# STRUCTURE ESTIMATED QUANTITIES 

Bridge No. SUM-77-0927R<br>Ramp C2 over I.R. 77 NB

## SUM-77/277/224-VARIOUS

PID No. 106002

Summit County, Ohio

## Prepared For:

The Ohio Department of Transportation District 4

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January 18, 2021

| Project: | Bridge No. SUM-77-0927R | Design: | RHC |
| :--- | :--- | :--- | :--- |
| Subject: | Stage 3 Estimated Quantities | Check: | DGN |
| Date: | $1 / 18 / 2021$ |  |  |

1/18/2021
ITEM 202 - STRUCTURE REMOVED, OVER 20 FOOT SPAN
area $=$
7010 sf
unit cost $=\quad \$ 18.00$ per sf
Lump sum $=\$ 126,180$

ITEM 202 - APPROACH SLAB REMOVED

| length $=$ | 25 ft |  |  |
| :--- | :---: | :--- | :--- |
| width $=$ | 28 ft |  |  |
| No. of approach slabs $=$ | 2 |  |  |
|  |  | Total $=\quad$ | $\underline{156}$ sy |

## ITEM 503-UNCLASSIFIED EXCAVATION

| Rear Abutment: |  |
| :--- | ---: |
| length = | 49.33 ft |
| width $=$ | 7.75 ft |
| depth $=$ | 15 ft |


| Forward Abutment: |  |
| :--- | :--- |
| length $=$ | ft |
| width $=$ | ft |
| depth $=$ | ft |

Abutment Subtotal $=\quad 212.39$ cy

| Piers: |  |
| :--- | :---: |
| length $=$ | 24.5 ft |
| width $=$ | 24.5 ft |
| depth = | 6.5 ft |
| no. of ftgs per pier = | 1 |
| no. of piers = | 2 |
| Pier Subtotal = | $\mathbf{2 8 9 . 0 1} \mathbf{~ c y ~}$ |
| Total = | $\underline{\mathbf{5 0 2} \mathbf{~ c y ~}}$ |

ITEM 505 - PILE DRIVING EQUIPMENT MOBILIZATION

Lump sum $=\$ 15,000$

## ITEM 507-STEEL PILES HP10x42, FURNISHED

Frwd. Abutment:
length $=\quad 40 \mathrm{ft}$
no. of piles $=$
10

## Total $=\quad \underline{400} \mathrm{ft}$

## ITEM 507-STEEL PILES HP10x42, DRIVEN

Frwd. Abutment:
length =
35 ft
no. of piles =

10
Total $=\quad \underline{350} \mathrm{ft}$

ITEM 509 - EPOXY COATED REINFORCING STEEL

| Abutments $=$ | 13,275 | lbs |
| :--- | :--- | :--- |
| Pier $=$ | 97,512 | lbs |
| Parapets $=$ | 19,903 | lbs |
| Slab $=$ | 92,954 | lbs |

ITEM 509 - NO. 4 GFRP DEFORMED BARS
SBR-1-20 Parapets $=\quad 12,353 \quad \mathrm{lbs}$
Total $=\quad 12,353 \mathrm{lbs}$

ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK
BRIDGE
Deck:
thickness $=\quad 8.5$ in
edge of deck
to bridge limits $=\quad 0.75 \mathrm{ft}$
total sum of spans $=\quad 343.25 \mathrm{ft}$
total length $=\quad 344.75 \mathrm{ft}$
O/O of deck width $=\quad 29.33 \mathrm{ft}$
$\begin{array}{ll}\text { Deck Volume }= & 265 \text { cy }\end{array}$
Haunch:
t/flange width $=\quad 22$ in
t/deck to t/web = $\quad 13$ in
haunch thick. $=\quad 2.7$ in
t/flange thick. (avg) $=\quad 1.8$ in
total no. of beams = $\quad 4$

| Over C.I.P. integral pier caps: |  |
| :--- | :---: |
| pier cap width $=$ | 7.00 ft. |
| length between beams $=$ | 6.17 ft. |
| Number of Pier caps $=$ | 2 |
| Pier Cap Haunch Subtotal $=$ | $\mathbf{3 ~ c y}$ |

Pier Cap Haunch Subtotal =
3 cy
Haunch Volume $=$
24 cy

## Cantilever:

cantilever length (right) $=\quad 2.66 \mathrm{ft}$ cantilever length (left) $=\quad 2.66 \mathrm{ft}$ Cantilever Volume = 17 cy

## Total $=$

306 cy

## ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET)

| area (left $)=$ | 4.08 sf |
| :--- | :---: |
| area (right $=$ | 4.08 sf |
| length $($ left $)=$ | 425.13 ft (Includes App Slab Parapets) |
| length $($ rightt $)=$ | 402.91 ft (Includes App Slab Parapets) |
|  |  |
|  |  |
|  | Total $=$ |
|  | $\underline{\mathbf{1 2 6}} \mathbf{~ c y}$ |

## ITEM 511 - CLASS QC4 MASS CONCRETE, SUBSTRUCTURE (PIER COLUMNS

| Column area $=$ | 28.27 sf |
| :--- | ---: |
| Column diameter $=$ | 6.00 ft |
| Total column height Pier 1= | 22.07 ft |
| Total column height Pier 2= | 20.86 ft |

## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING

Skew angle =

## Rear Abutment:

## Beam Seat

| beam seat length $=$ | 29.00 ft |
| :--- | ---: |
| average beam seat height $=$ | 5.00 ft |
| beam seat width $=$ | 3.00 ft |
| Beam Seat Volume $=$ | 688.75 cf |
|  |  |
| Backwall | 7.05 ft |
| average backwall height | 1.75 ft |
| backwall thickness = | 1.42 ft |
| approach slab thickness = | 29.00 ft |
| backwall length $=$ | 337.20 cf |

## Wingwalls

Left
thickness $=\quad 1.75 \mathrm{ft}$
L1 =
10.20 ft
7.25 ft

L2 =
H1 =
H2 =
12.50 ft
9.00 ft

Left wingwall area =
Left Wingwall Volume =
Right
thickness = $\quad 1.75 \mathrm{ft}$
L1 =
L2 $=$
H1 =
$\mathrm{H} 2=$
Right wingwall area =
Right Wingwall Volume =

## Rear Abutment Subtotal

114.8125 sf 200.92188 cf
10.14 ft
7.25 ft
11.30 ft
7.80 ft
101.8945 sf 178.31538 cf

Forward Abutment:
Beam Seat
beam seat length = 29.00 ft
average beam seat height $=\quad 4.21 \mathrm{ft}$
beam seat width $=$
3.00 ft

Beam Seat Volume =

| Backwall |  |
| :--- | ---: |
| average backwall height | 6.97 ft |
| backwall thickness = | 1.75 ft |
| approach slab thickness = | 1.42 ft |
| backwall length = | 29.00 ft |
| Back Wall Volume = | 333.14 cf |
| Wingwalls |  |
| Left |  |
| thickness = |  |
| L1 = | 1.75 ft |
| L2 = | 12.65 ft |
| H1 = | 8.71 ft |
| H2 = | 11.40 ft |
| Left wingwall area = | 7.00 ft |
| Left Wingwall Volume = | 125.048 sf |
| Right | 218.834 cf |
| thickness = |  |
| L1 = |  |
| L2 = | ft |
| H1 = | ft |
| H2 = | ft |
| Right wingwall area = | ft |
| Right Wingwall Volume = | ft |
| Forward Abutment Subtotal = | 0 sf |
|  | 0 cf |
|  |  |

## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, FOOTING

| Pier: |  |
| :--- | :---: |
| thickness = | 4.75 ft |
| width $=$ | 22.5 ft |
| length $=$ | 22.5 ft |
| no. of footing $=$ | 2 |
| Footing Volume $=$ | 178.13 cy |


| Rear Abut. Footing |  |
| :--- | ---: |
| footing thickness = | 3.00 ft |
| footing width $=$ | 5.75 ft |
| footing length $=$ | 35.33 ft |
| Footing Volume $=$ | 22.57 cy |

Frwd. Abut. Footing
footing thickness $=\quad 3.00 \mathrm{ft}$
footing width $=\quad 7.25 \mathrm{ft}$
footing length $=\quad 33.67 \mathrm{ft}$
Footing Volume $=\quad 27.12$ cy

Total $=\quad \underline{228}$ cy
ITEM 511 - CLASS QC4 CONCRETE MISC.: INTEGRAL POST-TENSIONED PIER CAPS
Pier Caps:

| pier cap width $=$ | 7.00 ft |
| :--- | ---: |
| pier cap height $=$ | 5.17 ft |
| pier cap length $=$ | 29.33 ft |
| no. of pier caps $=$ | 2 |

Total $=\quad \underline{79} \mathrm{cy}$

| Bridge deck |  |
| :---: | :---: |
| Left |  |
| perimeter $=$ | 9.81 ft |
| length $=$ | 425.13 ft |
| Right |  |
| perimeter $=$ | 9.81 ft |
| length = | 402.91 ft |
| Bridge Deck Subtotal = | 8123 sf |
| Abutments |  |
| Rear Abutment: |  |
| backwall height = | 7.05 ft |
| beam seat width = | 3.00 ft |
| exposed breastwall height = | 1.00 ft |
| backwall/breastwall length = | 29.00 ft |
| Abutment total = | 320.45 |
| Rear left wingwall |  |
| exposed height behind wall = | $\begin{aligned} & 5.00 \mathrm{ft} \\ & 0.50 \mathrm{ft} \end{aligned}$ |
| front exposed area = | 63.81 sf |
| top of wall = | 15.80 sf |
| back of wall = | 2.29 sf |
| Rear left total = | 81.90 sf |
| Rear right wingwall |  |
| depth of fill in front of wall $=$ | 5.00 ft |
| exposed height behind wall = | 0.50 ft |
| front exposed area = | 51.19 sf |
| top of wall = | 15.63 sf |
| back of wall = | 2.27 sf |
| Rear right total $=$ | 69.09 sf |
| Rear Abutment Subtotal $=$ | 471 sf |


| Forward Abutment: |  |
| :--- | ---: |
| backwall height $=$ | 6.97 ft |
| beam seat width $=$ | 3.00 ft |
| exposed breastwall height $=$ | 4.21 ft |
| backwall/breastwall length $=$ | 29.00 ft |
| Abutment total $=$ | 411.22 |


| Forward left wingwall |  |
| :--- | ---: |
| depth of fill in front of wall = | 2.00 ft |
| exposed height behind wall = | 0.50 ft |
| front exposed area = | 99.75 sf |
| top of wall $=$ | 19.86 sf |
| back of wall $=$ | 2.95 sf |
| Forward left total $=$ | 122.56 sf |


| Forward right wingwall |  |
| :--- | :--- |
| depth of fill in front of wall = | 0.00 ft |
| exposed height behind wall $=$ | 0.00 ft |
| front exposed area = | 0.00 sf |
| top of wall = | 0.00 sf |
| back of wall = | 0.00 sf |
| Forward right total = | 0.00 sf |
| Forward Abutment Subtotal $=$ | 534 sf |
| Abutment Subtotal $=$ | 1005 sf |


| Piers |  |
| :--- | ---: |
| Caps |  |
| length $=$ |  |
| thickness $=$ | 29.33 |
| height $=$ | 7.00 |
| Cap area $=$ | 5.17 |
| Columns | 552.6879 |
| diameter $=$ | 6.00 ft |
| average exposed height $=$ | 18.00 ft |
| no. of columns = | 1 |
| Column area = | 339.292 sf |
| No. of Piers = | 2 |
| Pier Subtotal $=$ | 1784 sf |

Total $=\quad 1,213$ sy

## ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL 5, AS PER PLAN

## BRIDGE

Girders Properties

|  | Unit Weight (lb./ft.) | Dimension Input (for plate girders) (all units are inches) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section ID | 267.5434 | 18 | 1 | 62 | 0.6875 | 18 | 1 |  |
| Section 1 | 519.349 | 22 | 2.5 | 62 | 0.6875 | 22 | 2.5 |  |
| Section 2 | 519.349 | 22 | 2.5 | 62 | 0.6875 | 22 | 2.5 |  |
| Section 3 | 350.9115 | 22 | 1 | 62 | 0.6875 | 22 | 1.75 |  |
| Section 4 |  |  | tfl. top | Dweb | tweb | Bfl. bot | top |  |

Section Lengths

| Section ID | Unit Weight (lb./ft.) | Girder 1 | Girder 2 | Girder 3 | Girder 4 | Total Lengths |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section 1 | 267.54 | 90.00 | 88.83 | 87.66 | 86.5 | 352.99 |
| Section 2 | 519.35 | 126.01 | 123.52 | 121.3 | 119 | 489.83 |
| Section 3 | 519.35 | 36.42 | 35.92 | 35.42 | 35 | 142.76 |
| Section 4 | 350.91 | 108.41 | 106.28 | 104.14 | 102 | 420.83 |

Detail factor $=\quad 1.010$

## Girder Subtotal =

576,400 lbs

## Splices:

Splice 1

| Splice 1 |
| :--- |
| Top Flange |
| outside plates = |
| inside plates $=$ |
| fill plates = |
| Bottom Flange |
| outside plates = |
| inside plates = |
| fill plates = |
| Web |
| plates $=$ |


| \# of plates | length (in) | width (in) | thick (in) |
| :---: | :---: | :---: | :---: |
| 1 | 36.75 | 18 | 0.625 |
| 2 | 36.75 | 8 | 0.75 |
| 1 | 18 | 18 | 1.5 |
|  |  |  |  |
| 1 | 36.75 | 18 | 0.625 |
| 2 | 36.75 | 8 | 0.75 |
| 1 | 18.5 | 18 | 1.5 |
|  |  |  |  |
| 2 | 56.5 | 15.75 | 0.375 |

Plate weight/splice =
953 lbs
Splice 2

| Top Flange |
| :--- |
| outside plates = |
| inside plates = |
| fill plates = |
| Bottom Flange |
| outside plates = |
| inside plates = |
| fill plates = |
| Web |
| plates $=$ |


| \# of plates | length (in) | width (in) | thick (in) |
| :---: | :---: | :---: | :---: |
| 1 | 43.75 | 22 | 0.625 |
| 2 | 43.75 | 10 | 0.75 |
| 1 | 21.5 | 22 | 1.5 |
|  |  |  |  |
| 1 | 64.75 | 22 | 1 |
| 2 | 64.75 | 10 | 1.125 |
| 1 | 32.375 | 22 | 0.75 |
|  |  |  |  |
| 2 | 55.25 | 15.75 | 0.375 |

Plate weight/splice =
1711 lbs

| Splice Bolts |
| :--- |
| Splice 1 |
| Top Flange $=$ |
| Bot. Flange $=$ |
| Web $=$ |



| Bolt + Washer weight |  |
| :--- | :---: |
| Top Flange $=$ | 64 lbs |
| Bot. Flange $=$ | 64 lbs |
| Web $=$ | 70 lbs |
|  |  |
| Total weight/splice = | 1151 lbs |
| No. of splices = | 8 |
|  |  |
| Splice 2 | \# of bolts len |
| Top Flange $=$ | 48 |
| Bot. Flange $=$ | 72 |
| Web $=$ | 44 |
|  | * from steel ma |
|  | washer weigh |
| Bolt + Washer weight |  |
| Top Flange $=$ | 76 lbs |
| Bot. Flange $=$ | 715 lbs |
| Web $=$ | 70 lbs |
| Total weight/splice $=$ | 1973 lbs |
| No. of splices $=$ | 8 |

## Splice + Bolts Subtotal = <br> 24,988 Ibs

## Intermediate Crossframes:

| Length of Diagonals = | 7.00 ft (weighted avg.) |
| :---: | :---: |
| No. of Diagonals = | 2 |
| Length of Horiz. = | 6.50 ft (weighted avg.) |
| No. of Horiz. = | 2 |
| Angle weight / ft. = | $15.00 \mathrm{lbs} / \mathrm{ft}$ |
| Crossframe weight $=$ | 405 lbs ==> per x-frame assembly |
| x-frame stiffeners? | y y or n |
| Length = | 62.000 in ==> web depth |
| Width = | 8.000 in |
| Thickness = | 0.500 in |
| Stiffener weight $=$ | 141 lbs ==> per x -frame assembly |
| x-frame gusset plates? | y y or n |
| Length = | 17.500 in ==> web depth |
| Width = | 15.000 in |
| Thickness = | 0.500 in |
| Gusset plate weight $=$ | 149 lbs ==> per x-frame assembly |
| Total Intermediate Crossframe |  |
| Assembly Weight = | 695 lbs. ==> per x-frame assembly |
| No. of assemblies = | 72 |
| Detail Factor = | 1.05 |
| Intermediate Crossframe Subtotal $=$ | 52506 lbs |

## End Crossframes:

| Length of Diagonals = | 6.50 ft (weighted avg.) |
| :---: | :---: |
| No. of Diagonals = | 2 |
| Length of Horiz. = | 8.00 ft (weighted avg.) |
| No. of Horiz. = | , |
| Angle weight / ft. = | $9.80 \mathrm{lbs} / \mathrm{ft}$ |
| End crossframe weight = | 206 lbs. ==> per x-frame assembly |
| End x -frame stiffeners (bearing stiffeners)? | y y or n |
| Length = | 62.000 in ==> web depth |
| Width = | 8.500 in |
| Thickness = | 1.000 in |
| Stiffener weight $=$ | 299 lbs ==> per x-frame assembly |
| Total x-frame End |  |
| Assembly Weight = | 505 lbs ==> per x-frame assembly |
| No. of assemblies = | 8 |
| Detail Factor = | 1.10 |
| End Crossframe Subtotal $=$ | 4441 lbs |
| Crossframe Subtotal $=$ | 56,947 lbs |
|  | Total $=6 \underline{65,400} \mathrm{lbs}$ |

## ITEM 513 - WELDED STUD SHEAR CONNECTORS

| No. rows per beam: |  |
| ---: | ---: |
| Girder 1 | 391 |
| Girder 2 | 449 |
| Girder 3 | 443 |
| Girder 4 | 384 |
| No. per row = | 3 |
|  |  |
| Total = | $\underline{\mathbf{5 , 0 0 1}} \mathbf{~ e a ~}$ |

ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT
ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, FINISH COAT

| Average Flange width $=$ | 21.00 in |
| :--- | ---: |
| Total Girder Depth $=$ | 66.00 in |
| Total beam Length $=$ | 1406.41 |
| Detail Factor $=$ | 1.20 |

Total $=\quad \underline{27,500} \mathbf{~ s f}$

## ITEM 514 - FINAL INSPECTION REPAIR

Length =

$$
\begin{array}{r}
1406.41 \mathrm{ft} \\
4 \mathrm{ea}
\end{array}
$$

No. Girders =
No. Crossframes =
Total $=\quad \underline{42}$ ea

## ITEM 516 -ARMORLESS PREFORMED JOINT SEAL

Forward approach slab width $=\quad 29.00 \mathrm{ft}$
Total $=\quad \underline{29} \mathrm{ft}$

ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL

| RA Length $=$ | 28.50 ft |
| :--- | :--- |
| FA Length $=$ | 28.50 ft |

Total $=\quad \underline{57} \mathrm{ft}$

## ITEM 518 - POROUS BACKFILL WITH GEOTEXTILE FABRIC

| Rear Abutment: |  |
| :--- | ---: |
| Thickness = | 2.00 ft |
| Height = | 10.63 ft |
| app. Slab thickness = | 1.42 ft |
| length = | 29.00 ft |
| Left wingwall = | 104.61 sf |
| Right wingwall = | 91.75 sf |
|  |  |
| Rear Abutment Subtotal = | 927 cf |
|  |  |
| Forward Abutment: | 2.00 ft |
| Thickness = | 9.76 ft |
| Height = | 1.42 ft |
| app. Slab thickness = | 29.00 ft |
| length = | 112.40 sf |
| Left wingwall = | 0.00 sf |
| Right wingwall = |  |
|  | 709 cf |
| Frwd. Abutment Subtotal = |  |

Total $=\quad \underline{61} \mathrm{cy}$

## ITEM 518-6" PERFORATED CORRUGATED PLASTIC PIPE

## Rear Abutment:

length $=\quad 49.34 \mathrm{ft}$
Forward Abutment:
length =
41.65 ft

Total $=\quad \underline{91} \mathrm{ft}$
ITEM 518-6" NON-PERFORATED CORRUGATED PLASTIC PIPE INCLUDING SPECIALS

Forward Abutment:
length $=\quad 10.00 \mathrm{ft}$
Forward Abutment: length $=\quad 0.00 \mathrm{ft}$
Total $=\quad \underline{10} \mathrm{ft}$

## ITEM 524 - DRILLED SHAFTS, 42" DIAMETER, INTO BEDROCK WITH QC/QA, AS PER PLAN

```
Rear Abutment:
length = 6 ft
no. of shafts =
    3
    Total= 
```


## ITEM 524 - DRILLED SHAFTS, 48" DIAMETER, ABOVE BEDROCK WITH QC/QA, AS PER PLAN

Rear Abutment:
length $=\quad 11.2 \mathrm{ft}$
no. of shafts =
3

Total $=\quad \underline{34} \mathrm{ft}$

ITEM 524 - DRILLED SHAFTS, 48" DIAMETER, INTO BEDROCK WITH QC/QA, AS PER PLAN

## Pier 1

length $=\quad 7 \mathrm{ft}$
no. of shafts $=\quad 4$

Pier 2
length $=\quad 7 \mathrm{ft}$
no. of shafts $=\quad 4$

Total $=\quad \underline{56} \mathrm{ft}$

ITEM 524 - DRILLED SHAFTS, 54" DIAMETER, ABOVE BEDROCK WITH QC/QA, AS PER PLAN

Pier 1
length =
no. of shafts =

Pier 2
length = no. of shafts $=$

27 ft
4
27.3 ft

4

Total $=$
218 ft

## ITEM 526 - REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=17")

| Rear Approach Slab: |  |
| :--- | ---: |
| Width = | 29 ft |
| Length $=$ | 30 ft |
|  |  |
| Frwd. Approach Slab: |  |
| Width $=$ | 29 ft |
| Length $=$ | 30 ft |

ITEM 526-TYPE A INSTALLATION

Rear Approach Slab:
26 ft
Total $=\quad \underline{26} \mathrm{ft}$

## ITEM 526 - TYPE C INSTALLATION

Forward Approach Slab: length =
length =

29 ft
Total $=\quad \underline{29} \mathrm{ft}$

## ITEM SPECIAL - STRUCTURES: TEMPORARY SUPPORT OF STEEL GIRDERS

Temporary Bents:
$\begin{array}{rr}\text { Cost per temporary bent }= & \$ 20,000.00 \\ \text { No. of temporary bents }= & 5\end{array}$

## ITEM 846 - POLYMER MODIFIED EXPANSION JOINT SYSTEM

Rear approach slab = depth $=$ width $=$
26.00 ft

2 in
20 in
Total $=$

## ITEM 855 - POST-TENSIONING STRAND TENDON

Unit wieght per strand= No. strands of per tendon $=$ Tendon 1 length = Tendon 2 length = No. of Tendon $1=$ No. of Tendon $2=$
$0.74 \mathrm{lbs} . / \mathrm{ft}$
19.00 each
27.9 ft
27.4 ft

6 each
6 each

Total $=\quad \underline{4665} \mathrm{lbs}$.

ITEM 869 - HIGH LOAD MULIT-ROTATIONAL (HLMR) BEARINGS, AS PER PLAN
Rear abutment $=$
4 each
Forward abutment $=$
4 each

Total $=\quad \underline{8}$ each

