STRUCTURE ESTIMATED QUANTITIES

Bridge No. SUM-77-0927R

Ramp C2 over I.R. 77 NB

SUM-77/277/224-VARIOUS PID No. 106002

Summit County, Ohio

Prepared For:

The Ohio Department of Transportation District 4



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January 18, 2021

Project:	Bridge No. SUM-77-0927R	Design:	RHC
Subject:	Stage 3 Estimated Quantities	Check:	DGN
Date:	1/18/2021		

ITEM 202 - STRUCTURE REMOVED, OVER 20 FOOT SPAN

area =	7010	sf
unit cost =	\$18.00	per sf

Lump sum = <u>\$126,180</u>

ITEM 202 - APPROACH SLAB REMOVED

length =	25	ft	
width =	28	ft	
No. of approach slabs =	2		
		Total =	<u>156</u> sy

ITEM 503 - UNCLASSIFIED EXCAVATION

Pier Subtotal =	289.01 cy
Piers: length = width = depth = no. of ftgs per pier = no. of piers =	24.5 ft 24.5 ft 6.5 ft 1 2
Abutment Subtotal =	212.39 су
Forward Abutment: length = ff width = ff depth = ff	t t
Rear Abutment: length = 49.33 ft width = 7.75 ft depth = 15 ft	t t

ITEM 505 - PILE DRIVING EQUIPMENT MOBILIZATION

Lump sum = <u>\$15,000</u>

ITEM 507 - STEEL PILES HP10x42, FURNISHED

Frwd. Abutment: lenath =		40 ft	
no. of piles =		10	
		Total =	<u>400</u> ft
ITEM 507 - STEEL PILES HP10x42, D	DRIVEN		
Frwd. Abutment: length =		35 ft	
no. of piles =		10	
		Total =	<u>350</u> ft
ITEM 509 - EPOXY COATED REINFO	RCING STEEL		
Abutments =	13,275	lbs	
Parapets =	19,903	lbs	
Slab=	92,954	lbs	
		Total =	223 644 lbs
		lotui	<u></u>
TEM 509 - NO. 4 GERP DEFORMED	BARS		
SBR-1-20 Parapets =	12,353	lbs	
		Total =	<u>12,353</u> lbs
ITEM 511 - CLASS QC2 CONCRETE	WITH QC/QA, BRIE	DGE DECK	
BRIDGE Dock:			
thickness =		8.5 in	
edge of deck			
to bridge limits =		0.75 ft	
total sum of spans =		343.25 ft	
Ω/Ω of deck width =		29.33 ft	
Deck Volume =		265 cy	,
		-	
Haunch:		00 in	
t/deck to t/web =		22 III 13 in	
haunch thick. =		2.7 in	
t/flange thick. (avg) =		1.8 in	
total no. of beams =		4	
Over C.I.P. integral pier cap	os:	7 00 7	
pier cap width = length between beams =		7.00 ft.	
Number of Pier caps =		2	
Pier Cap Haunch Subtota	I =	3 су	1
Haunch Volume =			
Cantilever:			2 60 #
canulever length (right) = cantilever length (left) =			2.00 IT 2.66 ft
Cantilever Volume =			17 cy
			-
Total =		<u>306</u> cy	1

24 cy

ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET)

area (left) =	4.08	sf
area (right) =	4.08	sf
length (left) =	425.13	ft (Includes App Slab Parapets)
length (rightt) =	402.91	ft (Includes App Slab Parapets)

Total = <u>126</u> cy

ITEM 511 - CLASS QC4 MASS CONCRETE, SUBSTRUCTURE (PIER COLUMNS)

	Total =	<u>45</u> су
Total column height Pier 2=	20.86 ft	
Total column height Pier 1=	22.07 ft	
Column diameter =	6.00 ft	
Column area =	28.27 sf	

.

ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING

Skew angle =	0.00 degrees	
Rear Abutment:		
<u>Beam Seat</u>		
beam seat length =	29.00 ft	
average beam seat height =	5.00 ft	
beam seat width =	3.00 ft	
Beam Seat Volume =	688.75 cf	
Destaur		
Backwall	7.05 #	
average backwall height	7.05 IL	14
backwall thickness =	1./5 IL	← <u>LI</u>
approach siab thickness =	1.42 ft	
backwall length =	29.00 ft	L2
Back Wall Volume =	337.20 ct	
Wingwalls		
l oft		Area 2 Area 1
thickness -	1 75 ft	▲ <u></u> ,
	10.20 ft	
12=	7 25 ft	H2 Area 3
LZ - H1 -	12 50 ft	
H2 -	0.00 ft	
l iz -	114 9125 of	•
Left Wingwall Maluma –	114.0125 Si 200.02189 of	
Len Wingwaii Volume =	200.92188 CI	
Right		
thickness =	1.75 ft	
L1 =	10.14 ft	
L2 =	7.25 ft	
H1 =	11.30 ft	
H2 =	7.80 ft	
Right wingwall area =	101.8945 sf	
Right Wingwall Volume =	178.31538 cf	
Rear Abutment Subtotal	52 cv	
Forward Abutment:		
Beam Seat		
beam seat length =	29.00 ft	
average beam seat height =	4.21 ft	
beam seat width =	3.00 ft	
Beam Seat Volume =	579.93 cf	

<u>Backwall</u>	
average backwall height	6.97 ft
backwall thickness =	1.75 ft
approach slab thickness =	1.42 ft
backwall length =	29.00 ft
Back Wall Volume =	333.14 cf
14/5	
<u>wingwaiis</u>	
Left	4 75 6
thickness =	1.75 ft
L1 =	12.65 ft
L2 =	8./1 ft
H1 =	11.40 ft
H2 =	7.00 ft
Left wingwall area =	125.048 st
Left Wingwall Volume =	218.834 cf
Right	
thickness =	ft
L1 =	ft
L2 =	ft
H1 =	ft
H2 =	ft
Right wingwall area =	0 sf
Right Wingwall Volume =	0 cf
Forward Abutment Subtotal =	42 cy
Total =	<u>94</u> cy

ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, FOOTING

Pier:		L.	
thickness =	4.75	ft	
width =	22.5	ft	
length =	22.5	ft	
no. of footing =	2		
Footing Volume =	178.13	су	
Rear Abut. Footing			
footing thickness =		3.00 ft	t
footing width =		5.75 ft	t
footing length =		35.33 ft	t
Footing Volume =		22.57 c	зy
Frwd. Abut. Footing			
footing thickness =		3.00 ft	t
footing width =		7.25 ft	t
footing length =		33.67 ft	t
Footing Volume =		27.12 c	зy
C C			•

Total = 228 cy

ITEM 511 - CLASS QC4 CONCRETE MISC .: INTEGRAL POST-TENSIONED PIER CAPS

7.00	ft
5.17	ft
29.33	ft
2	
Total =	<u>79</u> су
	7.00 5.17 29.33 2 Total =

ITEM 512 - SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)

<u>Bridge deck</u>	
perimeter =	9 81 ft
length =	425.13 ft
Right	
perimeter =	9.81 ft
length =	402.91 ft
Bridge Deck Subtotal =	8123 sf
<u>Abutments</u>	
Rear Abutment:	
backwall height =	7.05 ft
beam seat width =	3.00 ft
exposed breastwall height =	1.00 ft
backwall/breastwall length =	29.00 π
Adulment Iolai =	320.45
Rear left wingwall	
denth of fill in front of wall =	5.00 ft
exposed height behind wall =	0.50 ft
front exposed area =	63.81 sf
top of wall =	15.80 sf
back of wall =	2.29 sf
Rear left total =	81.90 sf
Rear right wingwall	
depth of fill in front of wall =	5.00 ft
exposed height behind wall =	0.50 ft
front exposed area =	51.19 sf
top of wall =	15.63 sf
back of wall =	2.27 sf
Rear right total =	69.09 sf
Rear Abutment Subtotal =	471 sf
Forward Abutment:	
backwall height =	6 97 ft
beam seat width =	3.00 ft
exposed breastwall height =	4.21 ft
backwall/breastwall length =	29.00 ft
Abutment total =	411.22
Forward left wingwall	
depth of fill in front of wall =	2.00 ft
exposed height behind wall =	0.50 ft
front exposed area =	99.75 sf
top of wall =	19.86 st
back of wall =	2.95 st
Forward left total =	122.56 ST
Forward right wingwall	
depth of fill in front of wall =	0.00 ft
exposed height behind wall =	0.00 ft
front exposed area =	0.00 sf
top of wall =	0.00 sf
back of wall =	0.00 sf
Forward right total =	0.00 sf
Forward Abutment Subtotal =	534 sf
Abutment Subtotal =	1005 sf

	Total =	<u>1,213</u> sy
Pier Subtotal =	1784	sf
No. of Piers =	2	
Column area =	339.292	sf
no. of columns =	1	
average exposed height =	18.00	ft
diameter =	6.00	ft
Columns		
Cap area =	552.6879	
height =	5.17	
thickness =	7.00	
length =	29.33	
Caps		
<u>Piers</u>		

ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL 5, AS PER PLAN

BRIDGE

Girders Properties							
		Dimension Input (for plate girders) (all units are inches)				es)	
Section ID	Unit Weight (lb./ft.)	Bfl. top	tfl. top	Dweb	tweb	Bfl. bot	tfl.bot
Section 1	267.5434	18	1	62	0.6875	18	1
Section 2	519.349	22	2.5	62	0.6875	22	2.5
Section 3	519.349	22	2.5	62	0.6875	22	2.5
Section 4	350 9115	22	1	62	0.6875	22	1 75

Section Lengths

Section ID	Unit Weight (lb./ft.)	Girder 1	Girder 2	Girder 3	Girder 4	Total Lengths
Section 1	267.54	90.00	88.83	87.66	86.5	352.99
Section 2	519.35	126.01	123.52	121.3	119	489.83
Section 3	519.35	36.42	35.92	35.42	35	142.76
Section 4	350.91	108.41	106.28	104.14	102	420.83

Detail factor =

1.010 576,400 lbs

Girder Subtotal =

Splices: Splice 1				
Top Flange	# of plates	length (in)	width (in)	thick (in)
outside plates =	1	36.75	18	0.625
inside plates =	2	36.75	8	0.75
fill plates =	1	18	18	1.5
Bottom Flange				
outside plates =	1	36.75	18	0.625
inside plates =	2	36.75	8	0.75
fill plates =	1	18.5	18	1.5
Web				
plates =	2	56.5	15.75	0.375
Plate weight/splice =	953	lbs		

Splice 2 Top Flange # of plates length (in) width (in) thick (in) outside plates = inside plates = 22 10 43.75 0.625 1 2 43.75 0.75 fill plates = Bottom Flange 1 21.5 22 1.5 outside plates = 64.75 22 1 1 inside plates = 64.75 10 1.125 2 fill plates = 32.375 22 1 0.75 Web 55.25 0.375 plates = 2 15.75 1711 lbs Plate weight/splice =

Splice Bolts	# of bolto	longth (in)	holt ut	washarwt
			140	
	40	1.5	148	11.3
оо. гiange =	40	1.5	148	11.3
/ep =	44	1.5	148	11.3
	^ from stee	i manual		
	* washer w	eight is per 1	00 ct.	
Bolt + Washer weight				
Top Flange =	64	lbs		
Bot. Flange =	64	lbs		
Veb =	70	lbs		
Total weight/splice =	1151	lbe		
No. of splices = $\frac{1}{2}$	0	601		
to or spilles -	0			
Splice 2	# of bolts	length (in)	bolt wt.	washer wt.
Top Flange =	48	1.5	148	11.3
Bot. Flange =	72	1.5	148	11.3
Web =	44	1.5	148	11.3
	* from stee	l manual		
	* washer w	eight is per 1	00 ct.	
Bolt + Washer weight				
Top Flange =	76	lbs		
Bot Flance =	115	lbs		
Web =	70	lbs		
1160 -	70	103		
Fotal weight/splice =	1973	lbs		
No. of splices =	8			
Splice + Bolts Subtotal =		24,988	lbs	
Intermediate Crossframes:				
_ength of Diagonals =	7.00	ft (weighted	avq.)	
No. of Diagonals =	2	, U	U /	
_ength of Horiz. =	6.50	ft (weighted	avg.)	
No. of Horiz. =	2	、 5 -	0,	
Angle weight / ft. =	15.00	lbs/ft		
Crossframe weight =	405	lbs ==> per :	k-frame as	sembly
				-
x-trame stiffeners?	У	y or n	1 41.	
Length =	62.000	in ==> web c	aepth	
Width =	8.000	in		
I hickness =	0.500	ın		
Stiffener weight =	141	lbs ==> per x	k-frame as	sembly
x-frame gusset plates?	у	y or n		
Length =	17.500	in ==> web o	lepth	
Width =	15.000	in		
Thickness =	0.500	in		
Gusset plate weight =	149	lbs ==> per ;	k-frame as	sembly
Total Intermediate Crossframe				
Assembly Weight =	695	lbs ==> per	x-frame a	ssembly
No of assemblies =	72	180 per	A nume a	controly
Detail Factor =	1.05			
Intermediate Crossframe Subtetel -	50500	lbo		
Intermediate Crosstrame Subtotal =	52506	IDS		

End Crossframes:

Length of Diagonals = No. of Diagonals = Length of Horiz. = No. of Horiz. =	6.50 ft (weighted avg.) 2 8.00 ft (weighted avg.) 1 9.80 lbc/ft
	3.00 IDS/IT
End crossframe weight =	206 lbs. ==> per x-frame assembly
End x-frame stiffeners (bearing stiffeners)? Length = Width = Thickness =	y y or n 62.000 in ==> web depth 8.500 in 1.000 in
Stiffener weight =	299 lbs ==> per x-frame assembly
Total x-frame End Assembly Weight = No. of assemblies = Detail Factor =	505 lbs ==> per x-frame assembly 8 1.10
End Crossframe Subtotal =	4441 lbs
Crossframe Subtotal =	56,947 lbs
	Total = <u>658,400</u> lbs

ITEM 513 - WELDED STUD SHEAR CONNECTORS

391
449
443
384
3

Total =

<u>5,001</u> ea

ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, FINISH COAT

Average Flange width =	21.00 i	in
Total Girder Depth =	66.00 i	n
Total beam Length =	1406.41	
Detail Factor =	1.20	

Total = <u>27,500</u> sf

ITEM 514 - FINAL INSPECTION REPAIR

Length =	1406.41	ft
No. Girders =	4	ea
No. Crossframes =	80	ea
	Total =	<u>42</u> ea
ITEM 516 -ARMORLESS PREFORMED JOINT SEAL		
Forward approach slab width =	29.00	ft
	Total =	<u>29</u> ft

ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL

FA Length =	28.50 28.50	ft
	Total =	<u>57</u> ft

ITEM 518 - POROUS BACKFILL WITH GEOTEXTILE FABRIC

	Total =	<u>61</u> су
Frwd. Abutment Subtotal =	709 cf	
Right wingwall =	0.00 sf	
Left wingwall =	112.40 sf	
length =	29.00 ft	
app. Slab thickness =	1.42 ft	
Height =	9.76 ft	
Thickness =	2.00 ft	
Forward Abutment:		
Rear Abutment Subtotal =	927 cf	
Right wingwall =	91.75 sf	
Left wingwall =	104.61 sf	
length =	29.00 ft	
app. Slab thickness =	1.42 ft	
Height =	10.63 ft	
Thickness =	2.00 ft	
Rear Abutment:		

ITEM 518 - 6" PERFORATED CORRUGATED PLASTIC PIPE

Rear Abutment: length =	49.34 ft	
Forward Abutment: length =	41.65 ft	
	Total =	<u>91</u> ft

ITEM 518 - 6" NON-PERFORATED CORRUGATED PLASTIC PIPE INCLUDING SPECIALS

Forward Abutment:		
length =	10.00 ft	
Forward Abutment:		
length =	0.00 ft	
	Total =	<u>10</u> ft

ITEM 524 - DRILLED SHAFTS, 42" DIAMETER, INTO BEDROCK WITH QC/QA, AS PER PLAN



ITEM 524 - DRILLED SHAFTS, 48" DIAMETER, ABOVE BEDROCK WITH QC/QA, AS PER PLAN

Rear Abutment: length = no. of shafts =

11.2 ft 3 Total = <u>34</u> ft

ITEM 524 - DRILLED SHAFTS, 48" DIAMETER, INTO BEDROCK WITH QC/QA, AS PER PLAN



ITEM 524 - DRILLED SHAFTS, 54" DIAMETER, ABOVE BEDROCK WITH QC/QA, AS PER PLAN

Pier 1 length = no. of shafts =	27 ft 4	
Pier 2 length = no. of shafts =	27.3 ft 4	
	Total =	<u>218</u> ft

ITEM 526 - REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=17")

<i>Rear Approach Slab:</i> Width = Length =	29 ft 30 ft			
<i>Frwd. Approach Slab:</i> Width = Length =	29 ft 30 ft			
	Total = <u>194</u> s	y		
ITEM 526 - TYPE A INSTALLATION				
Rear Approach Slab: length =	<mark>26</mark> ft			
	Total = <u>26</u> ft			
ITEM 526 - TYPE C INSTALLATION				
Forward Approach Slab: length =	<mark>29</mark> ft			
	Total = <u>29</u> ft			
ITEM SPECIAL - STRUCTURES: TE	MPORARY SUPPORT OF STEEL GIRDERS			
Temporary Bents: Cost per temporary bent = No. of temporary bents =	\$20,000.00 5			
	Lump sum = <u>\$100.000</u>			
ITEM 846 - POLYMER MODIFIED EXPANSION JOINT SYSTEM				
Rear approach slab = depth = width =	26.00 ft 2 in 20 in Total = 8 ct	F		

ITEM 855 - POST-TENSIONING STRAND TENDON

Unit wieght per strand=	0.74	lbs./ft.
No. strands of per tendon =	19.00	each
Tendon 1 length =	27.9	ft
Tendon 2 length =	27.4	ft
No. of Tendon 1 =	6	each
No. of Tendon 2 =	6	each

Total = <u>4665</u> lbs.

ITEM 869 - HIGH LOAD MULIT-ROTATIONAL (HLMR) BEARINGS, AS PER PLAN

		Total =	<u>8</u> each
Forward abutment =	4	each	
Rear abutment =	4	each	