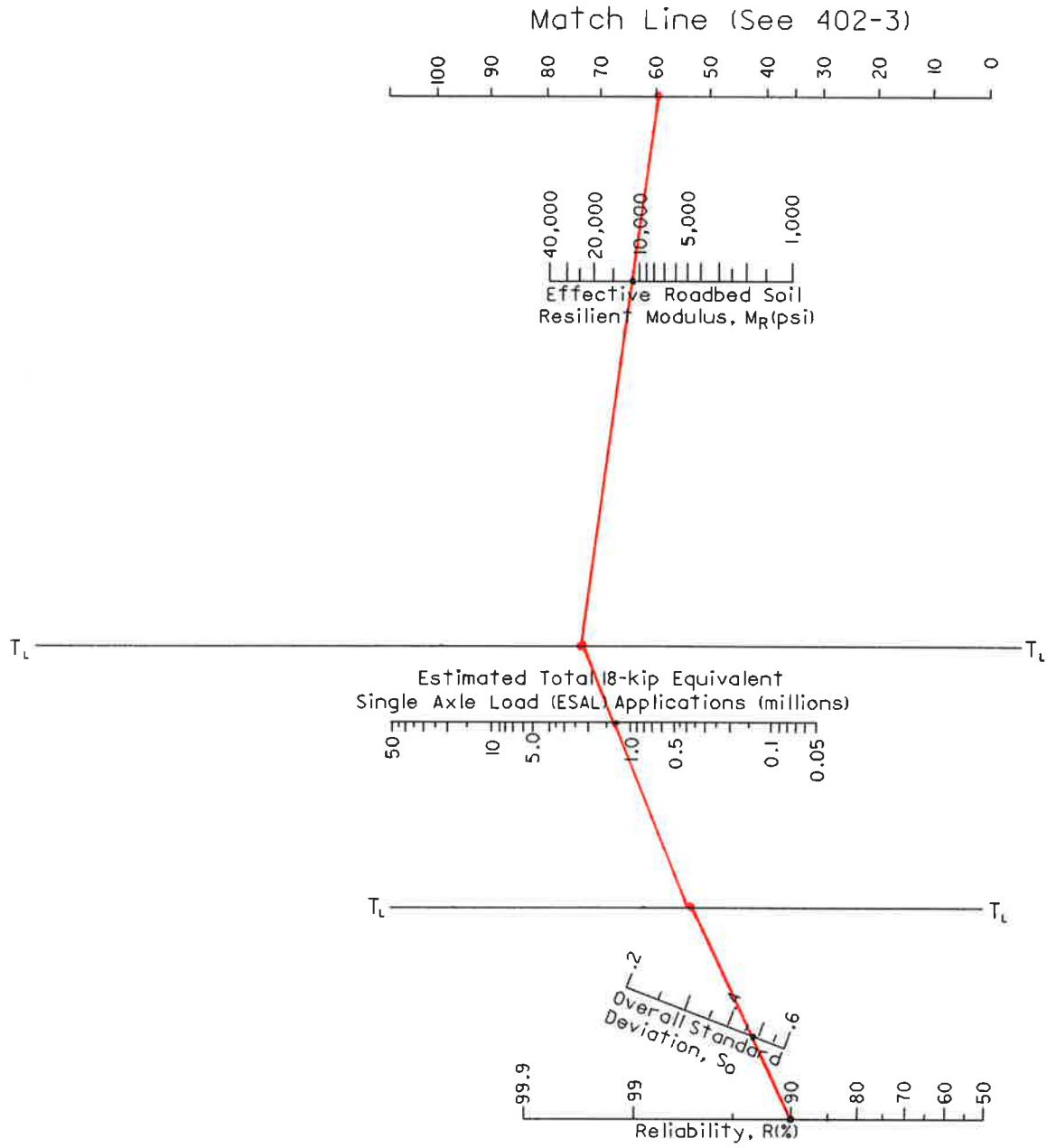
-----> 0.84

2.78

3.68

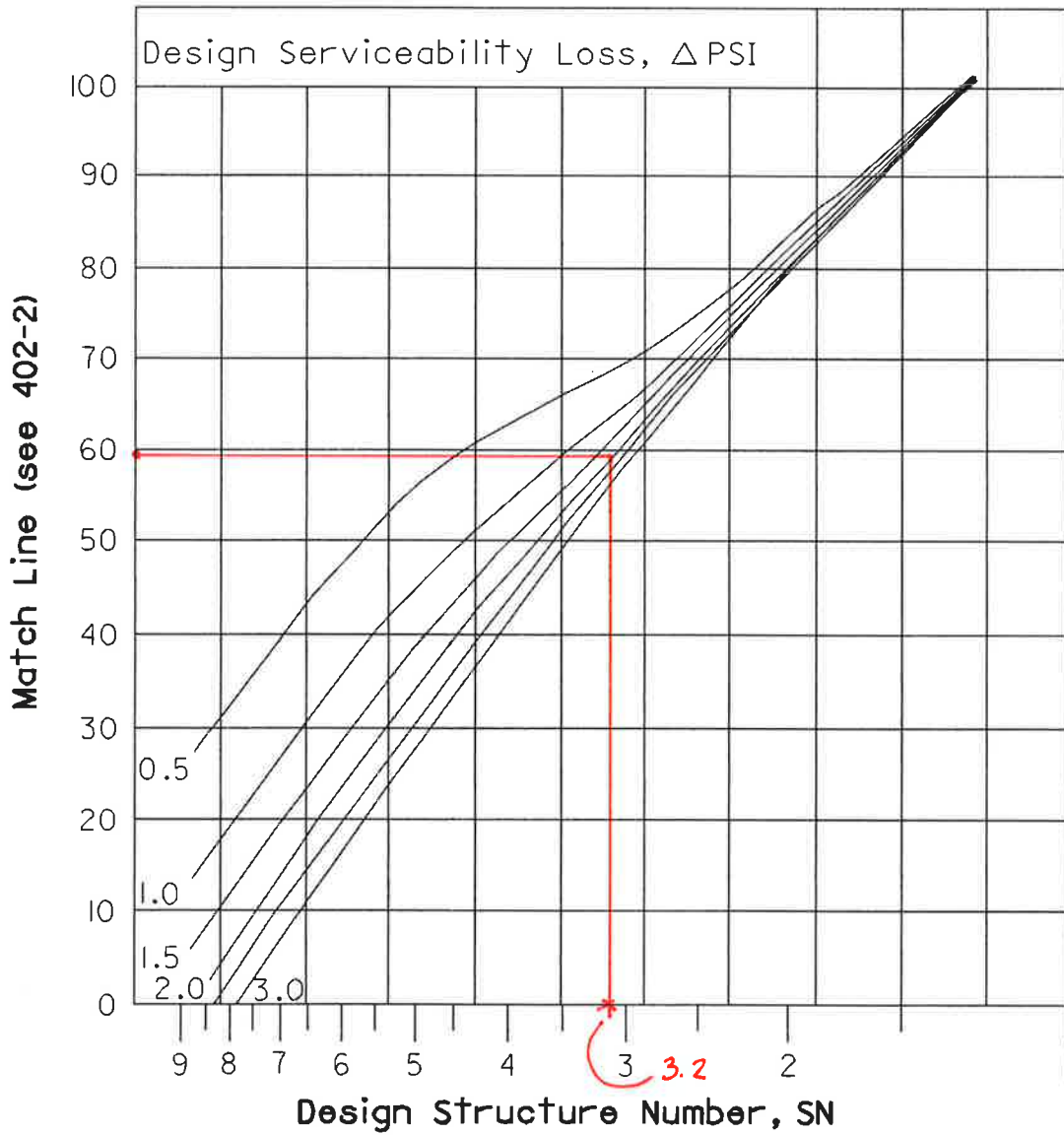
Though no superpave is required, due to short length of ramps compared with overall job length, pavement buildup for mainline will be recommended for use on Ramps A and B (442 items).

Flexible Pavement Design Chart Segment 1	402-2 July 2008
	Reference Section & Figure 402, 402-1(step 3)

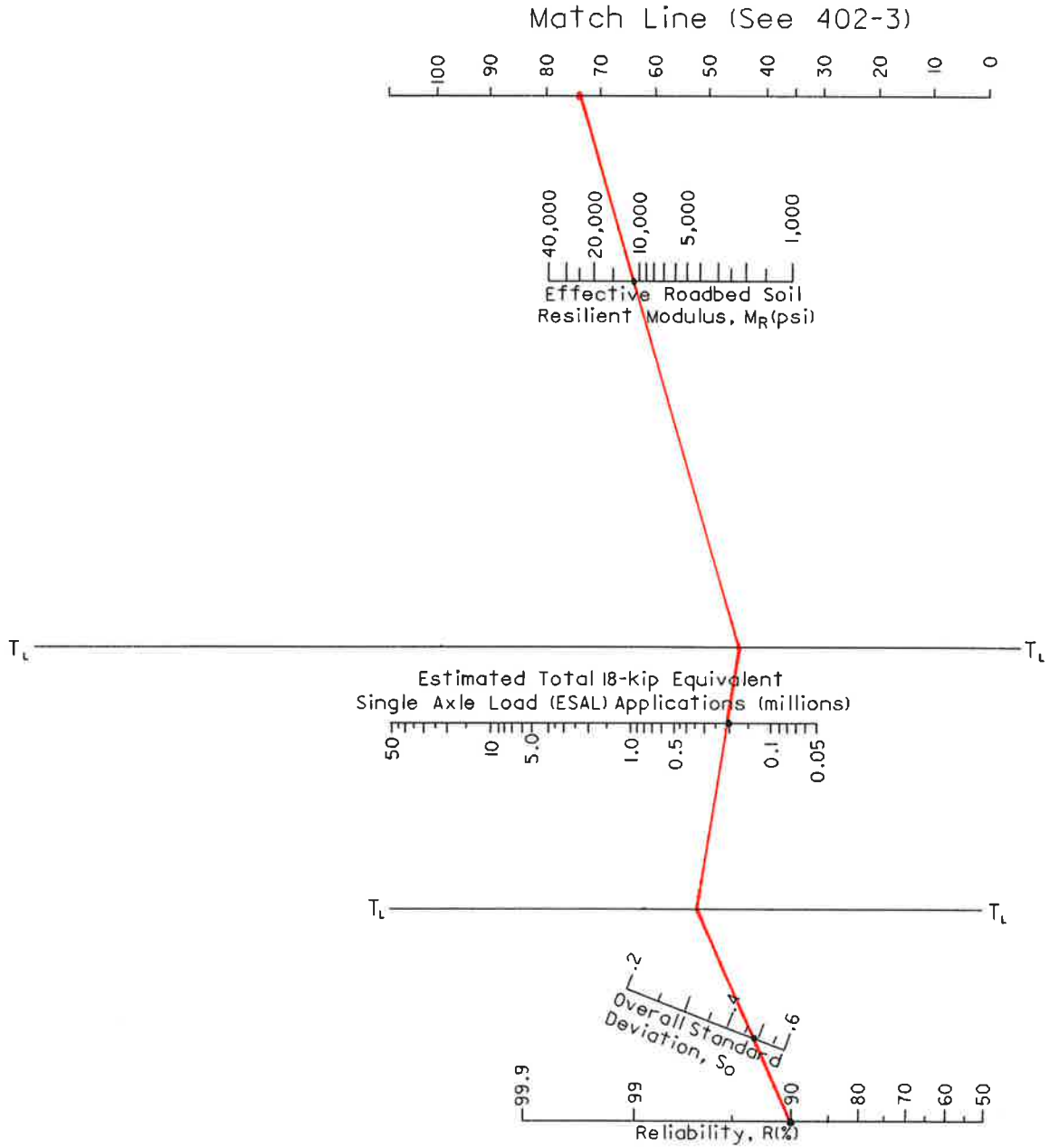


**Flexible Pavement Design Chart
Segment 2**

402-3
July 2008
Reference Section & Figure
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402-3

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