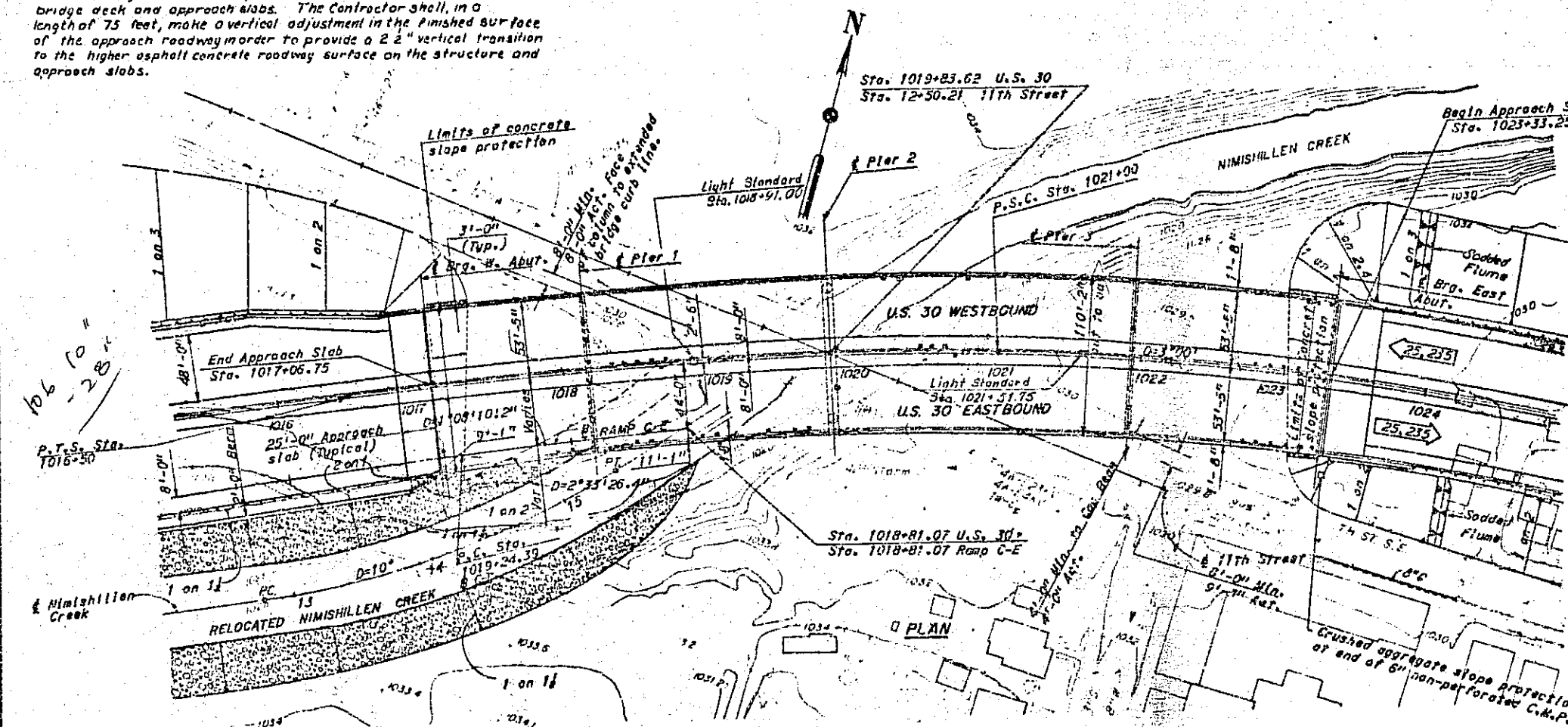


**STARK COUNTY
STA-30-15.66**

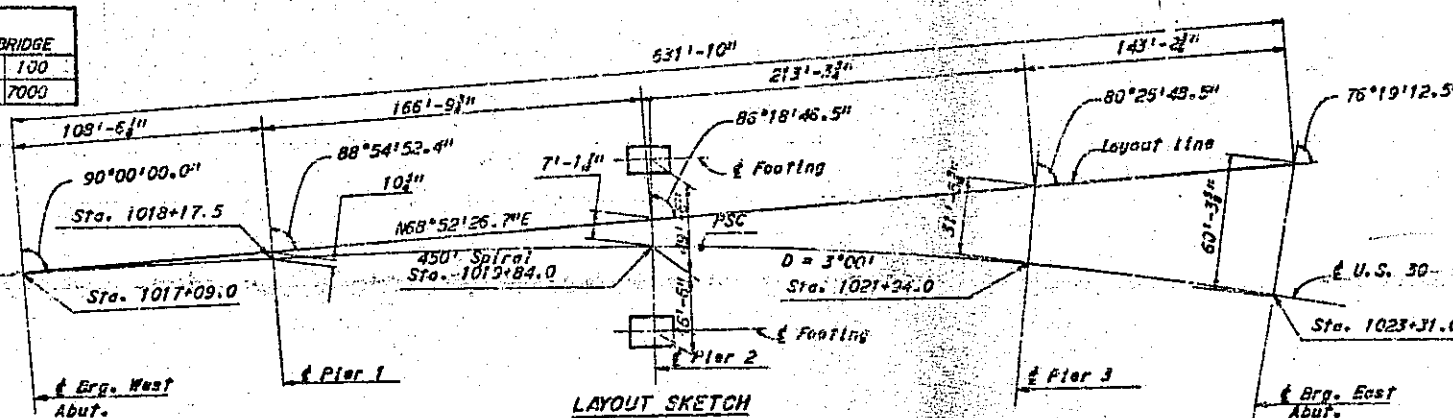
Note A:
Profile grade is at the top of Portland Cement Concrete on the bridge deck and approach slabs. The Contractor shall, in a length of 75 feet, make a vertical adjustment in the finished surface of the approach roadway in order to provide a 2" vertical transition to the higher asphalt concrete roadway surface on the structure and approach slabs.



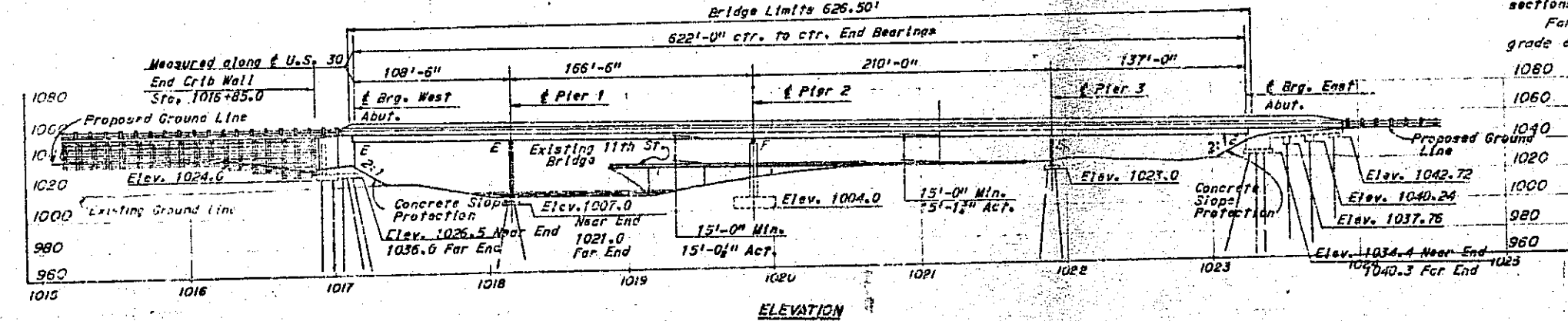
FLOOD DISCHARGE
EAST BRANCH NIMISHILLEN CREEK AT TUSCARAWAS STREET BRIDGE

Occurrence Interval (years)	5	10	20	50	100
Discharge Q (c.f.s.)	2600	3580	4450	5850	7000

High Water (1959) = 1023.5' ±
100 year High Water = 1025.5' ±
3500' Feet Upstream from the 11th St. Bridge.
Taken from Table 5, "Flood Discharge-Frequency Relationship", page 1A-23, "Flood Plain Study Main Report", Corps of Engineers, Feb. 1967.



PROFILE GRADE - RELOCATED NIMISHILLEN CREEK



ELEVATION

**EXISTING
UTILITY LEGEND**

4" G	Gas
W	Water
S	Storm
SS	Sanitary Sewer
T	Telephone
E	Power Lines
4W-4KV	4 Wire Primary
3W-E _s	3 Wire Secondary
0	Water Hydrant
0	Manhole
□	Catch Basin

U.S. 30 CURVE DATA

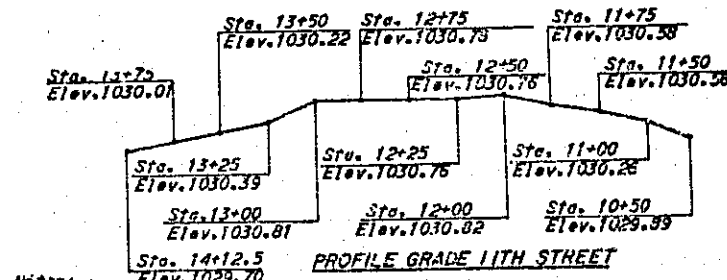
PTS = Sta. 1016+50.00
PSC = Sta. 1021+00.00
D _c = 3° 00' 00.0"
Δ = 22° 48' 22.0"
A _c = 9° 18' 22.0"
R _c = 1909.859'
T _c = 610.988'
L _c = 155.444'
L _c = 450.00'
L _c = 310.204'

RAMP C E CURVE DATA

PI = Sta. 1018+03.735
D _c = 1° 08' 10.2"
Δ = 1° 46' 48.2"
R = 5042.908'
T = 78.342'
L = 156.672'

SOUTH GUTTERLINE CURVE DATA

PI = Sta. 1018+10.21 @ C-E
D _c = 2° 33' 26.4"
Δ = 3° 35' 55.4"
R = 2240.445'
T = 70.384'
L = 140.722'



Notes:
The following items are not included in the bridge plans. See roadway plans for details.
1. Relocation or removal of existing utilities.
2. Approach grading, pavements and slabs.
3. Guardrails.
Denotes one way 1987 average daily traffic.

Foundation design and foundation quantities are based on a study of standard penetration test borings made at the site.
Denotes points of minimum vertical clearance.
All piles shall be 12" cast-in-place concrete with an estimated average pile length of 80' for the Abutments, 74' for Pier 1 and 68' for Pier 3. These estimates are based on boring data and are approximate only. The contractor shall assume full responsibility for the length of pile selected for driving.
Earthwork limits shown are schematic. Actual slopes shall conform to plan cross-sections.
For estimated quantities and profile grade elevations see Sheet E1.

Contractor shall use adequate precautions in constructing Pier 2 footings to avoid any damage to the existing 11th Street Bridge.

EXISTING 11TH STREET STRUCTURE

Type: Continuous Rolled Beam with reinforced concrete slab.
Spans: 2 Spans of 49'-7" = 99'-3" ctr. to ctr. End Bearings.
Roadway: 40'-0" face to face of curbs plus two 6'-1" sidewalks and rolls = 52'-2" out to out of structure.
Skew: 35°
Wearing Surface: 3" Fiber brick wearing course with 1" oil to sand bedding course.
Alignment: Tangent
Foundations: Spread Footing on gravel.

PROPOSED STRUCTURE

Type: Continuous Welded Girder with reinforced concrete slab.
Spans: 108'-6", 166'-6", 210'-0", 137'-0" = 622'-6" ctr. to ctr. of and bearings along U.S. 30.
Roadway: Various face to face of Parapets including 2'-6" median barrier.
Loading: HS20-44, Class I and the Alternate Military Loading
Skew: Radial to Baseline
Wearing Surface: 2 1/2" Asphalt Concrete
Approach Slab: 25'-0" (AS-1-72) Modified
Alignment: 450' Spiral to 3° Curve Right on U.S. 30.

Modify Std. Dwg. AS-1-72 by providing 3" clear over the top rebars instead of the 4" shown on the Std. Dwg. The jacking holes as called for on Std. Dwg. AS-1-72 are not required. Revised: 2-29-79 by RAS

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SITE PLAN

U.S. 30 OVER 11TH STREET	
P.R. NO. STA-30-15.95	STA. 1017+06.75
SCALE: 1"=50'-0"	STA. 1023+33.25
RELOCATED U.S. 30	
CANTON	STARK COUNTY OHIO
DRAWN BY: TRACED	CHECKED BY: REVIEWED BY:
DATE: 1-10-79	DATE: 1-10-79

PROFILE GRADE U.S. 30
(See Note A)

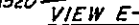
STA-30-15.95



PLAN



SECTION D

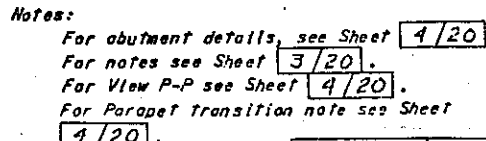


VIEW E-

...ing



VIEW A-A



H.N.T.B. BRIDGE NO. 18	Revised 2-23-79 By RAS
HOWARD, NEEDLES, TAMMEN & BERGENDOFF	2/20

CONSULTING ENGINEERS
KANSAS CITY CLEVELAND NEW YORK

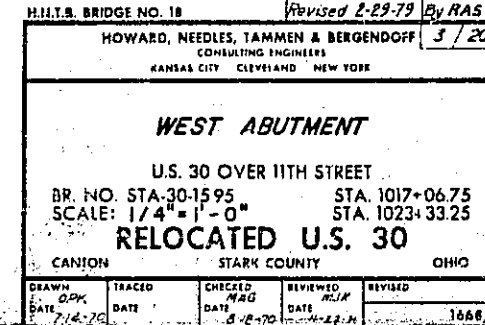
WEST ABUTMENT

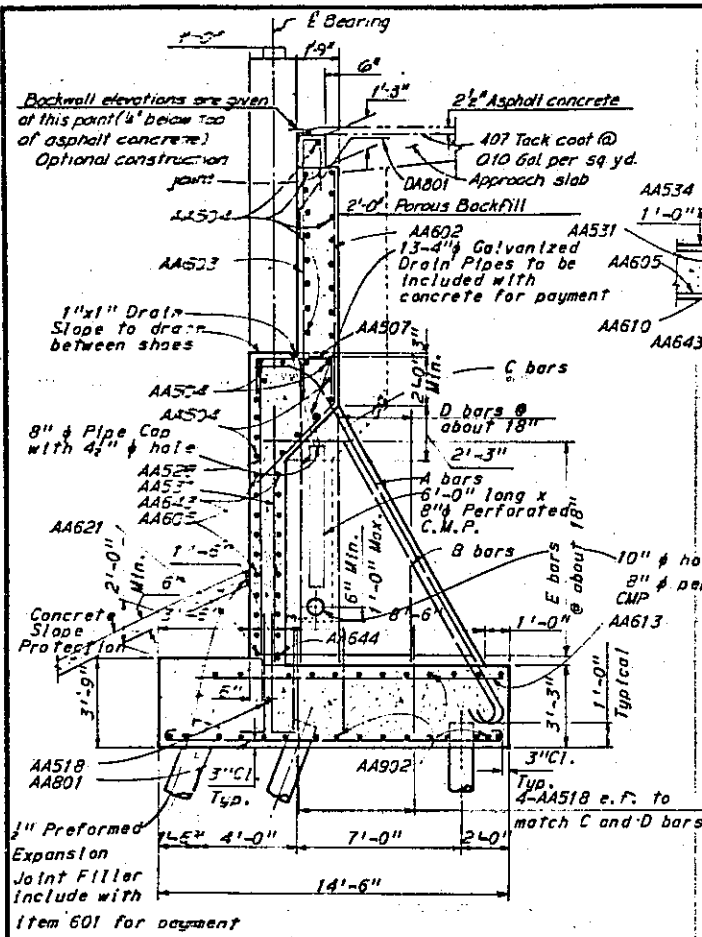
U.S. 30 OVER 11TH STREET
BR. NO. STA. 30-1575 STA. 1017+06.75

SCALE: 1/4" = 1'-0" STA. 1023+33.25
RELOCATED U.S. 30

CANTON		STARK COUNTY		OHIO
DRAWN J.C.	TRACED	CHECKED J.C.	REVIEWED J.C.	INDEXED

DATE 7-2-70	DATE	DATE 7-2-70	DATE 2-22-71	DATE
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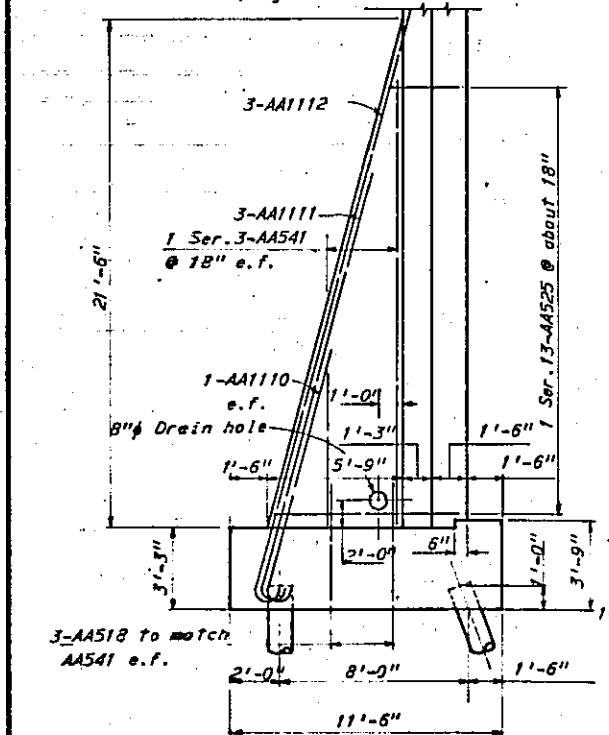




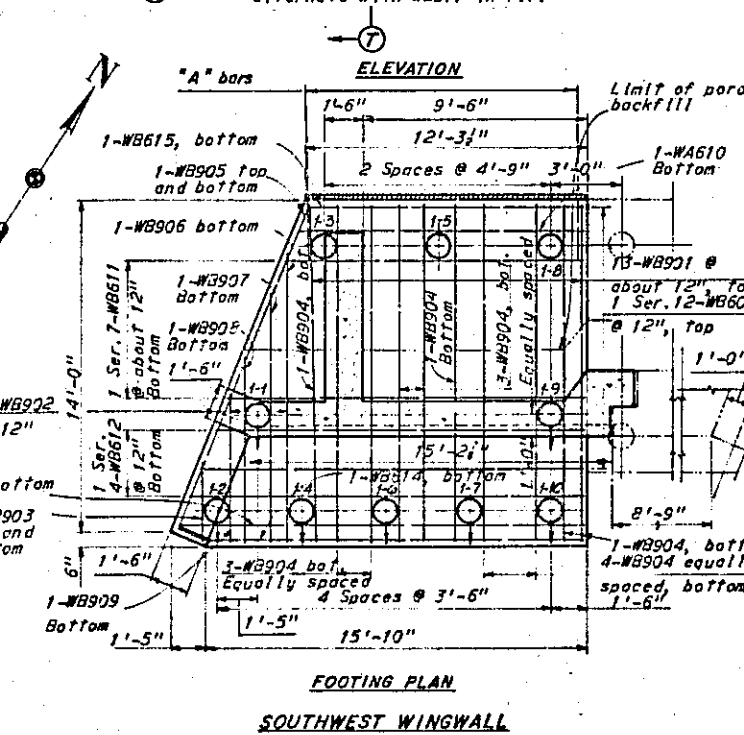
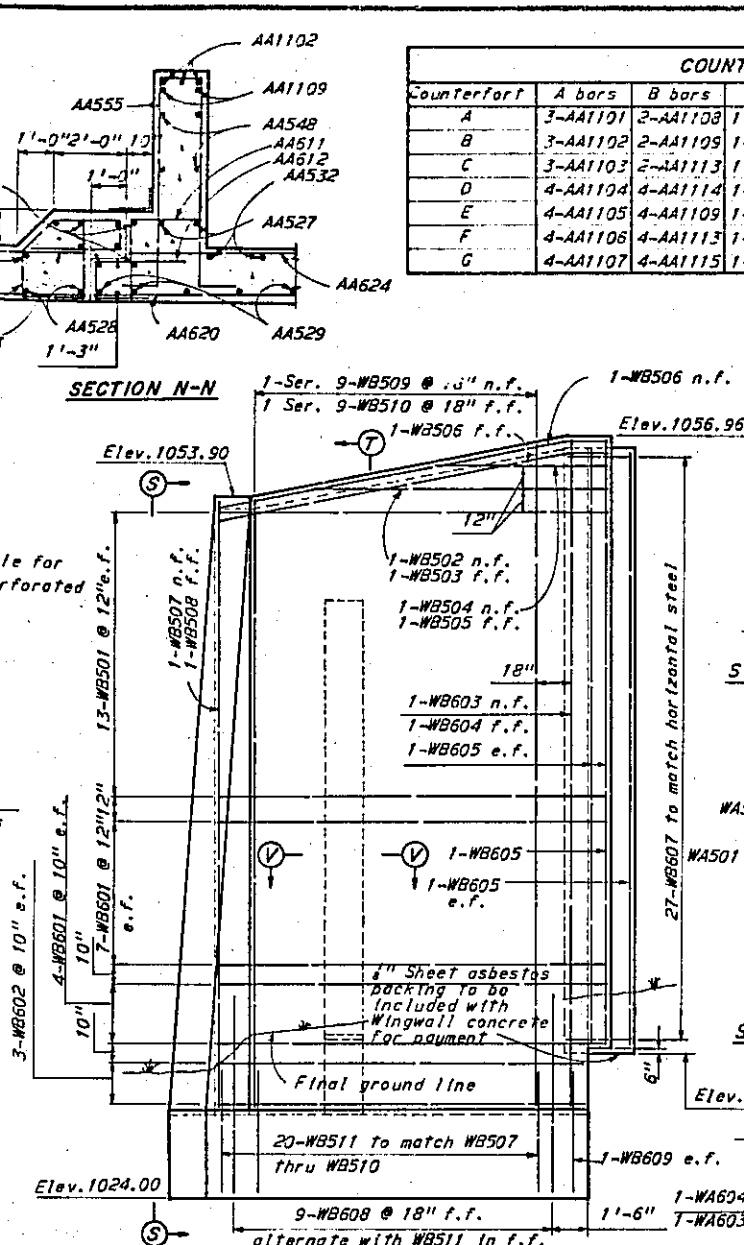
Backwall: Concrete. In addition to the provisions of 511.08, backwall concrete above the optional construction joint at the approach slab seat shall not be placed until after the deck concrete in the span adjacent to the abutment has been placed.

SECTION H-H

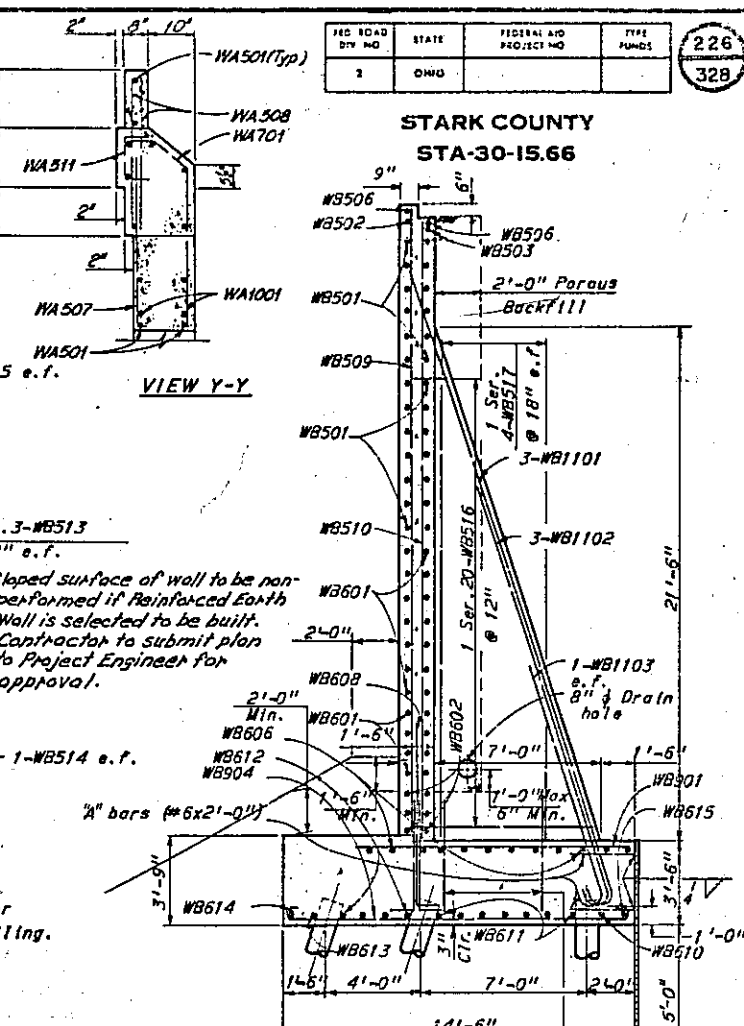
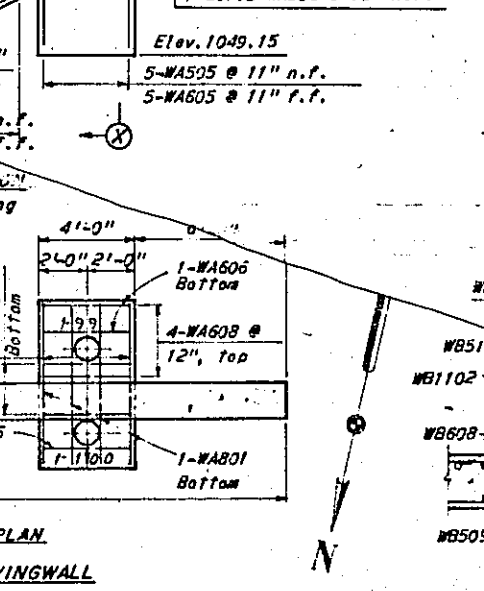
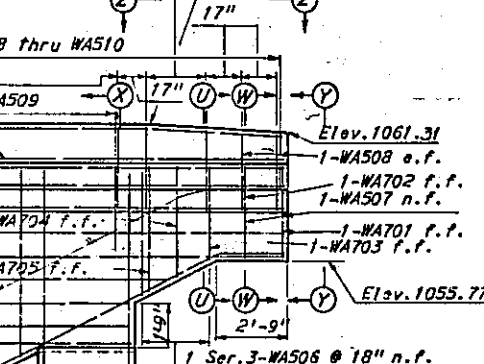
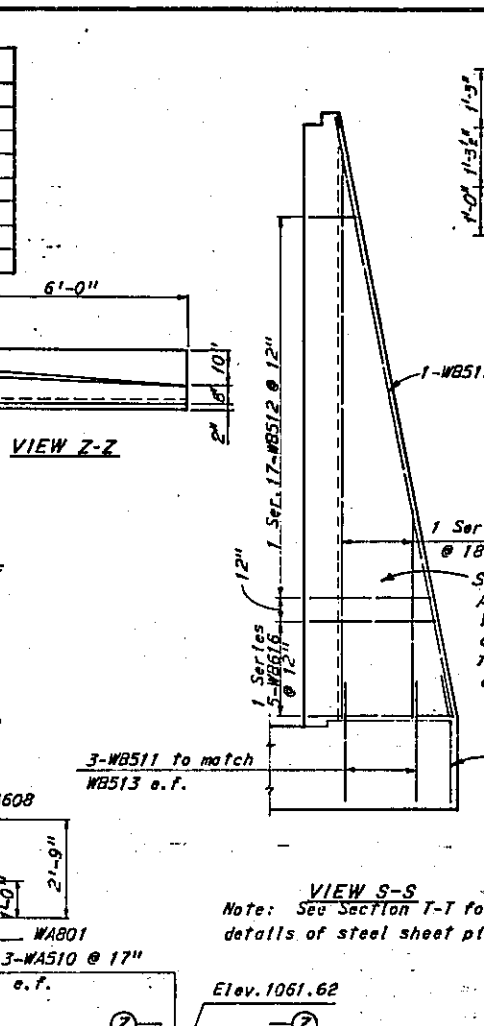
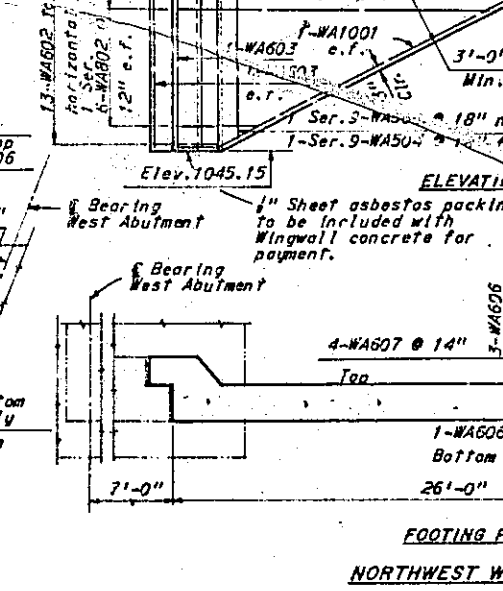
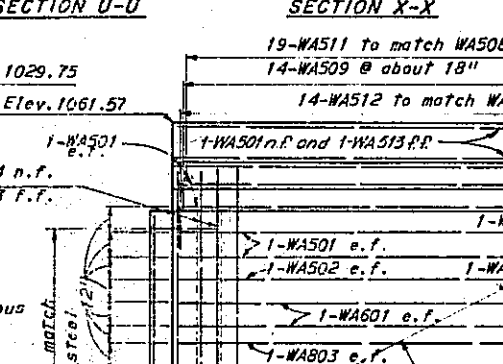
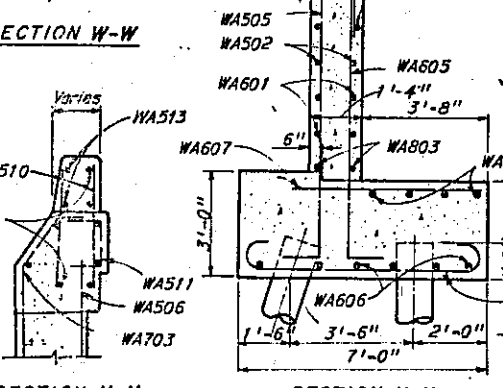
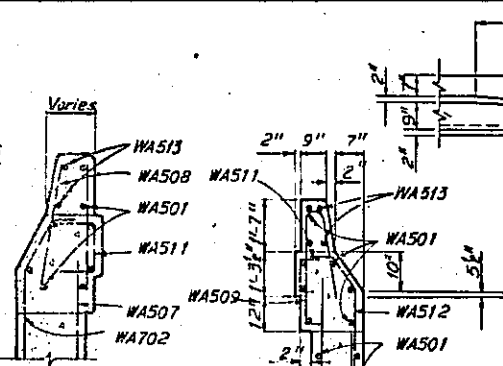
Note: See Section T-T for details of steel sheet piling.



Note: See Section T-T for details of steel sheet piling.



COUNTERFORT REINFORCING					
Counterfort	A bars	B bars	C bars	D bars	E bars
A	3-AA1101	2-AA1108	1-AA540 e.f.	1 Ser. 3-AA547 e.f.	1 Ser. 6-AA554
B	3-AA1102	2-AA1109	1-AA527 e.f.	1 Ser. 3-AA548 e.f.	1 Ser. 7-AA555
C	3-AA1103	2-AA1113	1-AA542 e.f.	1 Ser. 3-AA549 e.f.	1 Ser. 7-AA555
D	4-AA1104	4-AA1114	1-AA514 e.f.	1 Ser. 3-AA550 e.f.	1 Ser. 8-AA556
E	4-AA1105	4-AA1109	1-AA544 e.f.	1 Ser. 3-AA551 e.f.	1 Ser. 9-AA557
F	4-AA1106	4-AA1113	1-AA545 e.f.	1 Ser. 3-AA552 e.f.	1 Ser. 9-AA557
G	4-AA1107	4-AA1115	1-AA546 e.f.	1 Ser. 3-AA553 e.f.	1 Ser. 10-AA543



Notes:
All piles are 12" cast-in-place reinforced concrete.
All battered piles and sheet piling are battered 4 in 12.
○ Denotes battered piles and direction of batter.
Pile dimensions are measured along bottom of footing.
n.f. Denotes near face.
f.f. Denotes far face.
e.f. Denotes each face.
Piles are not shown in elevations.
For key details, see Sheets 2/20 and 3/20.
For reinforcement schedule see Sheets 18/20 and 19/20.
sheet steel piling for payment.
Sheet steel piling left in place shall be a minimum section modulus of 7 in. cubed per foot of wall. Used piling in good condition may be used.
Parapet transitions shall be as shown on Std. Drwg. BR-1-67 revised 1-1-71. Reinforcing Steel shall be field bent or cut to fit the revised shape.
Minimum reinforcing bar laps shall be 30 bar diameters.

H.N.T.B. BRIDGE NO. 18
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Revised 2-29-79 By RAS
4/20

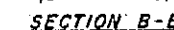
WEST ABUTMENT DETAILS

U.S. 30 OVER 11TH STREET
BR No. 1017-0675
SCALE 1"=10'-0"
STA. 1017+06.75
STA. 1023+33.25
RELOCATED U.S. 30

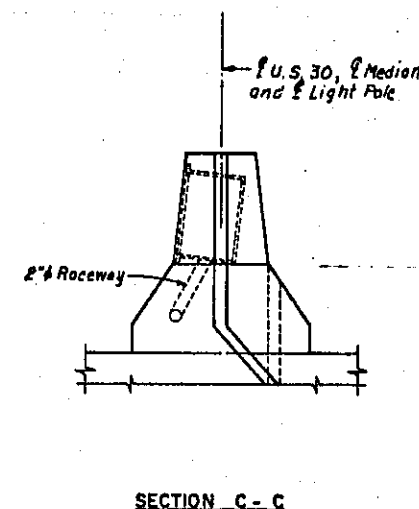
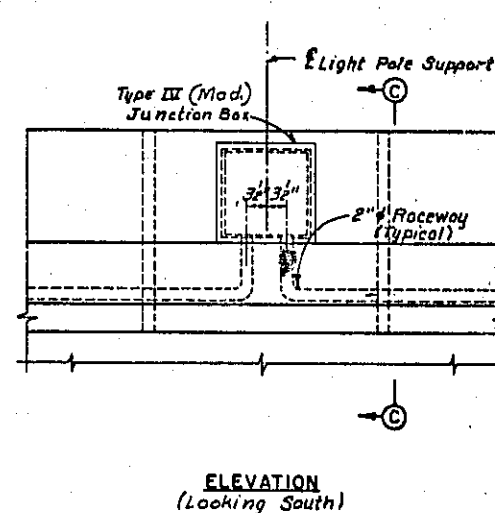
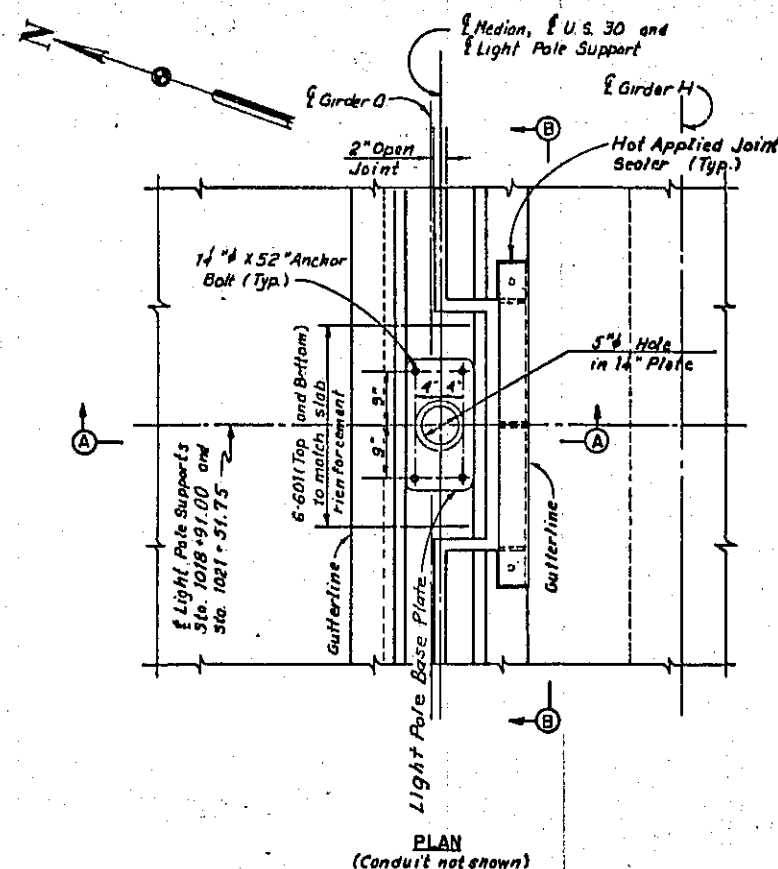
CANTON
STARK COUNTY
OHIO

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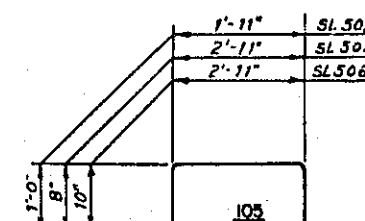
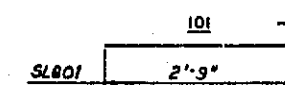
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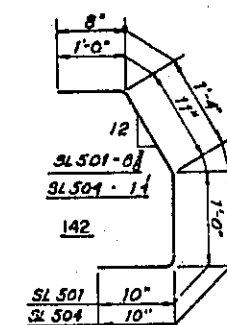
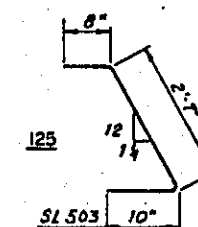
Notes:
For Parapet transition note:
see Sheet 4/20.
For other notes see Sheet 7/20
Minimum reinforcing bar laps
shall be 30 bar diameters.



REINFORCEMENT SCHEDULE					
MARK	NO.	LENGTH	TYPE	WEIGHT (LBS.)	
EFL501	5	3' - 7"	1A2	19	
EFL502	5	3' - 9"	105	20	
EFL503	4	3' - 11"	12S	16	
EFL504	4	3' - 11"	1A2	16	
EFL505	1	4' - 1"	105	4	
EFL506	3	4' - 5"	105	14	
EFL601	6	3'-5"	101	31	
Total Weight For One Light Support =				120	



Note: Reinforcement shown is for one (1) median mounted light pole support. Two (2) light pole supports are required.



Notes:

Transverse reinforcement bars SL501, SL502, SL503 and SL504 replace normal median transverse reinforcement bars SA621 thru SA627. Longitudinal reinforcement in the median shall be field cut as required at the light pole support. Field cutting shall be included with Item 509, Reinforcing Steel, for payment.

The 1/4" x 5/2" anchor bolts, nuts, and junction box shall be included with Item 625 for payment.

All structural steel comprising the curb area shall be included with Item 513, Structural Steel, for payment.

All structural steel shall be galvanized according to 711.02. Galvanizing of steel shall be included with

Item 513, Structural Steel, for payment. shall be included with

Hot applied joint sealer (705.01) shall be included with Item 511, Class "S" Concrete, Superstructure, for pavement.

For details of junction box and additional notes, see Ohio Standard Drawing HL-22

The following abbreviations are used:

n.f. = near face

$f, f_0 = \text{for face}$

Typ. = typical

Sig. = Station

H.N.T.B. BRIDGE NO. 18

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MEDIAN MOUNTED LIGHT POLE SUPPORT DETAILS

BB NO. STA-30-1595 STA 1017 0675

BR. NO. STA-50-1995
STA. 1017 06.75
STA. 1023 33.25

RELOCATED U.S. 30

CANTON STARK COUNTY OH

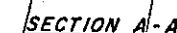
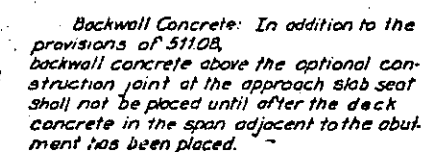
CANTON		STARK COUNTY		CHI	
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UNAWA DWS	THAGED DWS	CHECKED L.P.	REVIEWED	REVISED
DATE 3-23-79	DATE 3-26-79	DATE 4-6-79	DATE	

DATE 2/2/73	DATE 3/20/73	DATE 4/8/73	DATE
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1. *Journal of the American Medical Association*, 2000; 283: 2686-2692.



Notes:
For notes see Sheet **7/20**.
Minimum reinforcing bar
laps shall be 30 bar diameters.

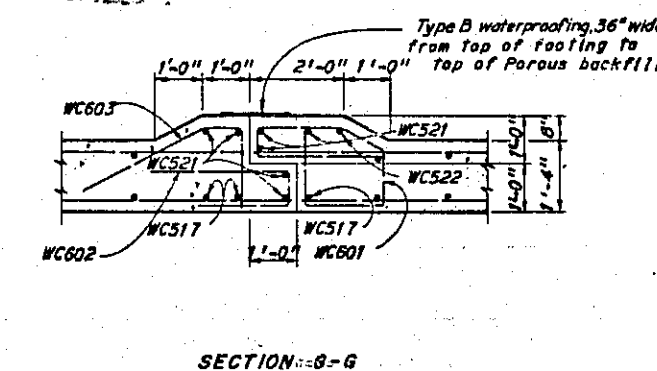
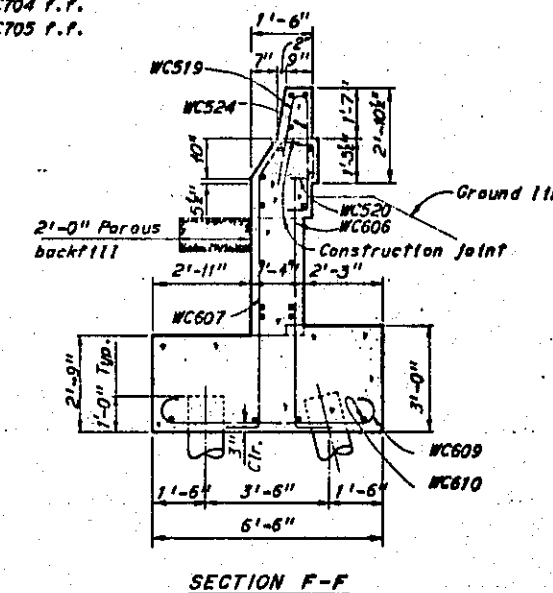
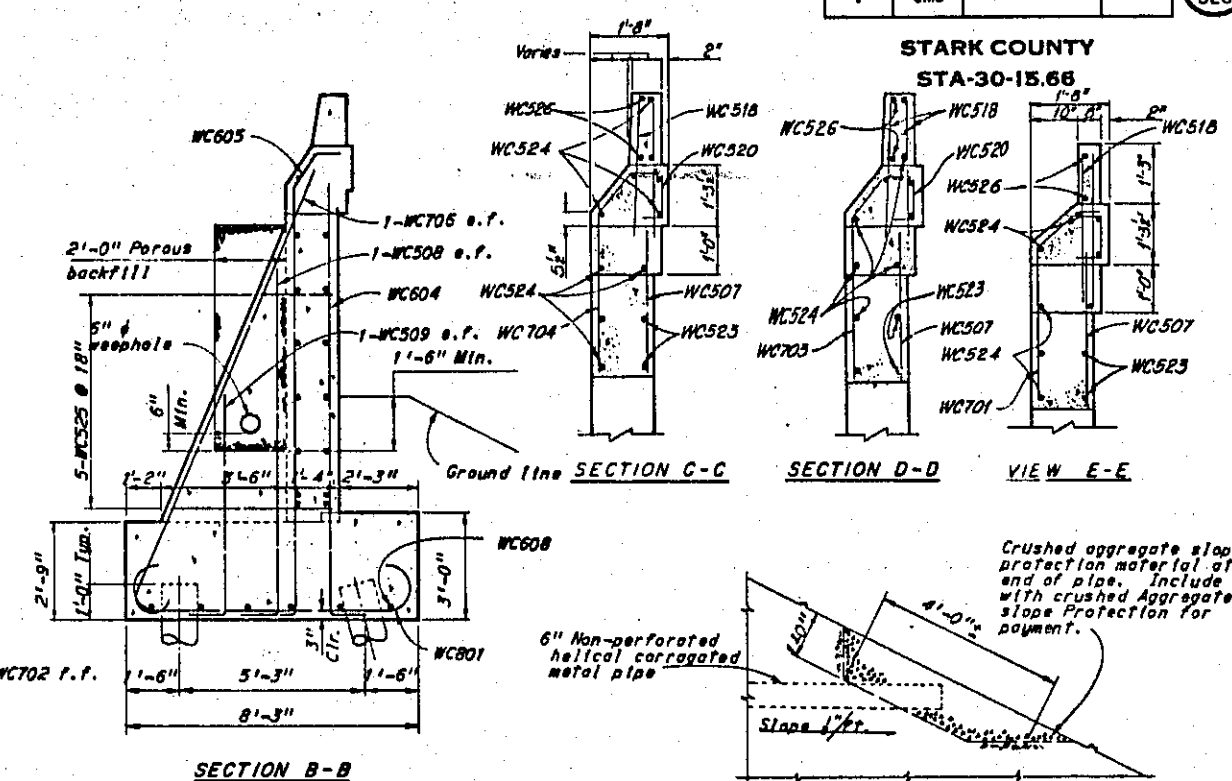
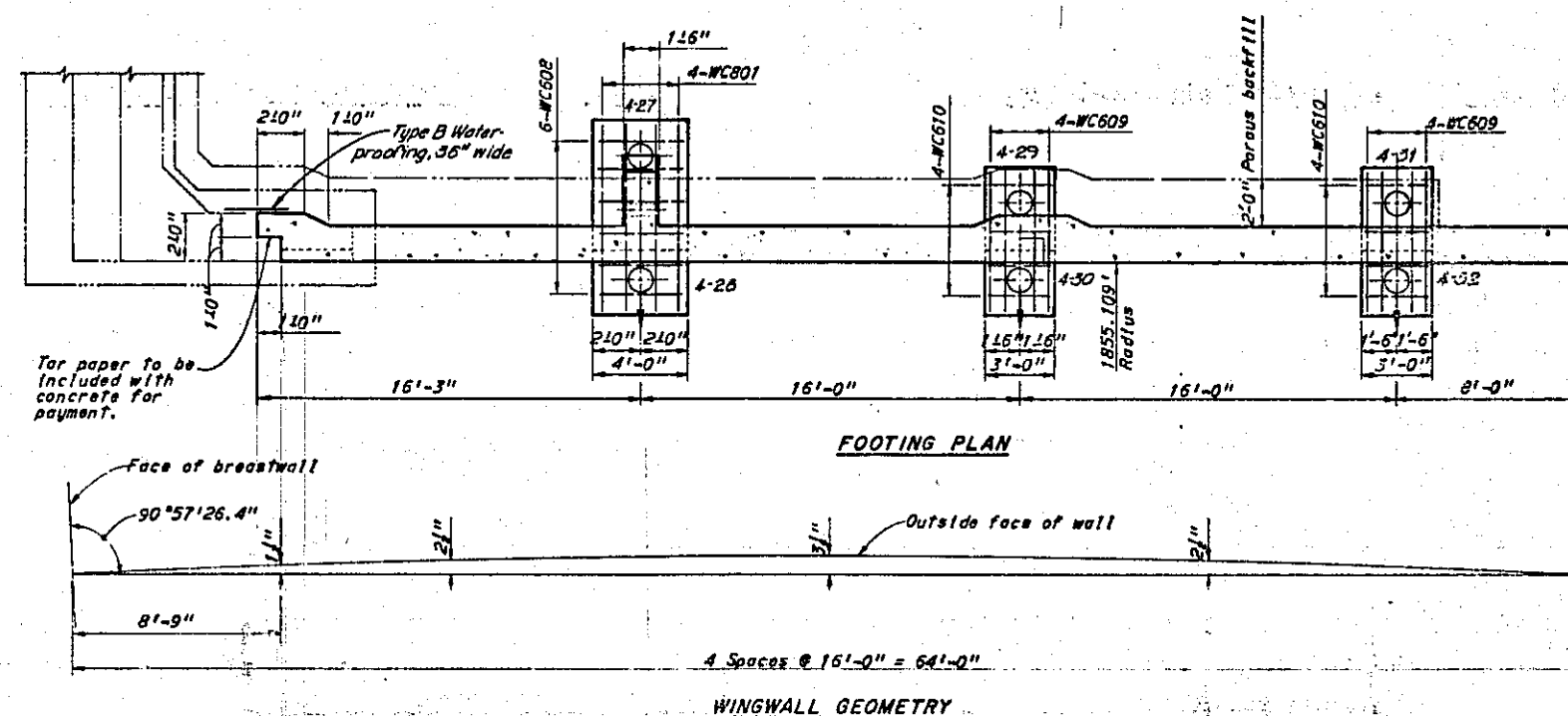
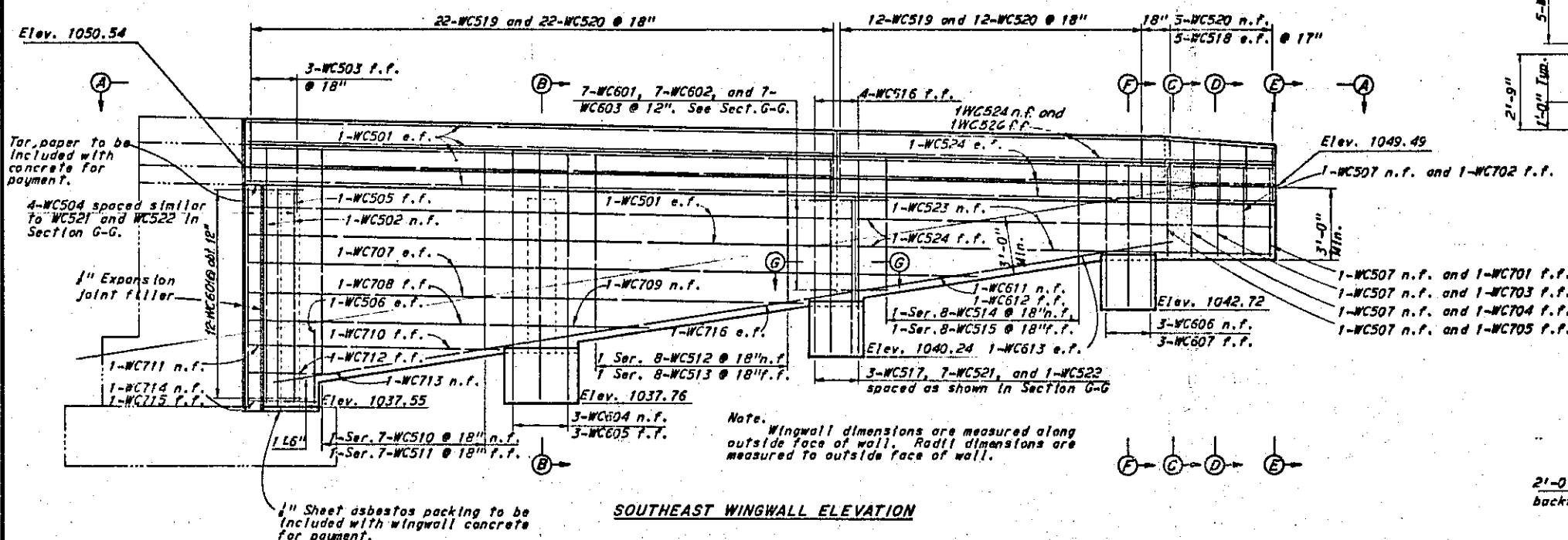
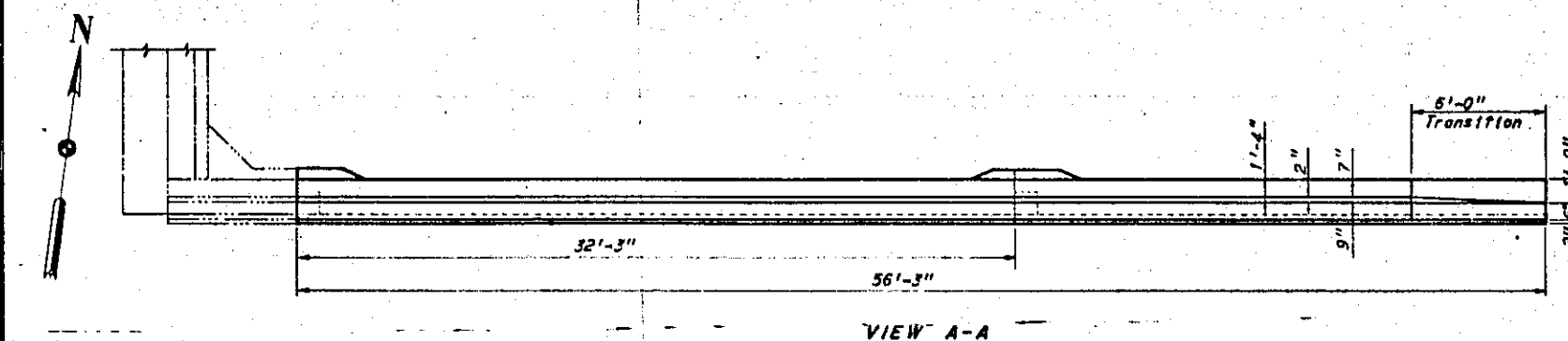
N.T.B. BRIDGE NO. 18 Revised 2-27-77 By RAS
 HOWARD, NEEDLES, TAMMEN & BERGENDOFF 6/2/77
 CONSULTING ENGINEERS
 KANSAS CITY CLEVELAND NEW YORK

EAST ABUTMENT

 U.S. 30 OVER 11TH STREET
 BR NO. STA. 30-15.95 STA. 1017+04.75
 SCALE: 1/4" = 1'-0" STA. 1023+34.25

RELOCATED U.S. 30
 CANTON STARKE COUNTY OHIO

 DRAWN T.P.S. TRACED CHECKED REVISOR
 DATE 8-7-79 DATE DATE DATE
 166



Notes:

For roadway end dam details see Ohio Standard Drawing SD-1-69, Sheets 1 and 2 of 4.

AB401 bars shall be placed 2' below top of bridge soot.

n.f. Denotes near face, f.f. denotes far face, and e.f. denotes each face.

For reinforcing schedule see Sheet 20/20.

All battered piles are battered 3 in 12.

Pile dimensions are measured along bottom of footing.

Piles not shown in elevation.

All piles are 12" ϕ C.I.P. concrete piles.

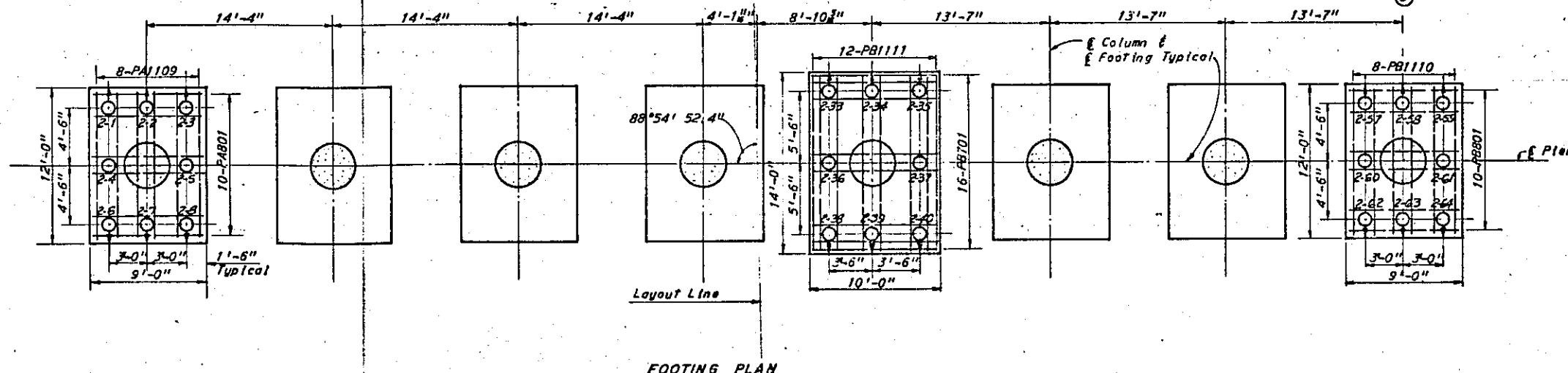
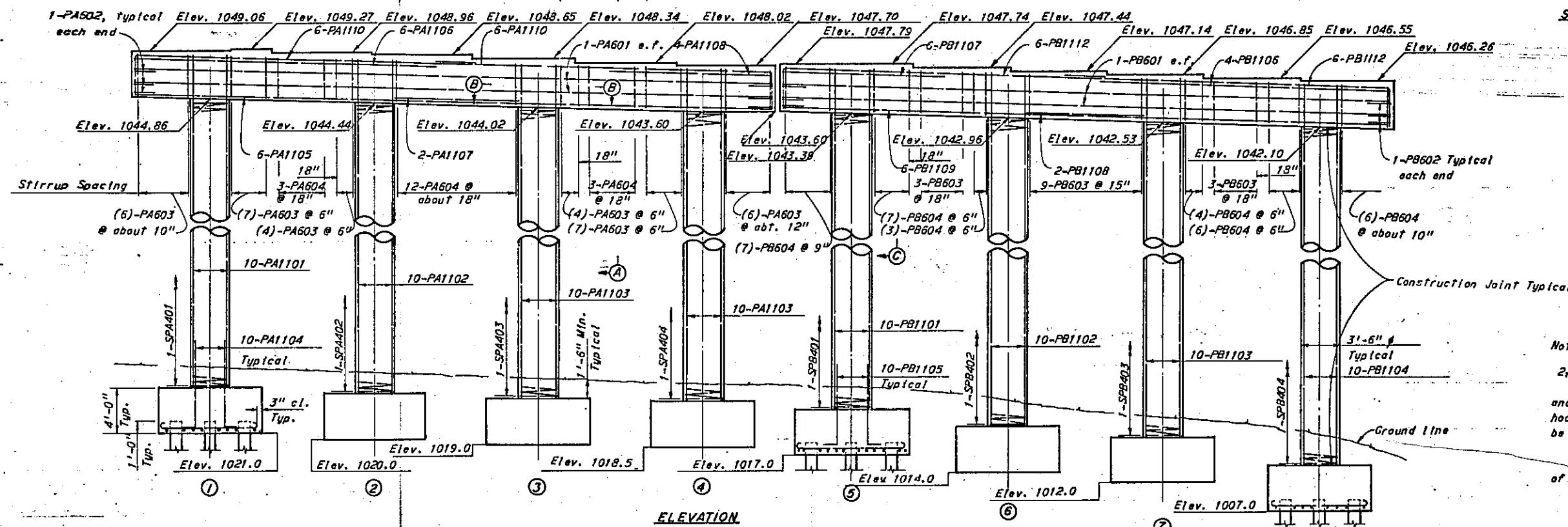
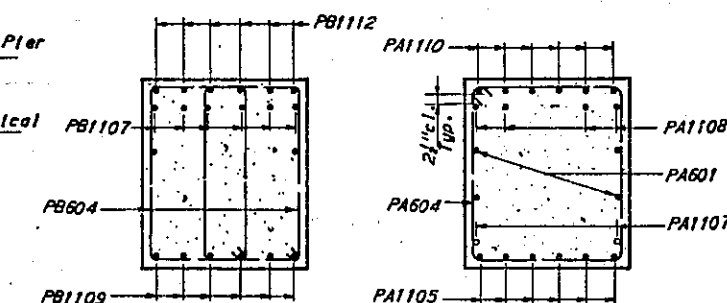
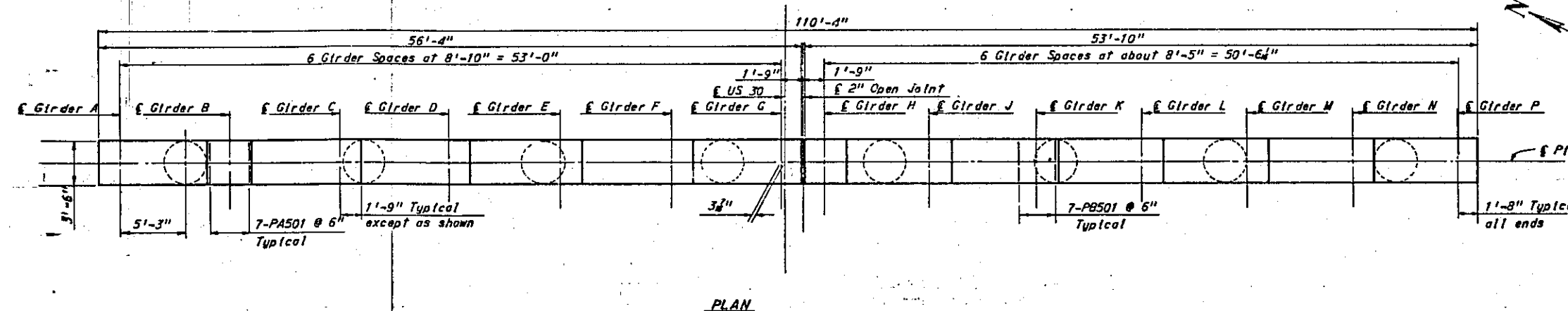
○ Denotes battered pile.

For notes concerning Parapet transition see Sheet 4/20.

Minimum reinforcing bar laps shall be 30 bar diameters.

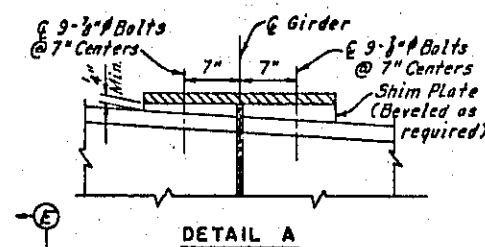
H.N.T.S. BRIDGE NO. 16		Revised E-29-79		By RAS	
HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONSULTING ENGINEERS				7/2	
KANSAS CITY CLEVELAND NEW YORK					
EAST ABUTMENT DETAILS					
U.S. 30 OVER 11TH STREET					
BR. NO. STA. 30-15.95			STA. 1017+06.75		
SCALE: 3/8" = 1'-0"			STA. 1023+33.25		
RELOCATED U.S. 30					
CANTON			STARK COUNTY		OHIO
DRAWN T. R. S.	TRACED	CHECKED	REVISED	DATE	REVIEWED
DATE 8-16-70	DATE	DATE 8-6-70	DATE 8-12-70		1661

STARK COUNTY
STA-30-15.66



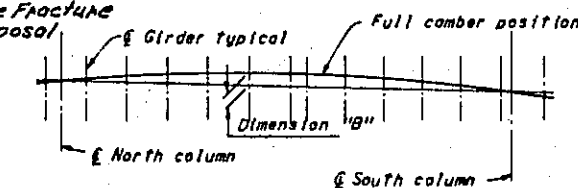
Notes:
PA501 and PB501 bars shall be placed 2" ± below top of cap beam.
The stirrups within 4 feet of the columns and in cantilever spans shall be placed with the hooks in the bottom of cap beam. All others shall be placed with hooks in the top.
All piles are 12" cast-in-place concrete.
Pile dimensions are measured along bottom of footing.
○ denotes battered pile.
All battered piles shall be battered 3 in 12.
e.f. denotes each face.
(*) denotes number of sets of 2 stirrup bars required. (7)-PA603 denotes 7 sets of 2-PA603.
Footings ②, ③ and ④ are identical to footing ①.
Footings ⑥ and ⑦ are identical to footing ⑤.
For reinforcement schedule see Sheet 19/20.
Minimum reinforcing bar laps shall be 30 bar diameters.

H.N.T.B. BRIDGE NO. 18		Revised 2-29-79 By RAS	
HOWARD, NEEDLES, TAMMEN & BERGENDOFF		8/20	
CONSULTING ENGINEERS		KANSAS CITY CLEVELAND NEW YORK	
PIER 1			
U.S. 30 OVER 11TH STREET			
BR. NO. STA-30-15.65	STA. 1017+06.75		
SCALE: 3/16" = 1'-0"	STA. 1023+33.25		
RELOCATED U.S. 30			
CANTON	STARK COUNTY	OHIO	
DATE 9/21/78	DATE 10/10/78	DATE 11/11/78	DATE 12/12/78



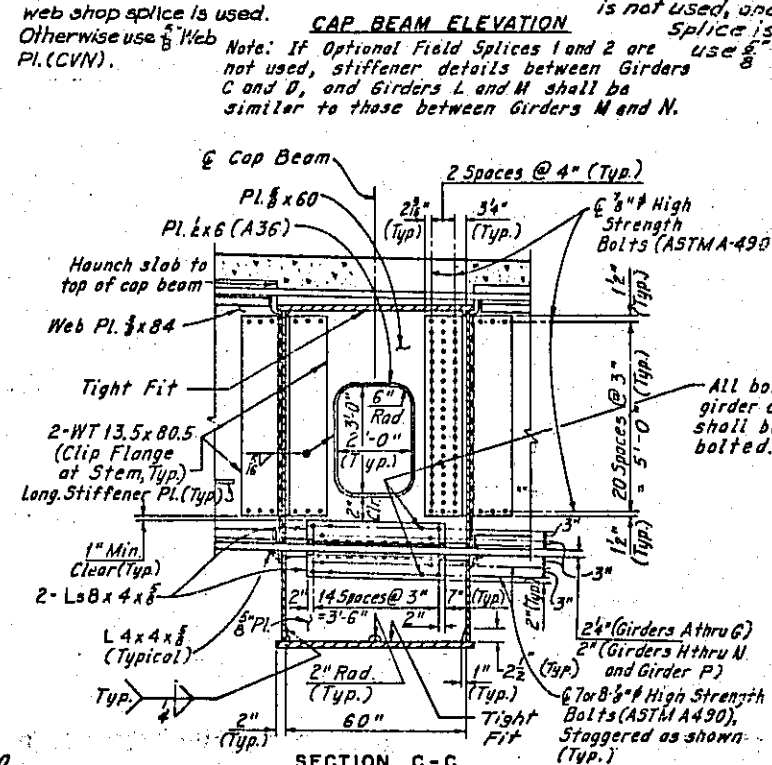
ALTERNATE CAPBEAM WEB TO FLANGE WELD:
A full penetration groove weld, with a continuous backup plate may be used as an alternate to the 1/4" fillet weld, both sides as shown on the capbeam elevation.

Note:
Material shall be fabricated
in accordance with the Fracture
Control Plan. See Proposal
Note.

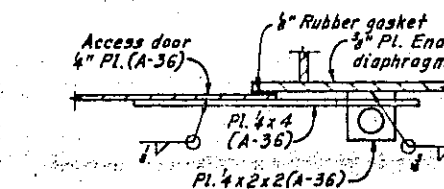


CAMBER DIAGRAM

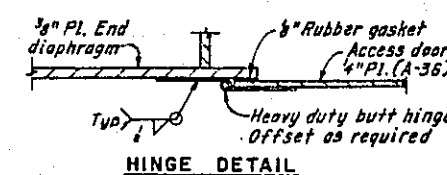
Note: 30% of dead load deflection due to steel



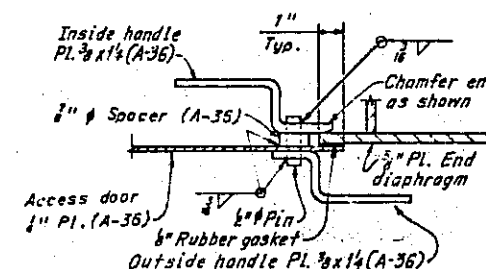
VIEW E-E
(South End Shown, North End Similar)



HASP DETAIL



HINGE DETAIL



HANDLE DETAIL

SECTION F-F

(Typical, All Girders - Girder flange
splice plates and flange to web
angles not shown).

Notes:

All structural steel shall be painted in accordance with item 514 of the specifications.

All bolts shown shall be 1/2" ASTM A-490 bolts, Type 3.

All steel shall be A-588, unless shown otherwise.

Longitudinal stiffeners shall be Pls. 1/2x6 (A-36) on outside face of webs only.

Transverse intermediate stiffeners shall be Pls. 1/2x9 (A-36) on inside face of webs and placed as shown and noted.

Bearing stiffeners and bearing diaphragms shall be either milled to bear on bottom flange or be attached to the bottom flange by a complete joint penetration weld.

Top and bottom flange plates shall be horizontal, normal to the centerline of the capbeam.

For Splice Details, see Sheet CD 4.

For bearing shoe details and column and footing details, see Sheet 10/20.

For longitudinal girder data, see Sheets 12/20 and 13/20.

H.N.T.S. BRIDGE NO. 18

HOWARD, NEEDLES, TAMMEN & BERGENDOFF	9 / 20
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STEEL CAP BEAM
PIER 2

U.S. 30 OVER 11TH STREET
 DP. NO. STA. 30+15.95 STA. 1017+06.75
 SCALE: 3/16" = 1'-0" STA. 1023+33.25

RELOCATED U.S. 30

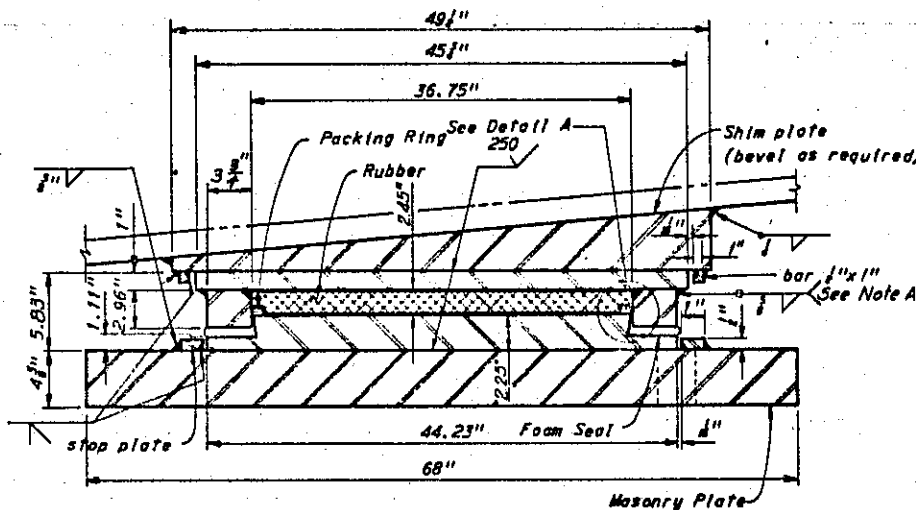
CANTON STARK COUNTY OHIO

DRAWN <i>C.P.</i>	TRACED <i>C.P.</i>	CHECKED <i>R.A.S.</i>	REVIEWED	REVISED
DATE <i>7-23-80</i>	DATE <i>8-4-80</i>	DATE <i>8-5-80</i>	DATE	

100

$$E_{\text{eff}} = \frac{1}{2} \left(\frac{1}{\epsilon_0} + \frac{1}{\epsilon_0} \right) = \frac{1}{\epsilon_0} \quad \text{for } \epsilon_0 = 1$$

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119 120 121 122 123 124 125 126 127 128 129 130 131 132 133 134 135 136 137 138 139 140 141 142 143 144 145 146 147 148 149 150 151 152 153 154 155 156 157 158 159 160 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188 189 190 191 192 193 194 195 196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220 221 222 223 224 225 226 227 228 229 230 231 232 233 234 235 236 237 238 239 240 241 242 243 244 245 246 247 248 249 250 251 252 253 254 255 256 257 258 259 260 261 262 263 264 265 266 267 268 269 270 271 272 273 274 275 276 277 278 279 280 281 282 283 284 285 286 287 288 289 290 291 292 293 294 295 296 297 298 299 300 301 302 303 304 305 306 307 308 309 310 311 312 313 314 315 316 317 318 319 320 321 322 323 324 325 326 327 328 329 330 331 332 333 334 335 336 337 338 339 340 341 342 343 344 345 346 347 348 349 350 351 352 353 354 355 356 357 358 359 360 361 362 363 364 365 366 367 368 369 370 371 372 373 374 375 376 377 378 379 380 381 382 383 384 385 386 387 388 389 390 391 392 393 394 395 396 397 398 399 400 401 402 403 404 405 406 407 408 409 410 411 412 413 414 415 416 417 418 419 420 421 422 423 424 425 426 427 428 429 430 431 432 433 434 435 436 437 438 439 440 441 442 443 444 445 446 447 448 449 450 451 452 453 454 455 456 457 458 459 460 461 462 463 464 465 466 467 468 469 470 471 472 473 474 475 476 477 478 479 480 481 482 483 484 485 486 487 488 489 490 491 492 493 494 495 496 497 498 499 500 501 502 503 504 505 506 507 508 509 510 511 512 513 514 515 516 517 518 519 520 521 522 523 524 525 526 527 528 529 530 531 532 533 534 535 536 537 538 539 540 541 542 543 544 545 546 547 548 549 550 551 552 553 554 555 556 557 558 559 560 561 562 563 564 565 566 567 568 569 570 571 572 573 574 575 576 577 578 579 580 581 582 583 584 585 586 587 588 589 590 591 592 593 594 595 596 597 598 599 600 601 602 603 604 605 606 607 608 609 610 611 612 613 614 615 616 617 618 619 620 621 622 623 624 625 626 627 628 629 630 631 632 633 634 635 636 637 638 639 640 641 642 643 644 645 646 647 648 649 650 651 652 653 654 655 656 657 658 659 660 661 662 663 664 665 666 667 668 669 670 671 672 673 674 675 676 677 678 679 680 681 682 683 684 685 686 687 688 689 690 691 692 693 694 695 696 697 698 699 700 701 702 703 704 705 706 707 708 709 710 711 712 713 714 715 716 717 718 719 720 721 722 723 724 725 726 727 728 729 730 731 732 733 734 735 736 737 738 739 740 741 742 743 744 745 746 747 748 749 750 751 752 753 754 755 756 757 758 759 760 761 762 763 764 765 766 767 768 769 770 771 772 773 774 775 776 777 778 779 780 781 782 783 784 785 786 787 788 789 790 791 792 793 794 795 796 797 798 799 800 801 802 803 804 805 806 807 808 809 810 811 812 813 814 815 816 817 818 819 820 821 822 823 824 825 826 827 828 829 830 831 832 833 834 835 836 837 838 839 840 841 842 843 844 845 846 847 848 849 850 851 852 853 854 855 856 857 858 859 860 861 862 863 864 865 866 867 868 869 870 871 872 873 874 875 876 877 878 879 880 881 882 883 884 885 886 887 888 889 890 891 892 893 894 895 896 897 898 899 900 901 902 903 904 905 906 907 908 909 910 911 912 913 914 915 916 917 918 919 920 921 922 923 924 925 926 927 928 929 930 931 932 933 934 935 936 937 938 939 940 941 942 943 944 945 946 947 948 949 950 951 952 953 954 955 956 957 958 959 960 961 962 963 964 965 966 967 968 969 970 971 972 973 974 975 976 977 978 979 980 981 982 983 984 985 986 987 988 989 990 991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004 1005 1006 1007 1008 1009 1010 1011 1012 1013 1014 1015 1016 1017 1018 1019 1020 1021 1022 1023 1024 1025 1026 1027 1028 1029 1030 1031 1032 1033 1034 1035 1036 1037 1038 1039 1040 1



The rubber used in the bearings shall be manufactured to the highest standards from a compound based on natural rubber and free from fillers other than carbon black and zinc oxide in sufficient quantity to give the required hardness. An antioxidant must be included in sufficient quantity and of such a type that a sample of the rubber tested in accordance with ASTM D 1149 will show no evidence of cracking when subjected to a concentration of one part per million ozone and a strain of 20 per cent. The rubber shall have the following characteristics.

Hardness	50 - 55 Shore A durometer
tensile strength	2700 lbs./sq. in.
tear strength	285 lbs./sq. in.
elation at break	600%
compression set.	25% of original thickness (ASTM D 395 Method B)

The bearings shall show no detectable increase in stiffening rate after 168 hours at -30°C . (-22°F .) and shall be no more than twice as stiff at this temperature as compared with $+20^{\circ}\text{C}$. ($+68^{\circ}\text{F}$.)

All structural steel in these bearings shall conform to ASTM A-36. Shim and masonry plate shall conform to ASTM A-588.

When assembling, the rubber should be entirely coated with a suitable lubricant which will have no adverse effect on materials in the bearings. A minimum of four brass split sealing rings, coated with tetra fluoro-ethylene fluorocarbon resin on their outer edges, shall be placed so as to prevent any possibility of extrusion between piston and cylinder. Split lines shall be offset 90° relative to each other.

The piston and cylinder assembly shall be sealed with a heavy polyurethane foam impregnated throughout with a special solution of petroleum resins, isobutylene and isoprene. This seal shall be held in compression by top screws holding the assembly together for shipping and installation. These screws shall be designed to break away under load.

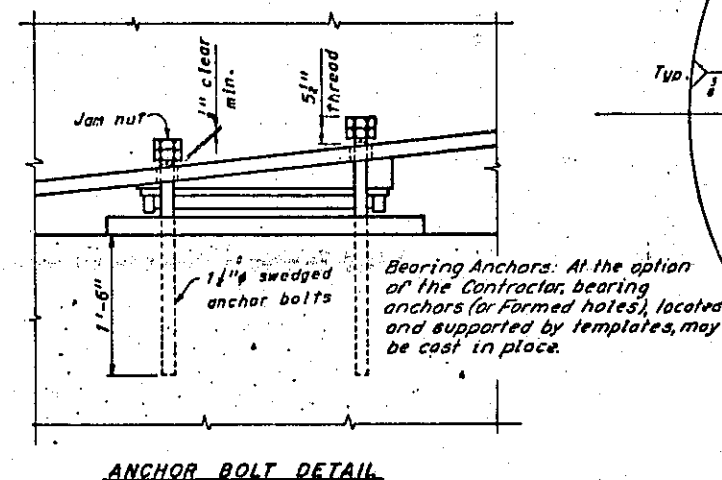
Welds shall be made and tested in accordance with the standard specifications. The welds shall be full fillet welds and the preparation for the internal weld of the cylinder shall be as shown.

Mating horizontal surfaces in the bearing plates shall have a finish not less than USASI #250. This same limit applies to the outer surfaces of the bearings and bearing of sole plates in direct contact with the bearings.

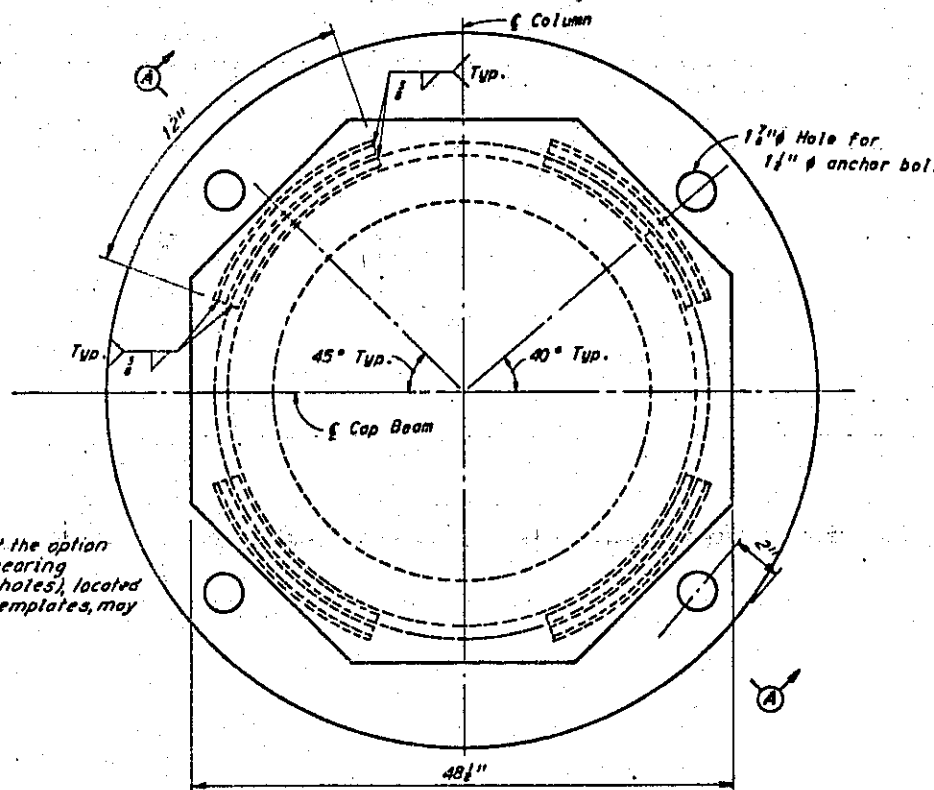
All external metal shall be metalized.

In addition to the requirements of the standard specifications, certified test reports from a recognized testing laboratory acceptable to the engineer, showing compliance with the above requirements, shall be furnished by the contractor at no extra cost to the State. Bearing devices shall be delivered to the job site at least 15 days prior to the date required for installation in order that tests may be conducted on the units to be used if required by the engineer.

No separate measurement and payment will be made for furnishing and placing the confined elastomer utilized in bearing devices, it being understood that the payment therefore is included in the payment made for structural steel, Item 513.



ANCHOR BOLT DETAIL



PLAN

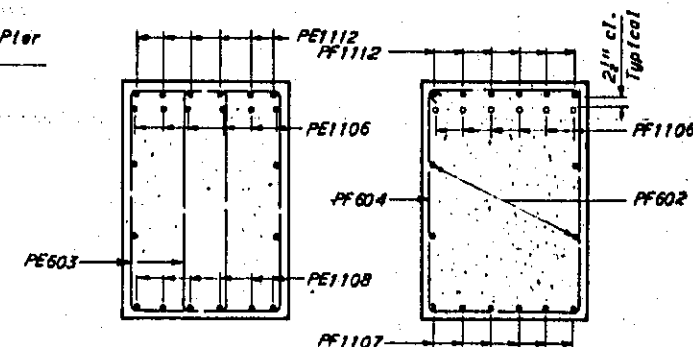
Note: Bearing is symmetrical about both axes.

Note: Special care shall be taken when placing reinforcing steel so as not to interfere with anchor bolt setting.
(N) denotes North Column - (S) denotes South Column
Bars marked PC are for North Column
Bars marked PD are for South Column
For footing layout see Sheet 1/20.
For Cap Beam details see Sheet 9/20.
For reinforcement schedule see Sheet 13/20.
Minimum reinforcing bar laps shall be 30 bar diameters

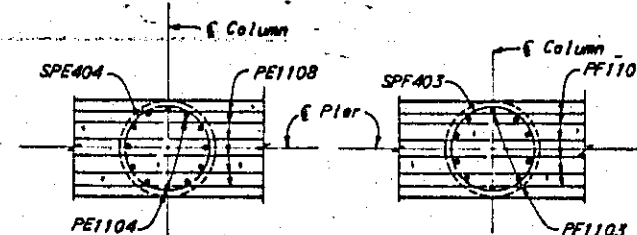
Bearing Notes:
In addition to the provisions of Section 500 of the Specifications, and particularly those of 60104, additionally the bearing fabricator shall have had at least 6 years of experience in constructing, testing and furnishing bearings similar to those described herein for other transportation projects.

BEARING DEVICE DETAILS
(Bearing Capacity-1850 Tons)

STARK COUNT
STA-30-15.66



SECTION A-A



SECTION C-C

Notes:

PE501 and PF501 bars shall be placed $2\frac{1}{2}" \pm$ below top of cap beam.

The stirrups within 4 feet of E columns and in cantilever spans shall be placed with the hooks in the bottom of the cap beam. All others shall be placed with hooks in the top.

All piles are 12" x cast-in-place concrete. Pile dimensions are measured along bottom of footing.

○ — Denotes battered pile

All battered piles shall be battered 3 in 12 e.r. Denotes each face

(*) Denotes number of sets of 2 stirrups bars required. (7) — PE603 denotes 7 sets of 2-PE603.

Footing ④ is identical to footing ⑦.

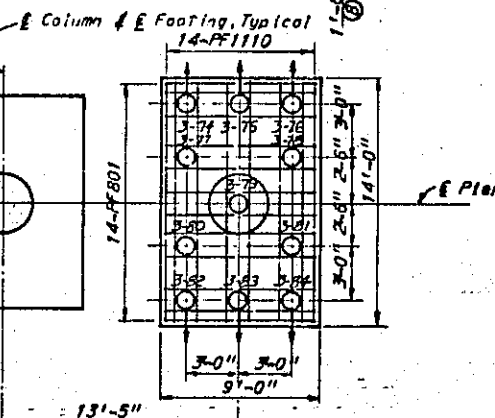
Footing ② is identical to footing ③.

Footing ⑤ is identical to footing ⑥ except as noted.

Footing ⑦ is identical to footing ⑥.

For reinforcement schedule see Sheet 281.

Minimum reinforcing bar laps shall be 30 bar diameters.



M.N.T.B. BRIDGE NO. 18

Revised 2-29-79 By RA

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CONSULTING ENGINEERS
RAMBAM CITY CLEVELAND NEW YORK

PIER 3

U.S. 30 OVER 11TH STREET

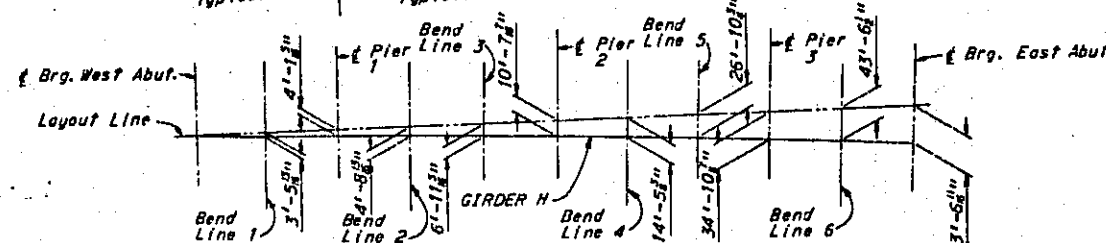
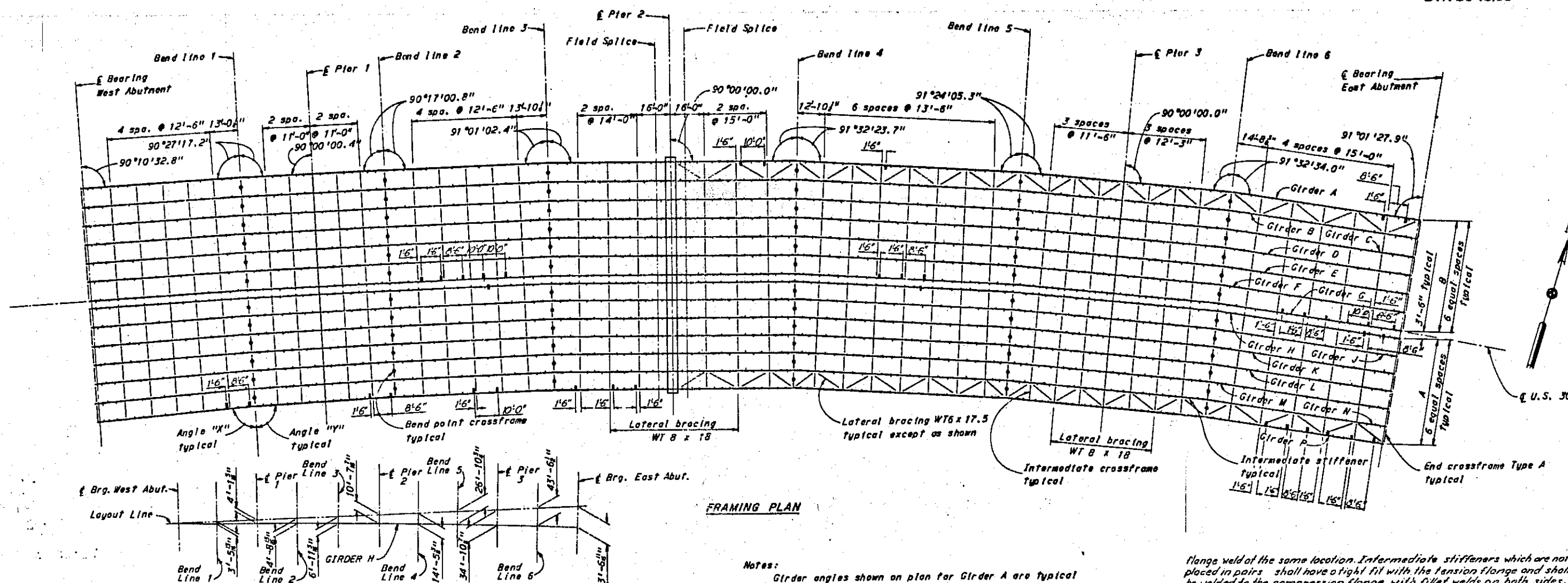
BR. NO. STA. 30-15.95 STA. 1017+06.75
SCALE: 3/16" = 1'-0" STA. 1023+33.25

RELOCATED U.S. 30

CANTON STARK COUNTY OHIO

DRAWN BY mag	TRACED	CHECKED JWC	REVIEWED WJK	REVISED
DATE 2-23-70	DATE	DATE 4-11-70	DATE 4-11-71	166

STARK COUNTY
STA-30-15.06



LAYOUT SKETCH

GIRDER ANGLES "X" AND "Y"					
	WEST ABUTMENT	BEND AND SPLICE LINE 1		PIER 1	BEND AND SPLICE LINE 2
GIRDER	ANGLE "Y"	ANGLE "X"	ANGLE "Y"	ANGLE "X"	ANGLE "Y"
H	89°49'29.3"	89°32'40.7"	89°32'41.6"	90°00'00.8"	89°42'59.2"
J	90°06'14.9"	89°15'55.1"	89°41'27.0"	89°51'15.4"	89°34'15.0"
K	90°23'02.6"	88°59'07.4"	89°50'12.7"	89°42'29.7"	89°25'29.3"
L	90°39'52.8"	88°42'17.2"	89°59'03.3"	89°33'39.1"	89°16'38.7"
M	90°56'45.4"	88°25'24.6"	90°07'52.6"	89°24'49.8"	89°07'49.4"
N	91°13'40.4"	88°08'29.6"	90°16'43.5"	89°15'58.9"	88°58'58.5"
P	91°30'37.7"	87°51'32.3"	90°25'39.7"	89°07'02.7"	88°50'02.3"

GIRDER SPACING		
	A	B
WEST ABUT.	53'-2.8"	53'-0"
BEND 1	51'-0.8"	53'-0"
PIER 1	50'-1.6"	53'-0"
BEND 2	50'-0"	53'-0"
BEND 3	50'-0.8"	53'-0.8"
PIER 2	50'-0"	53'-0"
BEND 4	50'-0.8"	53'-0.8"
BEND 5	50'-0.8"	53'-0.8"
PIER 3	50'-0"	53'-0"
BEND 6	50'-0.8"	53'-0.8"
EAST ABUT.	50'-0.8"	53'-0.8"

FRAMING PLAN

Notes:

Girder angles shown on plan for Girder A are typical for all girders except as shown in table of girder angles. Dimensions to crossframes are along Girder A. All crossframes are not perpendicular to Girder A.

Top and bottom flanges are identical in size and material. All stiffeners shall be placed normal to girder flanges.

All intermediate stiffener to web welds shall be 1/2" continuous fillets on both sides of stiffeners.

For detail of field splices see Sheet **CD4**.

Intermediate stiffeners shall be Pls. 4 & 8, (ASTM A36). Intermediate stiffeners at crossframes shall be placed in pairs, except that no intermediate stiffeners shall be placed on the exterior face of Girders A and P.

For Camber and Deflection Table see Sheet **17/20**.

Intermediate stiffeners shall be placed as shown on framing plan, equally spaced between crossframes or crossframes and bearing stiffeners or crossframes and splices, or splice and web of cap beam.

The girders shall be fabricated to lines parallel to profiles formed by top of pavement elevations directly over girders, plus the camber required to compensate for dead load deflection.

The allowance to be made in screed settings to compensate for dead load deflections due to concrete and asphalt wearing course are to be made above or below the top of the pavement elevations as required. Screeds may require further adjustment due to irregularities in fabricated steel.

For details of curb plates of concrete bridge railing see Ohio Standard Drawing SD-1-69, Sheet 2 of 4.

Intermediate stiffeners shall have a tight fit with the tension flange and may have either a tight fit with, or be welded to the compression flange with fillet welds on both sides, the same size as the web to

flange weld of the same location. Intermediate stiffeners which are not placed in pairs shall have a tight fit with the tension flange and shall be welded to the compression flange with fillet welds on both sides, the same size as the web to flange weld at the same location.

For intermediate crossframe details see Sheet **15/20**.

All crossframes are intermediate crossframes unless shown otherwise.

For details of roadway and dam, see Ohio Standard Drawing SD-1-69, Sheet 1 of 4, and Sheet **CD5**.

For bend point crossframe details see Sheet **16/20**.

Denotes bend point crossframe.

For details of standard scupper see Ohio Drawing SD-1-69, Sheet 3 of 4.

For underpass light support details see Lighting Plans.

For end crossframe Type A details see Sheet **CD5**.

Denotes Ohio standard scupper, to be placed as shown.

For lateral bracing connection details see Sheet **13/20**.

All structural steel shall be painted in accordance with Item 514 of the specifications.

H.N.T.B. BRIDGE NO. 18 Revised J-23-79 By RAS

HOWARD, NEEDLES, TAMMEN & BERGENDOFF
CONSULTING ENGINEERS
KANSAS CITY CLEVELAND NEW YORK

FRAMING PLAN

U.S. 30 OVER 11TH STREET

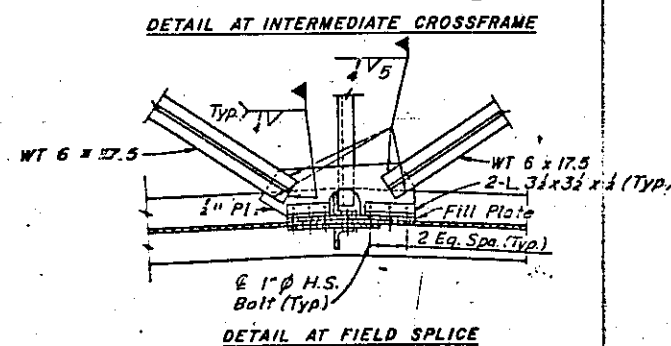
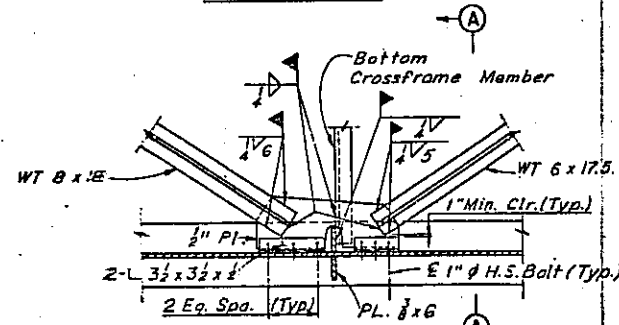
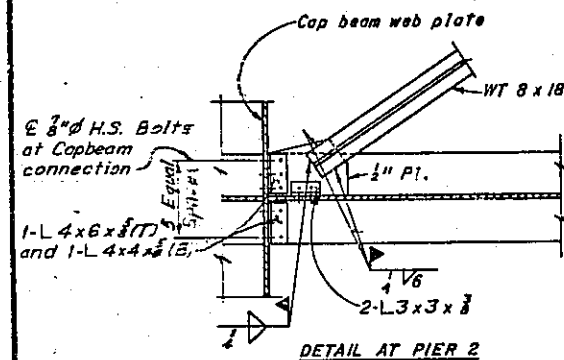
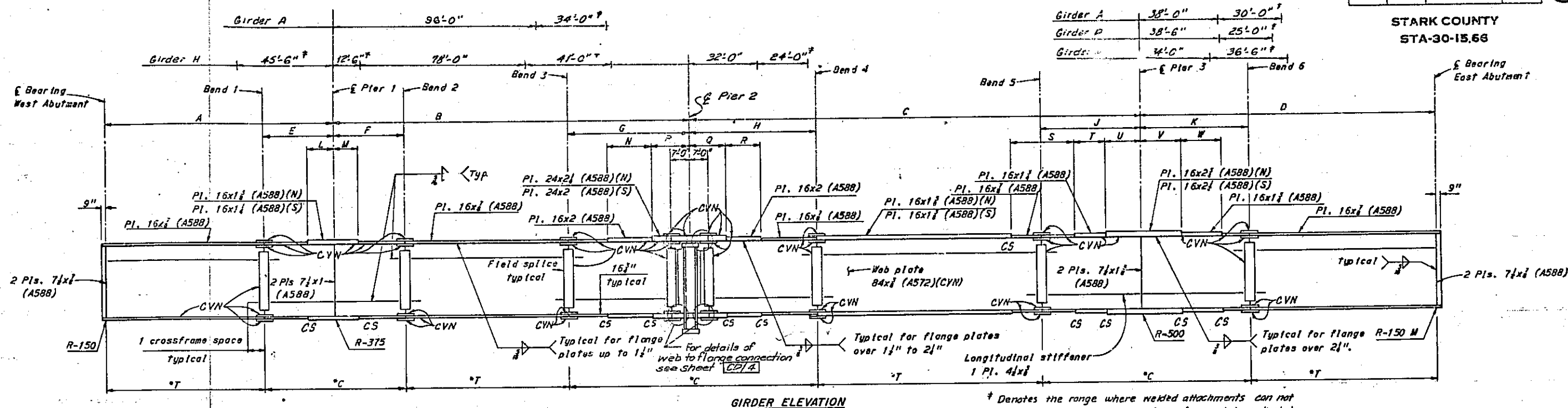
BR. NO. STA-30-15.95 STA. 1017+06.75
SCALE: 1" = 25'-0" STA. 1023+33.25

RELOCATED U.S. 30

CANTON STARK COUNTY OHIO

DRAWN CAC	TRACED	CHECKED MJS	REVIEWED RLA	REVISED
DATE 6-18-70	DATE	DATE 7-27-72	DATE 8-28-71	1668

STARK COUNTY
STA-30-15.68



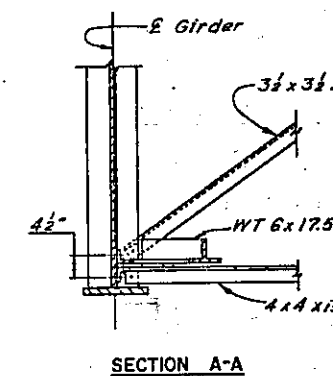
LATERAL BRACING CONNECTION DETAILS

All bolts are 1" high strength, ASTM A325, except as noted.

GIRDER DIMENSIONS										
GIRDER	A	B	C	D	E	F	G	H	J	K
A	109'-6"	168'-11"	215'-5"	140'-9"	33'-5"	33'-5"	57'-11"	60'-5"	48'-3"	51'-5"
B	109'-4"	168'-6"	214'-7"	140'-2"	33'-4"	33'-4"	57'-9"	60'-2"	48'-1"	51'-2"
C	109'-2"	168'-1"	213'-8"	139'-6"	33'-3"	33'-3"	57'-8"	59'-11"	47'-10"	50'-11"
D	109'-0"	167'-8"	212'-9"	138'-11"	33'-2"	33'-1"	57'-7"	59'-8"	47'-7"	50'-8"
E	108'-10"	167'-3"	211'-10"	138'-3"	33'-1"	33'-1"	57'-3"	59'-5"	47'-5"	50'-5"
F	108'-8"	166'-11"	210'-11"	137'-7"	33'-0"	33'-0"	57'-2"	59'-3"	47'-2"	50'-3"
G	108'-6"	166'-6"	210'-0"	137'-0"	33'-0"	33'-0"	57'-0"	59'-0"	47'-0"	50'-0"
H	108'-5"	166'-4"	209'-8"	136'-9"	32'-11"	32'-11"	56'-11"	58'-11"	46'-11"	49'-11"
J	108'-3"	165'-11"	208'-10"	136'-2"	32'-10"	32'-10"	56'-9"	58'-8"	46'-8"	49'-8"
K	108'-1"	165'-7"	207'-11"	135'-6"	32'-10"	32'-10"	56'-7"	58'-5"	46'-6"	49'-5"
L	107'-11"	165'-2"	207'-1"	134'-11"	32'-9"	32'-10"	56'-6"	58'-3"	46'-3"	49'-2"
M	107'-9"	164'-10"	206'-3"	134'-4"	32'-8"	32'-9"	56'-4"	58'-0"	46'-1"	49'-0"
N	107'-7"	164'-5"	205'-4"	133'-9"	32'-7"	32'-9"	56'-2"	57'-9"	45'-10"	48'-9"
P	107'-6"	164'-1"	204'-6"	133'-2"	32'-6"	32'-8"	56'-0"	57'-6"	45'-8"	48'-6"

FLANGE DIMENSIONS										
GIRDER	L	M	N	P	Q	R	S	T	U	V
A thru D	13'-0"	11'-0"	21'-0"	18'-0"	17'-0"	16'-0"	30'-0"	15'-0"	17'-0"	19'-0"
E thru G	13'-0"	11'-0"	20'-0"	18'-0"	16'-0"	16'-0"	32'-0"	15'-0"	15'-0"	18'-0"
H thru K	10'-0"	9'-0"	19'-0"	16'-0"	16'-0"	16'-0"	36'-0"	14'-0"	13'-0"	16'-0"
L thru P	10'-0"	9'-0"	19'-0"	16'-0"	16'-0"	16'-0"	35'-0"	14'-0"	13'-0"	16'-0"

ROCKER DIMENSIONS										
ROCKER NO.	A	B	C	D	F	G	H	K	L	WEIGHT (lbs)
R-150M	3"	14"	3"	2 1/2"	1"	8 1/2"	14 1/2"	11 1/2"	22"	440
R-375	3 1/2"	20"	3 1/2"	3 1/2"	1 1/2"	12"	21 1/2"	14 1/2"	28"	1180
R-500	4"	22"	4"	4"	1 1/2"	12"	22 1/2"	16"	33"	1620



Notes:
The table of Rocker Dimensions applies to the standard details shown on Ohio Standard Drawing RB-1-55.
For details of R-150 rocker see Ohio Standard Drawing RB-1-55.
For details of support at Pier 2 see Sheet 10/20.

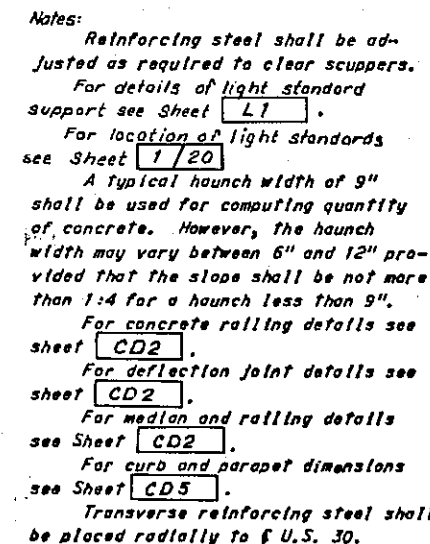
Notes:
Longitudinal stiffeners are located on inside face of girders only.
Longitudinal stiffeners shall be placed to clear field splices, bearing stiffeners, and intermediate stiffeners by 1".
(N) denotes North roadway and Girders A thru G.
(S) denotes South roadway and Girders H thru P.
All structural steel shown is A.S.T.M. A36 except as noted as A572 or A588.
For other notes see Sheet 12/20.
*T denotes area of tension in the bottom flange. The top flange in these areas is in compression.
*C denotes area of compression in bottom flange. CYN, see Sheet 0N1.
In lieu of ASTM A588 structural steel the Contractor may use ASTM A572 Grade 50 structural steel for plate thicknesses up to 2" inclusive.

H.N.T.B. BRIDGE NO. 18
HOWARD, NEEDLES, TAMMEN & BERGENDOFF
CONSULTING ENGINEERS
KANSAS CITY CLEVELAND NEW YORK

Revised 5-23-79 By RAS

GIRDER ELEVATION
U.S. 30 OVER 11TH STREET
BR. NO. STA-30-15.95 STA. 1017+06.75
SCALE: NONE STA. 1023+33.25
RELOCATED U.S. 30
CANTON STARK COUNTY OHIO

DRAWN: CAC
CHECKED: JAC
DATE: 6-20-79
DATE: 6-20-79
DATE: 6-20-79



For super-elevation transition scheme
see Sheet 17/20.
Minimum reinforcing bar lops shall
be 30 bar diameters.

H.N.T.B. BRIDGE NO. 18

Revised 2-29-79 By RAS

HOWARD, NEEDLES, TAMMEN & BERGENDOFF

CONSULTING ENGINEERS

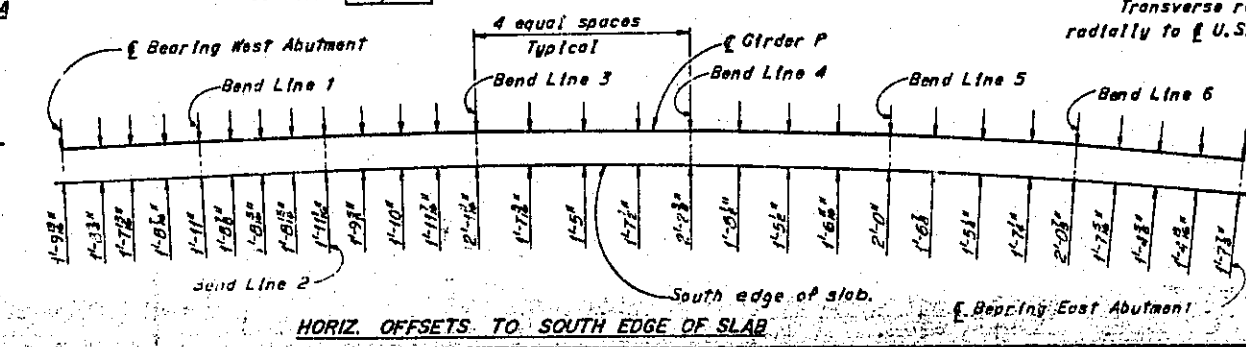
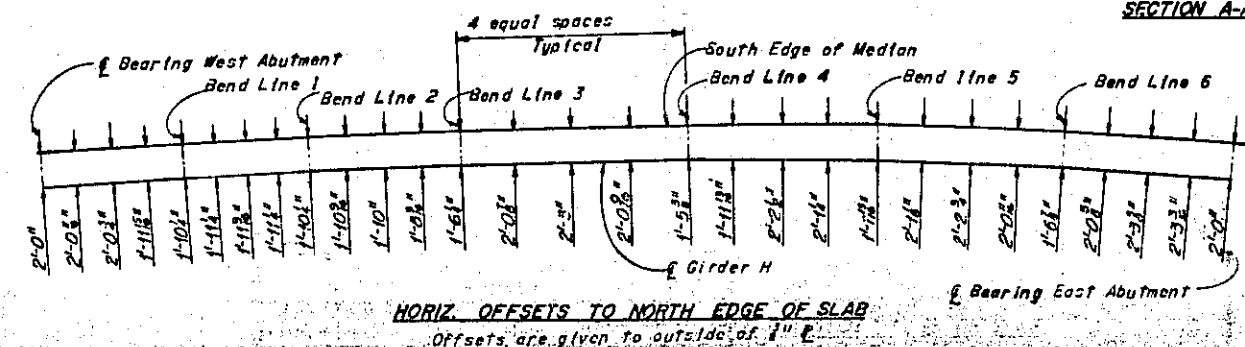
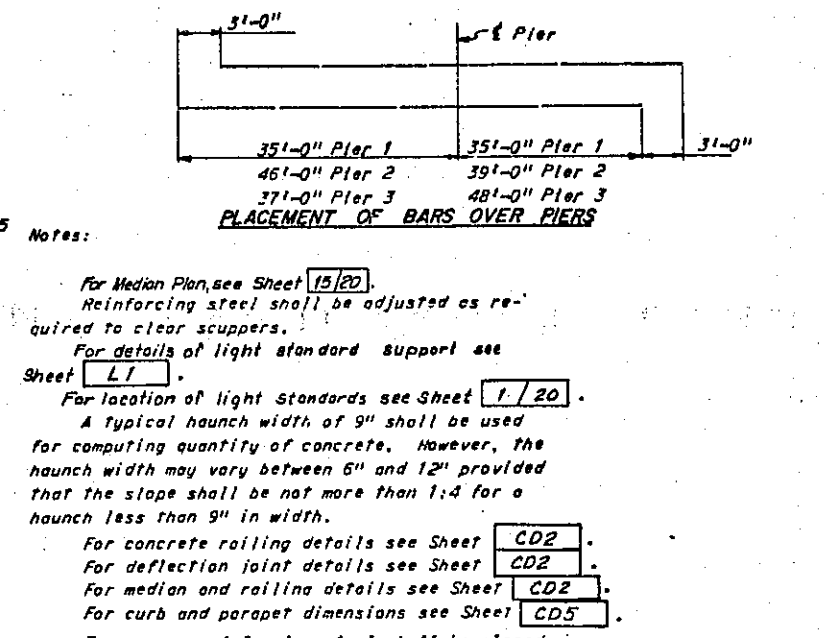
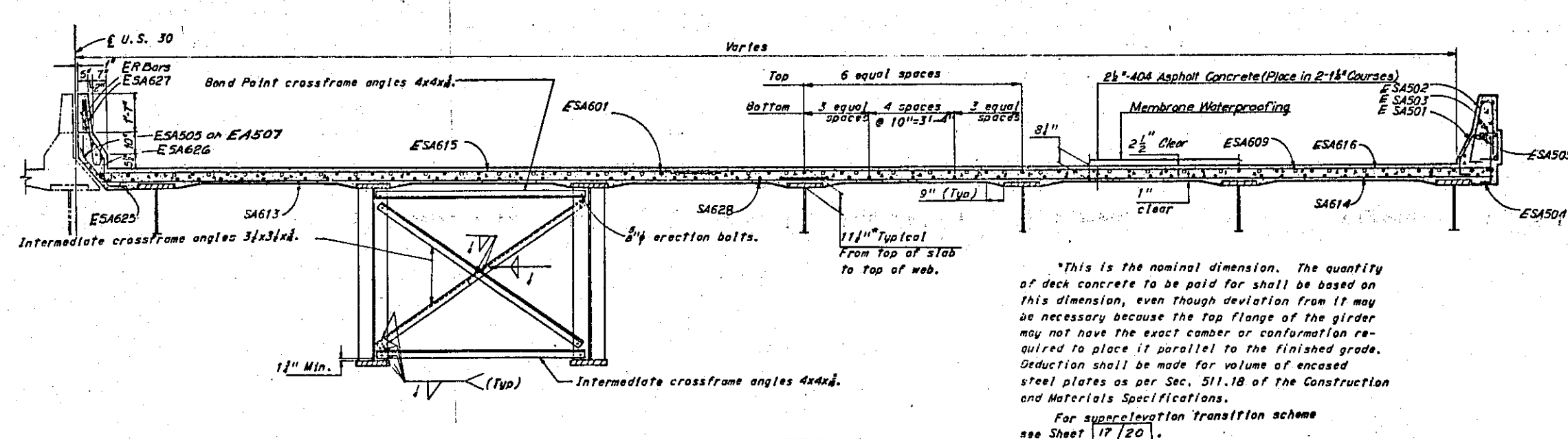
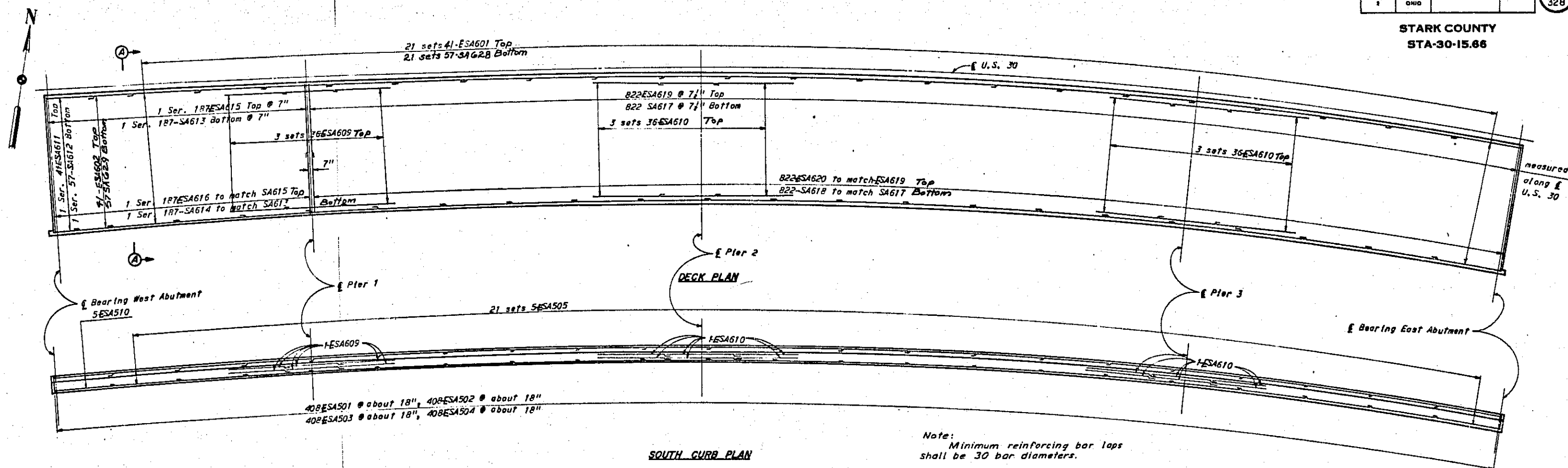
KANSAS CITY CLEVELAND NEW YORK

DECK PLAN

U.S. 30 OVER 11TH STREET
BR. NO. STA-30-15 95 STA. 1017+06.75
SCALE: NONE STA. 1023+33.25
RELOCATED U.S. 30

CANTON		STARK COUNTY		OHIO	
DRAWN BY DATE 7/28/79	TRACED DATE	CHECKED DATE 7-2-79	REVIEWED DATE 7-29-79	REVISED	
				186	

STARK COUNTY
STA-30-15.86



H.M.T.B. BRIDGE NO. 18

HOWARD, NEEDLES, TAMMEN & BERGENDOFF 16/20

CONSULTING ENGINEERS
KANSAS CITY CLEVELAND NEW YORK

DECK PLAN

U.S. 30 OVER 11TH STREET

SR. NO. STA-30-1595 STA. 1017+06.75
SCALE: NONE STA. 1023+33.25

RELOCATED U.S. 30

CANTON STARK COUNTY OHIO

DRAWN BY A. TRACED	CHECKED C.P.K.	REVIEWED M.J.K.	REVISION
DATE 7/18/70	DATE 7-9-70	DATE 7-28-71	16.65

North Gutterline

South Median Gutterline

South Gutterline

Profile Grade

Varies

9'-3"

36'-0"

6'-9"

2'-6"

6'-9"

5'-0"

Rounding

.0417'/'

.0156'/'

.0417'/'

.0156'/'

See Detail

STA. 1016 + 62.41

See

.0417'/'

.0417'/'

.0417'/'

.0417'/'

STA. 1018 + 68.52

See

.0710'/'

.0710'/'

.0710'/'

.0710'/'

STA. 1021 + 00.00

See

.0710'/'

.0710'/'

.0710'/'

.0710'/'

STA. 1024 + 00.00

1'-3"

4'-9"

2'-0"

E U.S. 50

Profile

Grade

South Median Gutterline

0.0156'/'

STA. 1016+62.41 TO STA. 1019+55.72

4'-0" Rounding

0.0521'/' Maximum

0.0156'/'

STA. 1019+65.72 TO STA. 1024+00.00

The diagram illustrates the camber of a bridge deck at different stages of construction. It shows a cross-section of the bridge deck with three main points of interest: the bearing abutment, the piers, and the full camber position. The camber is built up in stages, with the final full camber position being the sum of the camber at the bearing abutment and the camber at the piers.

These values are given on Line 5, Typical.

H.N.T.B. BRIDGE NO. 18		Revised 3-23-79 By RAS	
HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONSULTING ENGINEERS KANSAS CITY CLEVELAND NEW YORK			17/20
DEFLECTION AND CAMBER			
U.S. 30 OVER 11TH STREET			
BR NO STA-30-1595		STA. 1017+06.75	
SCALE: NONE		STA. 1023+33.25	
RELOCATED U.S. 30			
CANTON		STARK COUNTY OHIO	
DRAWN BY A. FREED	CHECKED CAC	REVIEWED M-C	REVISOR
DATE 6/8/70	DATE 7-8-70	DATE 7-8-70	1668

MARK	NUMBER	LENGTH	TYPE	DIMENSIONS			SERIES INCR.	WEIGHT POUNDS
				A	B	C		
EAST ABUTMENT								
AB421	98	3'-7"	105	1'-0"	1'-10"			215
AB501	84	5'-4"	105	1'-7"	5'-5"			731
AB502	84	7'-7"	104	6'-8 1/2"	1'-0"			685
AB503	84	7'-7"	105	2'-2 1/2"	3'-5"			565
AB504	4	3'-8"	152	2'-7"	3'-4"	7 1/2"		15
AB505	4	3'-7"	143	1'-1"	6"			15
AB506	13	11'-1"	109	3'-0"	2'-3"			150
AB507	10	12'-1"	109	3'-6"	2'-3"			126
AB508	6	9'-10"	117	6'-8"	4'-9"	1'-1 1/2"		62
AB509	2	12'-8"	117	0'-6"	6'-9"	1'-1 1/2"		40
AB510	6	8'-3"	108	1'-7"	5'-8"	4'-9"		52
AB511	3	11'-1"	108	1'-7"	9'-6"	5'-9"		35
AB516	10	5'-3"	Str					55
AB518	10	3'-3"	Str					34
AB519	15	5'-9"	114	3'-7"				106
AB520	20	1'-10"	105	6"	1'-0"			38
AB521	6	7'-8"	104	6'-3"	1'-7"			48
AB522	9	6'-3"	Str					59
AB523	7	9'-8"	104	8'-3"	1'-7"			30
AB524	1	8'-6"	Str					9
AB525	5	9'-3"	Str					53
AB526	7	7'-6"	Str					48
								83
AB530	23	24'-6"	Str					588
AB531	3	3'-6"	Str					11
AB532	1	4'-3"	Str					4
AB533	1	4'-9"	Str					5
AB534	1	5'-9"	Str					5
AB535	23	24'-6"	Str					588
AB536	6	20'-6"	Str					128
AB537	3	18'-9"	Str					59
AB538	3	22'-0"	104	20'-6 1/2"	1'-7"			69
AB539	3	14'-0"	104	12'-6 1/2"	1'-7"			44
AB540	6	10'-6"	Str					66
AB541	1	11'-9"						12
AB542	1	16'-9"	104	15'-3 1/2"	1'-7"			18
AB543	9	9'-6"	Str					89
AB544	3	16'-0"	104	14'-6 1/2"	1'-7"			50
AB545	23	29'-3"	Str					702
AB546	23	29'-6"	Str					708
AB547	1	4'-6"	Str					5
AB548	2	9'-6"	Str					20
AB549	2	6'-9"	Str					14
AB550	9	1'-0"	Str					9
AB551	2	19'-0"	Str					40
AB601	84	14'-5"	106	5'-5"	6'-8"	2'-8"		1820
AB602	108	19'-0"	105	9'-4"	1'-5"			3204
AB603	108	4'-11"	105	1'-11"	1'-5"			798
AB604	108	5'-11"	105	2'-8"	1'-1"	10"		960
AB605	5	5'-2"	115	3'-7"	6"	10"		39
AB606	5	18'-8"	105	8'-9"	1'-6"			140
AB607	5	7'-9"	Str					58
AB608	1	17'-8"	105	8'-9"	6"			26
AB609	13	5'-0"	106	8"	2'-9"	1'-11"		98
AB610	13	3'-5"	104	1'-11"	1'-8"			66
AB611	1	7'-5"	115	5'-9"	6"	10"		11
AB612	9	11'-0"	115	9'-3"	6"	10"		149
AB613	10	18'-8"	105	9'-0"	1'-0"			280
AB614	1	4'-6"	Str					7
AB701	1	4'-8"	115	3'-3 1/2"	8 1/2"	6"		10
AB702	1	4'-8"	115	3'-3 1/2"	8"	6"		10
AB703	1	5'-0"	115	3'-7"	7 1/2"	7"		10
AB704	1	5'-9"	115	4'-4"	8 1/2"	8"		12
AB705	1	6'-6"	115	5'-0"	6"	9"		13

MARK	NUMBER	LENGTH	TYPE	DIMENSIONS			SERIES INCR.	WEIGHT POUNDS
				A	B	C		
AB801	14	46'-0"	Str.					1720
AB802	7	24'-0"	Str.					448
AB803	2	19'-10"	104	17'-6"	2'-5"			108
AB804	3	14'-0"	Str.					112
AB805	2	16'-7"	104	14'-3"	2'-5"			84
AB806	2	14'-3"	Str.					76
AB807	1	17'-6"	Str.					47
AB808	3	10'-9"	Str.					89
DB801	73	5'-1"	151	1'-6"	2'-1"	2'-1"		931
							Total	17,866
SOUTHEAST WINGWALL								
WC501	14	31'-9"	Str.					464
WC502	2	13'-0"	Str.					27
WC503	3	4'-5"	115	2'-8"	9"			15
WC504	4	11'-6"	Str.					48
WC505	1	15'-0"	115	13'-0"	9"			16
WC506	2	3'-6"	Str.					7
WC507	5	4'-0"	Str.					21
WC508	2	11'-0"	104	10'-2"	1'-0"			23
WC509	2	7'-4"	104	6'-8"	1'-0"			15
WC510	1 Ser. 7	11'-9" to 10'-3"	Str.					50
WC511	1 Ser. 7	12'-0" to 13'-5"	115	10'-2" to 11'-8"	6"	10"	2'-8"	93
WC512	1 Ser. 8	7'-0" to 8'-11"	Str.				5"	66
WC513	1 Ser. 8	8'-11" to 11'-0"	115	7'-2" to 9'-2"	6"	10"	3'-8"	83
WC514	1 Ser. 8	8'-9" to 6'-3"	Str.				2'-4"	46
WC515	1 Ser. 8	6'-5" to 5'-3"	115	4'-8" to 4'-6"	6"	10"	2'-8"	62
WC516	4	4'-9"	115	2'-8"	9"			14
WC517	3	10'-5"	104	9'-7 1/2"	1'-0"			33
WC518	10	3'-3"	Str.					34
WC519	34	6'-9"	114	3'-7"				239
WC520	30	1'-10"	105	6"	1'-0"			75
WC521	7	9'-3"	104	8'-4 1/2"	1'-0"			60
WC522	1	5'-0"	Str.					5
WC523	2	22'-6"	Str.					41
WC524	10	23'-5"	Str.					245
WC525	1 Ser. 5	8'-10" to 11'-10"	121	2'-0" to 4'-6"	1'-2"		1'-3"	41
WC526	2	22'-0"	Str.					46
WC601	19	9'-7"	132					273
WC602	7	5'-2"	106	2'-11"	8"	11'-1"		53
WC603	7	4'-3"	118	2'-0"	9 1/2"	11'-8"		45
WC604	3	13'-6"	104	12'-8"	1'-0"			51
WC605	3	15'-0"	115	12'-7 1/2"	6"	10"		68
WC606	3	7'-1"	104	6'-1"	1'-0"			34
WC607	3	9'-5"	116	7'-1"	6"	10"		42
WC608	6	3'-6"	Str.					32
WC609	2	7'-4"	100	6'-0"				80
WC610	8	2'-6"	Str.					30
WC611	1	6'-0"	Str.					10
WC612	1	7'-9"	Str.					12
WC613	2	16'-6"	Str.					50
WC701	1	5'-7"	115	4'-2"	8 1/2"	6"		11
WC702	1	5'-7"	115	4'-3"	8"	6"		12
WC703	1	5'-9"	115	4'-4"	7 1/2"	7"		12
WC704	1	5'-9"	115	4'-4"	6 1/2"	8"		12
WC705	1	5'-9"	115	4'-4"	6"	9"		12
WC706	2	14'-0"	101	13'-2"				57
WC707	4	31'-9"	Str.					260
WC708	1	22'-0"	Str.					45
WC709	1	21'-0"	Str.					43
WC710	1	17'-9"	Str.					36
WC711	1	16'-9"	Str.					34
WC712	1	6'-0"	Str.					12
WC713	1	5'-0"	Str.					10
WC714	1	3'-6"	Str.					7
WC715	1	2'-5"	Str.					5
WC716	2	32'-0"	Str.					131
WC801	4	9'-11"	100	7'-9"				705

MARK	NUMBER	LENGTH	TYPE	DIMENSIONS			SERIES	WEIGHT INCR.	POUNDS
				A	B	C			
							Total	3,529	
EPOXY COATED REINFORCING STEEL									
ERS01	28	15'-3"	Str.					445	
ERS02	156	7'-0"	Str.					2600	
ERS03	48	6'-6"	Str.					325	
ERS04	56	6'-0"	Str.					350	
ERS05	56	14'-9"	Str.					1475	
ERS06	68	14'-6"	Str.					1027	
ERS07	20	14'-3"	Str.					298	
ERS08	20	14'-0"	Str.					292	
ERS09	44	15'-0"	Str.					688	
E	2	Light Pole Supports (Epoxy Coated)						240	
ESA501	834	3'-2"	143	9"	6"			2755	
ESA502	834	5'-7"	144					4857	
ESA503	834	2'-0"	105	7 1/2"	1'-0"			1740	
ESA504	834	2'-6"	104	7 1/2"	2'-0"			2175	
ESA505	325	30'-0"	Str.					10,170	
ESA506	5	11'-0"	Str.					57	
ESA507	5	24'-6"	Str.					128	
ESA510	5	13'-0"	Str.					68	
ESA601	1701	30'-0"	Str.					76,647	
ESA602	81	16'-3"	Str.					1977	
ESA603	1 Ser. 40	17'-5" to 32'-5"	101	16'-9" to 31'-9"			4 3/8	1497	
ESA607	1091	33'-5"	101	32'-9"				54800	
ESA608	1091	25'-8"	101	25'-0"				42100	
ESA609	228	24'-9"	Str.					8475	
ESA610	456	29'-9"	Str.					20400	
ESA611	1 Ser. 41	5'-6" to 18'-6"	Str.				3 7/8	739	
ESA615	1 Ser. 187	23'-8" to 24'-11"	101	23'-0" to 24'-3"			1 1/8"	6823	
ESA616	1 Ser. 187	34'-5" to 35'-6"	101	33'-9" to 34'-10"			1 1/8"	9815	
ESA619	822	25'-5"	101	24'-9"				31381	
ESA620	922	31'-9"	101	31'-1"				39200	
ESA621	1070	3'-3"	153	10"	1'-1"	1'-6"		5223	
ESA622	1070	2'-11"	119	1'-7"	10"	9 3/8"		4688	
ESA623	1070	2'-6"	Str.					4018	
ESA624	1070	3'-1"	119	2'-7 1/2"	7 1/2"	1 1/4"		4955	
ESA625	1070	4'-6"	151	2'-3"	1'-0 1/2"	1'-3 1/2"		7232	
ESA626	1070	3'-3"	154	1'-1"	1'-6"			5223	
ESA627	1070	3'-4"	101	2'-8"				5357	
						TOTAL	=	360,240	
DECK									
SA604	1 Ser. 56	16'-9" to 31'-9"	Str.				3 1/2"	2040	
SA605	1091	28'-3"	Str.					46,250	
SA606	1091	29'-6"	Str.					48,300	
SA612	1 Ser. 57	5'-6" to 18'-6"	Str.				2 3/8"	1227	
SA613	1 Ser. 187	28'-3" to 29'-6"	Str.				1 1/8"	8110	
SA614	1 Ser. 187	27'-9" to 29'-6"	Str.				1/8"	8040	
SA617	822	28'-0"	Str.					34570	
SA618	822	28'-0"	Str.					34570	
SA628	2373	30'-0"	Str.					106,920	
SA629	113	16'-3"	Str.					2758	
						TOTAL	=	292,790	
REINFORCING STEEL SAMPLES									
Refer to CMS Sections 106.03, 700, 709.01 through 709.05 and 709.08. Sufficient additional reinforcing steel shall be provided for sampling. Random samples shall be replaced in the structures by the additional steel, spliced in accordance with 509.08.									