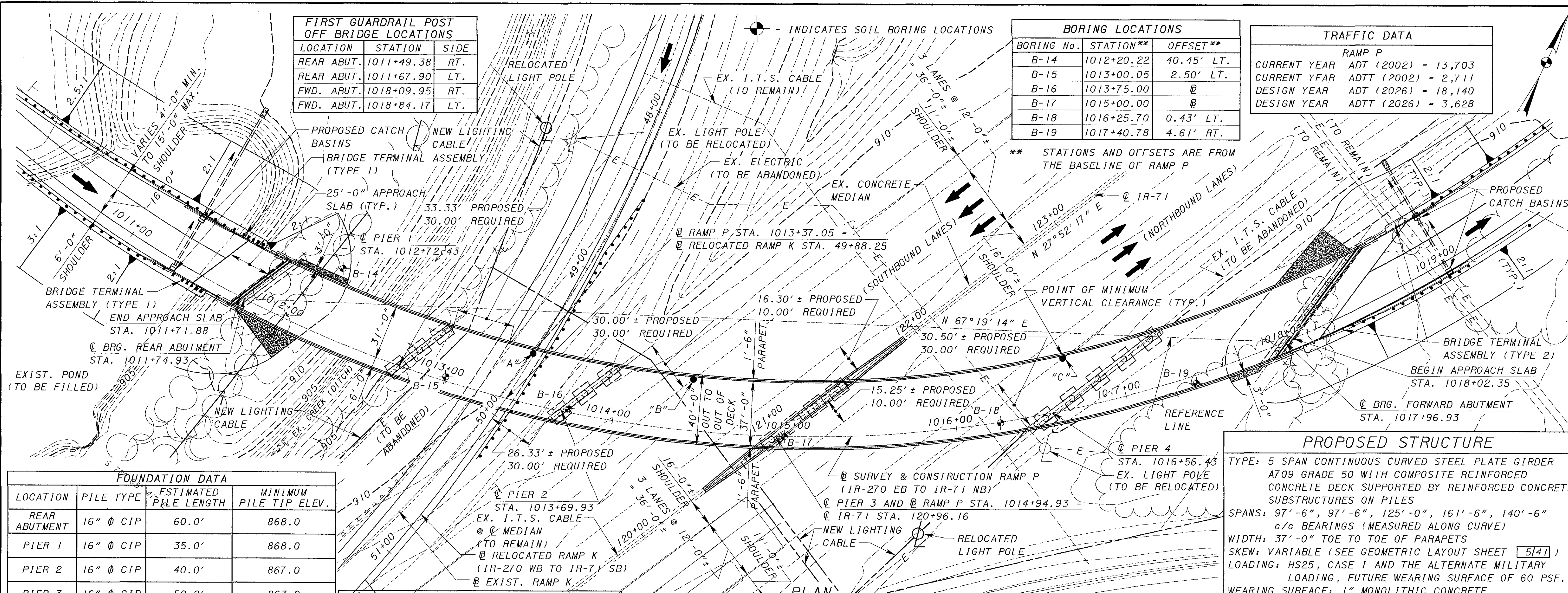


LOCATION	STATION	SIDE
REAR ABUT.	1011+49.38	RT.
REAR ABUT.	1011+67.90	LT.
FWD. ABUT.	1018+09.95	RT.
FWD. ABUT.	1018+84.17	LT.

BORING No.	STATION**	OFFSET**
B-14	1012+20.22	40.45' LT.
B-15	1013+00.05	2.50' LT.
B-16	1013+75.00	Ø
B-17	1015+00.00	Ø
B-18	1016+25.70	0.43' LT.
B-19	1017+40.78	4.61' RT.

TRAFFIC DATA	
RAMP P	
CURRENT YEAR	ADT (2002) = 13,703
CURRENT YEAR	ADTT (2002) = 2,711
DESIGN YEAR	ADT (2026) = 18,140
DESIGN YEAR	ADTT (2026) = 3,628



FOUNDATION DATA			
LOCATION	PILE TYPE	ESTIMATED PILE LENGTH	MINIMUM PILE TIP ELEV.
REAR ABUTMENT	16" Ø CIP	60.0'	868.0
PIER 1	16" Ø CIP	35.0'	868.0
PIER 2	16" Ø CIP	40.0'	867.0
PIER 3	16" Ø CIP	50.0'	863.0
PIER 4	16" Ø CIP	45.0'	862.7*
FORWARD ABUTMENT	16" Ø CIP	55.0'	863.0

C.I.P. = CAST-IN-PLACE PILE
* - PILE TIP AT TOP OF BEDROCK

TEMPORARY BENCHMARK #1	
#5 REBAR WITH CAP, @ IR-71 STA. 123+14.06,	ELEV. 912.37
TEMPORARY BENCHMARK #2	
#5 REBAR WITH CAP, @ IR-71 STA. 123+69.93,	ELEV. 911.12

- NOTES:**
- FOR ALIGNMENT, GEOMETRY AND SUBSTRUCTURE LAYOUT, SEE GEOMETRIC LAYOUT SHEET 547.
 - EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS.
 - ALL SHEETS WITH PLAN DIMENSIONS ARE SHOWN HORIZONTAL.
 - THE PROPOSED PROFILE GRADE IS ONLY WITHIN THE BRIDGE LIMITS. SEE ROADWAY PLANS FOR PAVEMENT ELEVATIONS BEYOND THE BRIDGE LIMITS.

PROPOSED STRUCTURE

TYPE: 5 SPAN CONTINUOUS CURVED STEEL PLATE GIRDER A709 GRADE 50 WITH COMPOSITE REINFORCED CONCRETE DECK SUPPORTED BY REINFORCED CONCRETE SUBSTRUCTURES ON PILES

SPANS: 97'-6", 97'-6", 125'-0", 161'-6", 140'-6"
c/c BEARINGS (MEASURED ALONG CURVE)

WIDTH: 37'-0" TOE TO TOE OF PARAPETS

SKWEW: VARIABLE (SEE GEOMETRIC LAYOUT SHEET 547)

LOADING: HS25, CASE I AND THE ALTERNATE MILITARY LOADING, FUTURE WEARING SURFACE OF 60 PSF.

WEARING SURFACE: 1" MONOLITHIC CONCRETE

APPROACH SLAB: AS-1-81 (25'-0" LONG)

ALIGNMENT: 8°00'00" CURVE

SUPERELEVATION: 0.060 FT/FT AND 0.040 FT/FT

LATITUDE: N 40°06'34"

LONGITUDE: W 82°58'42"

STRUCTURE FILE NO.: 2511452

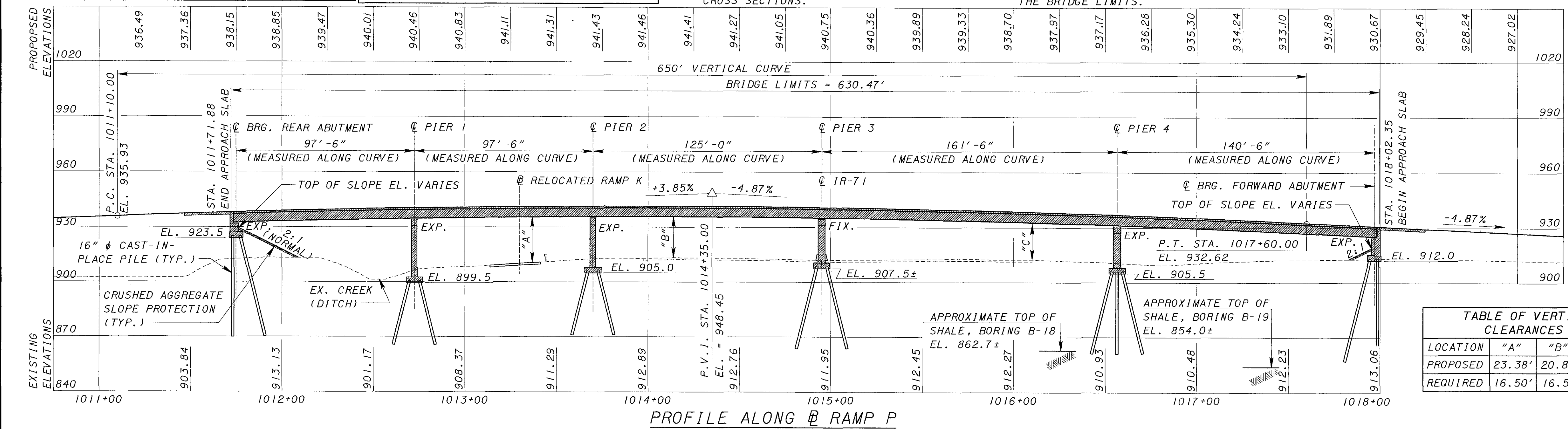


TABLE OF VERTICAL CLEARANCES			
LOCATION	"A"	"B"	"C"
PROPOSED	23.38'	20.82'	16.57'
REQUIRED	16.50'	16.50'	16.50'

DESIGN SYSTEMS CORPORATION
 9600 S. STATE ST., SUITE 300, CLEVELAND, OHIO 44124-4000
 DATE: 7/19/05
 REVISED: 2511452
 DRAWN: CAG
 CHECKED: NFF
 DESIGNED: BTA
 FRANKLIN COUNTY
 STA. 1011+71.88
 STA. 1018+02.35
 SITE PLAN
 BRIDGE NO. FRA-270-2583A
 RAMP P OVER IR-71
 FRA-270-24.47
 PID 77320
 1/41
 769/846

STRUCTURE GENERAL NOTES

REFER TO THE FOLLOWING STANDARD BRIDGE DRAWINGS:

A-1-69	REVISED	07-19-02
AS-1-81	REVISED	07-19-02
EXJ-4-87	REVISED	07-19-02
GSD-1-96	REVISED	07-19-02
SBR-1-99	REVISED	07-19-02

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS:

898 DATED 07-16-04

DESIGN SPECIFICATIONS:

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2002, INCLUDING THE 2003 INTERIM SPECIFICATIONS, THE AASHTO "GUIDE SPECIFICATIONS FOR HORIZONTALLY CURVED STEEL GIRDER HIGHWAY BRIDGES", 2003 AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING:

HS25, CASE 1 AND THE ALTERNATE MILITARY LOADING.
FUTURE WEARING SURFACE (FWS) OF 60 lbs/sq ft

DESIGN DATA:

CONCRETE CLASS QSC2 - COMPRESSIVE STRENGTH 4500 P.S.I. (SUPERSTRUCTURE)
CONCRETE CLASS QSC1 - COMPRESSIVE STRENGTH 4000 P.S.I. (SUBSTRUCTURE)

REINFORCING STEEL - ASTM A615 OR A996
GRADE 60 MINIMUM YIELD STRENGTH 60,000 P.S.I.
SPIRAL REINFORCEMENT MAY BE PLAIN BARS,
ASTM A82 OR A615

STRUCTURAL STEEL - ASTM A709 GRADE 50 - YIELD STRENGTH 50,000 P.S.I.

DECK PROTECTION METHOD:

EPOXY COATED REINFORCING STEEL
2 1/2" CONCRETE COVER

MONOLITHIC WEARING SURFACE:

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1" THICK.

ITEM 203, EMBANKMENT, AS PER PLAN:

PLACE AND COMPACT EMBANKMENT MATERIAL IN 6 INCH LIFTS FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT BETWEEN STATIONS 1010+71.88 TO 1018+02.35.

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, CONSTRUCT THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. DO NOT BEGIN THE INSTALLATION OF THE ABUTMENT PILES UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED.

FRICTION TYPE PILES:

THE PILES ORDER LENGTHS, AND ULTIMATE BEARING VALUES ARE AS FOLLOWS:

REAR ABUTMENT PILES:

14 PILES 65 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 145 TONS PER PILE
1 DYNAMIC LOAD-TESTING ITEM

PIER 1 PILES:

18 PILES 40 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 175 TONS PER PILE

PIER 2 PILES:

18 PILES 45 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 165 TONS PER PILE
1 DYNAMIC LOAD-TESTING ITEM

PIER 3 PILES:

24 PILES 55 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 150 TONS PER PILE
1 DYNAMIC LOAD-TESTING ITEM

PIER 4 PILES:

25 PILES 50 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 180 TONS PER PILE
1 DYNAMIC LOAD-TESTING ITEM

FORWARD ABUTMENT PILES:

22 PILE 60 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 175 TONS PER PILE
1 DYNAMIC LOAD-TESTING ITEM

BATTERED PILES:

THE BLOW COUNT FOR BATTERED PILES SHALL BE THE BLOW COUNT DETERMINED FOR VERTICAL PILES OF THE SAME ULTIMATE BEARING VALUE DIVIDED BY AN EFFICIENCY FACTOR (D). COMPUTE THE EFFICIENCY FACTOR (D) AS FOLLOWS:

$$D = \frac{1-UG}{\sqrt{(1+G^2)}}$$

U = COEFFICIENT OF FRICTION, WHICH IS ESTIMATED AT 0.05 FOR DOUBLE-ACTING AIR OPERATED OR DIESEL HAMMERS; 0.1 FOR SINGLE-ACTING AIR OPERATED OR DIESEL HAMMERS; AND 0.2 FOR DROP HAMMERS.

G = RATE OF BATTER (1/3, 1/4, ETC.)

ITEM 514, FIELD PAINTING STRUCTURAL STEEL,

FINISH COAT:

THE COLOR OF THE FINISH COAT SHALL BE A GRAY COLOR MEETING FEDERAL STANDARD NUMBER 16440.

ITEM 516, ELASTOMERIC BEARINGS WITH INTERNAL LAMINATES (ONLY),

AS PER PLAN:

IN ADDITION TO THE PROVISIONS OF ITEM 516, THE BEARINGS SHALL ALSO CONFORM TO THE FOLLOWING SPECIFICATIONS:

1.0 DESIGN AND MATERIALS REQUIREMENTS:

1.1 PTFE

- A. PTFE FABRIC FIBERS SHALL CONFORM TO THE FOLLOWING:
 - 1. THE RESIN FROM WHICH THE FIBERS ARE PRODUCED SHALL BE 100 PERCENT PTFE CONFORMING TO ASTM D4894.
 - 2. TENSILE STRENGTH - ASTM D2256 - 24,000 PSI (MINIMUM).
 - 3. ELONGATION - ASTM D2256 - 75 PERCENT (MINIMUM).
 - 4. THE TFE FABRIC SHALL HAVE A MINIMUM THICKNESS OF 0.0625 INCH (COMPRESSED). MAXIMUM THICKNESS SHALL BE 0.125 INCH (COMPRESSED).
- B. FINISHED UNFILLED PTFE SHEET SHALL BE MADE FROM 100 PERCENT VIRGIN PTFE RESIN AND SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
 - 1. TENSILE STRENGTH D4894 - 2800 PSI (MINIMUM).
 - 2. ELONGATION ASTM D4894 - 200 PERCENT (MINIMUM).
 - 3. SPECIFIC GRAVITY - ASTM D792 - 2.13 (MINIMUM).
 - 4. MELTING POINT - ASTM D4894 - 623° F +/- 2
 - 5. MINIMUM THICKNESS SHALL BE 0.125 INCH. SHEET SHALL BE RECESSED AND EPOXY BONDED INTO A STEEL SUBSTRATE. THE SHOULDERS OF THE RECESS SHALL BE SHARP AND SQUARE AND THE DEPTH SHALL BE EQUAL TO ONE-HALF OF THE PTFE SHEET THICKNESS.
 - 6. PTFE SHEET SHALL BE COMMERCIALY ETCHED ON ITS BONDING SIDE.
- C. MAXIMUM CONTACT STRESSES SHALL CONFORM TO THE FOLLOWING:

MATERIAL	AVERAGE CONTACT STRESSES		EDGE CONTACT STRESS	
	PERMANENT LOADS	ALL LOADS	PERMANENT LOADS	ALL LOADS
WOVEN PTFE FIBER OVER METALLIC SUBSTRATE	4.0 KSI	6.0 KSI	5.0 KSI	7.5 KSI
CONFINED UNFILLED SHEET PTFE	4.0 KSI	6.0 KSI	5.0 KSI	7.5 KSI

1.2 STAINLESS STEEL

STAINLESS STEEL SHEET SURFACE SHALL CONFORM TO ASTM A167 OR A240 TYPE 304. STAINLESS STEEL IN CONTACT WITH PTFE SHALL HAVE A #8 MIRROR FINISH OR BETTER.

2.0 FABRICATION:

2.1 ATTACHMENT OF SHEET PTFE TO SUBSTRATE:

- A. PTFE SHEET SHALL BE RECESSED INTO AND BONDED TO A STEEL SUBSTRATE.
- B. PTFE SHALL BE RECESSED FOR ONE HALF ITS THICKNESS.
- C. THE BONDING SURFACE OF THE STEEL SHALL BE CLEANED OF RUST, SCALE, OIL AND GREASE BY BLAST CLEANING AND THEN WIPED CLEAN WITH A CLEANING SOLVENT. BLAST CLEANING SHALL BE PERFORMED WITHIN A MAXIMUM OF FOUR HOURS PRIOR TO BONDING.
- D. THE ADHESIVE MATERIAL, AND THE BONDING PROCEDURES TO BE USED, AND SURFACE PREPARATION SHALL CONFORM TO THE REQUIREMENTS OF FEDERAL SPECIFICATION MMM-A-134 AND THE MANUFACTURER'S RECOMMENDATIONS.
- E. AFTER COMPLETION OF THE BONDING OPERATION, THE PTFE SURFACE SHALL BE SMOOTH AND FREE FROM BUBBLES.
- F. TOLERANCES - PTFE SUBSTRATES:
SUBSTRATE FLATNESS: CLASS A

2.2 ATTACHMENT OF PTFE FABRIC TO SUBSTRATE

- A. PTFE FABRIC SHALL BE MECHANICALLY INTERLOCKED AND BONDED TO THE STEEL SUBSTRATE.
- B. THE BONDING SURFACE OF THE STEEL SHALL BE CLEANED OF RUST, SCALE, OIL AND GREASE BY BLAST CLEANING AND THEN CLEANED WITH SOLVENT. BLAST CLEANING SHALL BE PERFORMED WITHIN A MAXIMUM OF FOUR HOURS PRIOR TO BONDING.
- C. THE MECHANICAL INTERLOCK, ADHESIVE BONDING MATERIAL AND PROCEDURES, AND SURFACE PREPARATION SHALL CONFORM TO THE REQUIREMENTS OF FEDERAL SPECIFICATION MMM-A-134 AND THE MANUFACTURER'S RECOMMENDATIONS.
- D. MIGRATION OF EPOXY THROUGH THE FABRIC WILL NOT BE PERMITTED.
- E. FABRIC SHALL BE FURNISHED IN ONE PIECE. EDGES SHALL BE OVERSEWN OR RECESSED SO THAT NO CUT FABRIC EDGES ARE EXPOSED.
- F. TOLERANCES - PTFE SUBSTRATE:
SUBSTRATE SURFACES: CLASS A

2.3 ATTACHMENT OF SHEET STAINLESS STEEL

STAINLESS STEEL SHALL BE ATTACHED TO ITS STEEL SUBSTRATE BY A CONTINUOUS SEAL WELD AROUND ITS ENTIRE PERIMETER. WELDS SHALL CONFORM TO THE AWS REQUIREMENTS FOR STAINLESS STEEL. THE WELDER SHALL BE PRE-QUALIFIED BY TEST WELDS PREPARED, WELDED AND TESTED IN ACCORDANCE WITH 6.7 OF ANSI/AWS D1.3, STRUCTURAL WELDING CODE - SHEET STEEL. AFTER WELDING, THE STAINLESS STEEL SHEET SHALL BE FLAT, FREE FROM WRINKLES AND IN CONTINUOUS CONTACT WITH ITS BACKING PLATE. AFTER WELDING THE ENTIRE STAINLESS STEEL SURFACE SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1.2 OF THIS SPECIFICATION AFTER WELDING. NO ROUGHNESS FROM THE WELD PROTRUDING ABOVE THE SURFACE OF THE STAINLESS STEEL WILL BE PERMITTED.

2.4 WELDING

WELDING AS A MEANS OF ATTACHMENT SHALL BE DONE IN A CONTROLLED MANNER AND SHALL CONFORM TO SPECIFICATION 513. WELDING TO A STEEL PLATE WHICH HAS BONDED TFE SURFACE MAY BE PERMITTED PROVIDING WELDING PROCEDURES ARE ESTABLISHED WHICH RESTRICT THE MAXIMUM TEMPERATURE REACHED BY THE BOND AREA TO LESS THAN 300° F, AS DETERMINED BY TEMPERATURE, INDICATING PENCILS, OR OTHER SUITABLE MEANS.

3.0 METHOD OF MEASUREMENT AND BASIS OF PAYMENT

REFER TO CMS ITEM 516.

MAINTENANCE OF TRAFFIC:

IR-71 TRAFFIC WILL BE MAINTAINED ON INTERSTATE ROUTE 71 AT ALL TIMES EXCEPT AS PERMITTED FOR FALSEWORK AND STEEL ERECTION. WITH A MINIMUM VERTICAL CLEARANCE OF 14'-6". FOR MAINTENANCE OF TRAFFIC NOTES, PERMITTED LANE CLOSURE AND DETAILS REFER TO THE ROADWAY PLANS.

ITEM 512, SEALING OF CONCRETE SURFACES

(EPOXY-URETHANE):

THE CONCRETE SEALING MATERIAL SHALL BE THE "LIGHT NEUTRAL" COLOR MEETING THE FEDERAL COLOR STANDARD NO. 17778 AS PER THE DETAILS ON THE PLANS.

INDEX OF SHEETS:

- SITE PLAN [1/41]
- GENERAL PLAN AND ELEVATION [2/41]
- GENERAL NOTES [3/41]
- ESTIMATED QUANTITIES [4/41]
- GEOMETRIC LAYOUT [5/41]
- PILE LAYOUT PLAN [6/41]
- REAR ABUTMENT DETAILS [7/41] THRU [9/41]
- FORWARD ABUTMENT DETAILS [10/41] THRU [12/41]
- PIER DETAILS [13/41] THRU [20/41]
- FRAMING PLAN [21/41]
- GIRDER ELEVATION [22/41]
- SUPERSTRUCTURE DETAILS [23/41] & [24/41]
- CAMBER DIAGRAMS [25/41] THRU [27/41]
- BEARING DETAILS [28/41] & [29/41]
- TRANSVERSE SECTION [30/41]
- SLAB PLAN [31/41]
- PARAPET DETAILS [32/41]
- SCREED ELEVATIONS - [33/41] THRU [35/41]
- STRIP SEAL EXPANSION JOINT DETAILS [36/41]
- APPROACH SLAB DETAILS [37/41]
- REINFORCING STEEL LIST [38/41] THRU [41/41]

DESIGN AGENCY: **TANS SYSTEMS CORPORATION**
 5 CLEVELAND DRIVE, SUITE 450
 CLEVELAND, OHIO 44115-4500

DATE	7/19/05
REVIEWED	REER
STRUCTURE FILE NUMBER	2511452
DRAWN	CAG
CHECKED	NFF

GENERAL NOTES
 BRIDGE NO. FRA-270-2583A
 RAMP P OVER IR-71

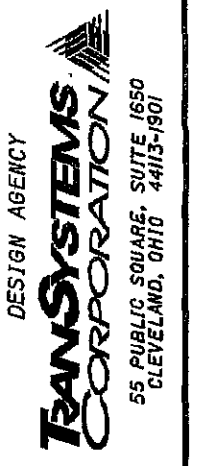
FRA-270-24.47
 PID 77320

3 / 41
 771
 846

MADE BY BTA DATE 5/23/05
CHECK'D BY NFF DATE 7/19/05

ESTIMATED QUANTITIES

ITEM	EXTENSION	TOTAL	UNIT	DESCRIPTION	REAR ABUTMENT	FORWARD ABUTMENT	PIERS	SUPERSTRUCTURE	GENERAL	AS PER PLAN REFERENCE SHEET NUMBER
503	11100	LUMP		COFFERDAMS, CRIBS AND SHEETING					LUMP	
503	21300	LUMP		UNCLASSIFIED EXCAVATION					LUMP	
505	11100	LUMP		PILE DRIVING EQUIPMENT MOBILIZATION					LUMP	
507	00700	5725	FT	16" CAST-IN-PLACE REINFORCED CONCRETE PILES, DRIVEN	840	1210	3675			
507	00750	6330	FT	16" CAST-IN-PLACE REINFORCED CONCRETE PILES, FURNISHED	910	1320	4100			
509	10000	279370	POUND	EPOXY COATED REINFORCING STEEL	9406	14460	80860	174644		
512	10100	2344	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	92	151	632	1469		
513	10300	1316859	POUND	STRUCTURAL STEEL MEMBERS, LEVEL 5				1316859		
513	20000	12435	EACH	WELDED STUD SHEAR CONNECTORS				12435		
514	00060	64081	SQ FT	FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT				64081		
514	00066	64081	SQ FT	FIELD PAINTING STRUCTURAL STEEL, FINISH COAT				64081		
516	11211	129	FT	STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL, AS PER PLAN	45	84				36/41
516	44100	5	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), (21" x 27" x 2.79")			5			
516	44201	5	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (10 1/2" x 17" x 3.02")	5					28/41 & 29/41
516	44201	5	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (13" x 20" x 3.48")		5				28/41 & 29/41
516	44300	2	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), (17" x 26" x 4.94")			2			
516	44301	3	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (17" x 26" x 4.94")			3			28/41 & 29/41
516	44400	2	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), (18" x 25" x 5.45")			2			
516	44400	2	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), (18" x 25" x 5.63")			2			
516	44401	3	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (18" x 25" x 5.63")			3			28/41 & 29/41
516	44401	3	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (23" x 31 1/2" x 5.65")			3			28/41 & 29/41
518	21200	128	CU YD	POROUS BACKFILL WITH FILTER FABRIC	41	87				
518	40000	135	FT	6" PERFORATED CORRUGATED PLASTIC PIPE	50	85				
518	40010	47	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS	17	30				
523	20000	5	EACH	DYNAMIC LOAD TESTING	1	1	3			
526	25000	338	SQ YD	REINFORCED CONCRETE APPROACH SLABS (T-15")	114	224				
601	20000	480	SQ YD	CRUSHED AGGREGATE SLOPE PROTECTION	240	240				
898	11100	978	CU YD	QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE				978		
898	20100	255	CU YD	QC/QA CONCRETE, CLASS QSC1, SUBSTRUCTURE (PIER ABOVE FOOTING)			255			
898	20150	360	CU YD	QC/QA CONCRETE, CLASS QSC1, SUBSTRUCTURE (ABUTMENT)	117	243				
898	20300	167	CU YD	QC/QA CONCRETE, CLASS QSC1, FOOTING			167			



DESIGN AGENCY
DATE 7/19/05
REVIEWED RER
STRUCTURE FILE NUMBER 2511452
DRAWN CAG
REVISOR
DESIGNED BTA
CHECKED NFF

ESTIMATED QUANTITIES
BRIDGE NO. FRA-270-2583A
RAMP P OVER IR-71

FRA-270-24.47
PID 77320

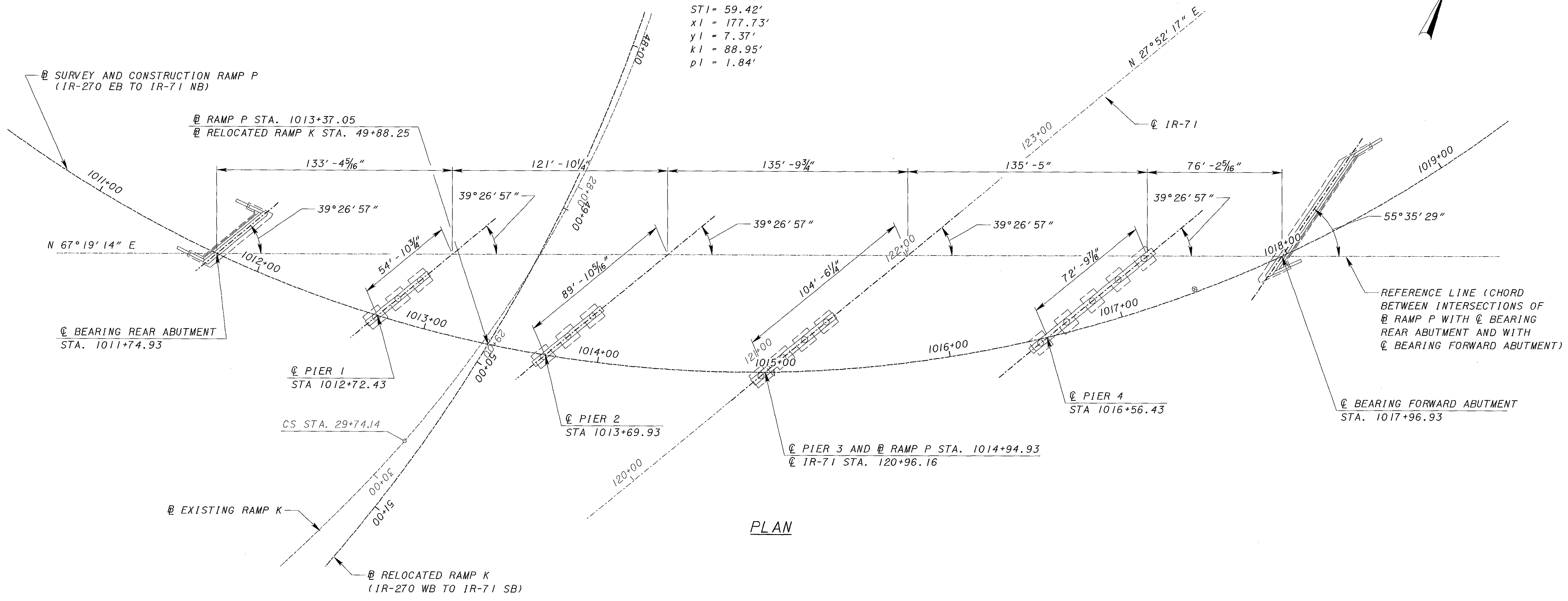
4/41
772
846

**CURVE DATA
B CONSTRUCTION RELOCATED RAMP K**

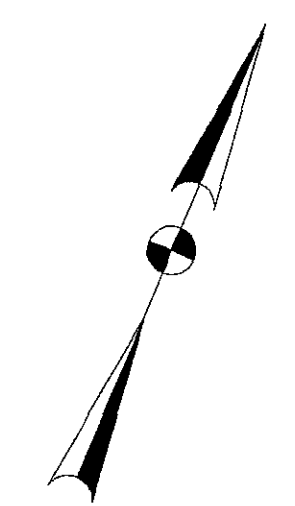
P.I. STA. = 49+09.36
 $\Delta = 36^\circ 51' 34''$ (RT)
 $Dc = 6^\circ 00' 00''$
 $R = 954.93'$
 $T = 318.21'$
 $L = 614.32'$
 $E = 51.62'$

**CURVE DATA
B CONSTRUCTION RAMP P**

P.I. STA. = 1017+83.67 $\Delta c = 105^\circ 48' 14''$ (LT)
 $\Delta = 112^\circ 55' 26''$ (LT) $Lc = 1,322.55'$
 $Dc = 8^\circ 00' 00''$ $Ts1 = 1,170.23'$
 $R = 716.20'$ $Ts2 = 1,082.50'$
 $Ls1 = 178.00'$ $Es = 581.78'$
 $Theta1 = 7^\circ 07' 12''$ $e_{max} = 0.060$
 $LT1 = 118.76'$
 $ST1 = 59.42'$
 $x1 = 177.73'$
 $y1 = 7.37'$
 $kl = 88.95'$
 $pl = 1.84'$



PLAN



DESIGN AGENCY
TRANSYSTEMS CORPORATION
 55 PUBLIC SQUARE, SUITE 1650
 CLEVELAND, OHIO 44115-9901

DESIGNED	DATE	REVIEWED	DATE
BTA	7/19/05	REK	7/19/05
CHECKED	FILE NUMBER	STRUCTURE	FILE NUMBER
NFF	2511452		

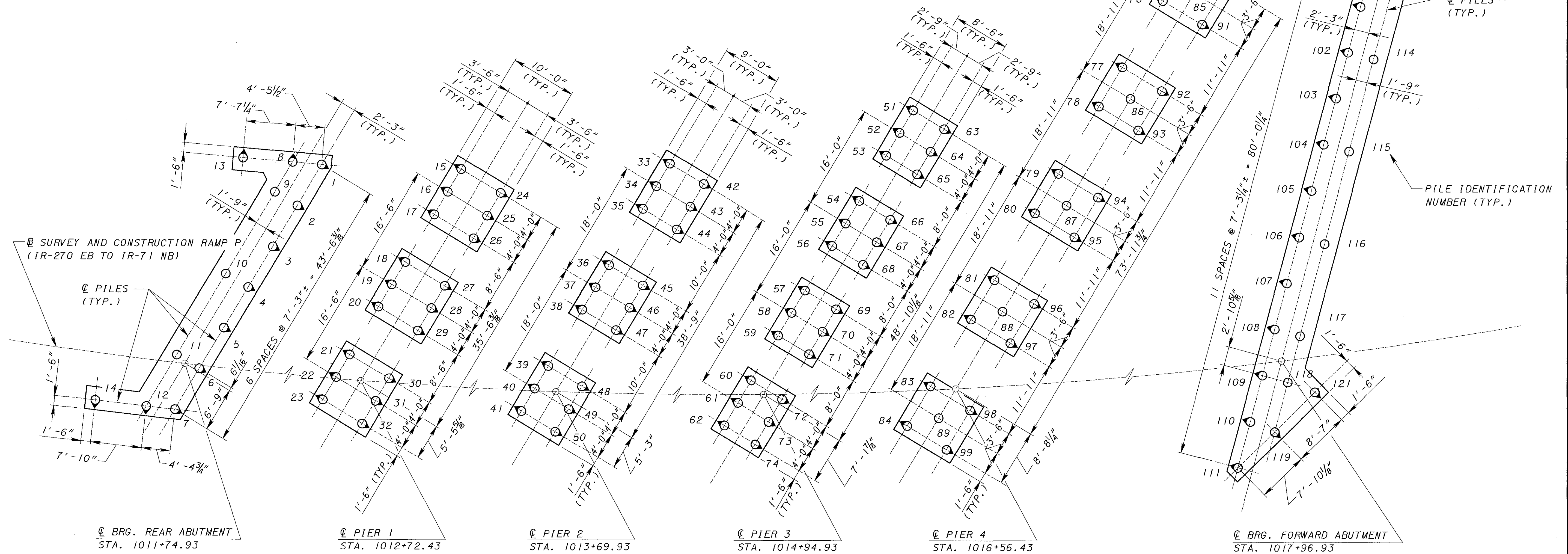
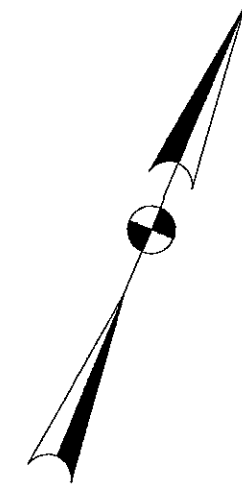
GEOMETRIC LAYOUT
 BRIDGE NO. FRA-270-2583A
 RAMP P OVER IR-71

FRA-270-24.47
 PID 77320

5/41

773
846

PILE DATA						
LOCATION	REAR ABUTMENT	PIER 1	PIER 2	PIER 3	PIER 4	FORWARD ABUTMENT
PILE NUMBER	1 THRU 14	15 THRU 32	33 THRU 50	51 THRU 74	75 THRU 99	100 THRU 121
PILE TYPE	16" Ø CIP	16" Ø CIP	16" Ø CIP	16" Ø CIP	16" Ø CIP	16" Ø CIP
BATTERED PILES	1 THRU 8, 12, 13 AND 14	15 THRU 32	33 THRU 50	51 THRU 74	75 THRU 84 AND 90 THRU 99	100 THRU 112, 119, 120 AND 121
VERTICAL PILES	9, 10 AND 11				85 THRU 89	113 THRU 118
CUT-OFF ELEVATION	924.50	901.00	906.50	909.00	907.00	913.00
ESTIMATED TIP ELEVATION	864.50	866.00	866.50	859.00	862.00	858.00
ESTIMATED LENGTH	60.00'	35.00'	40.00'	50.00'	45.00'	55.00'

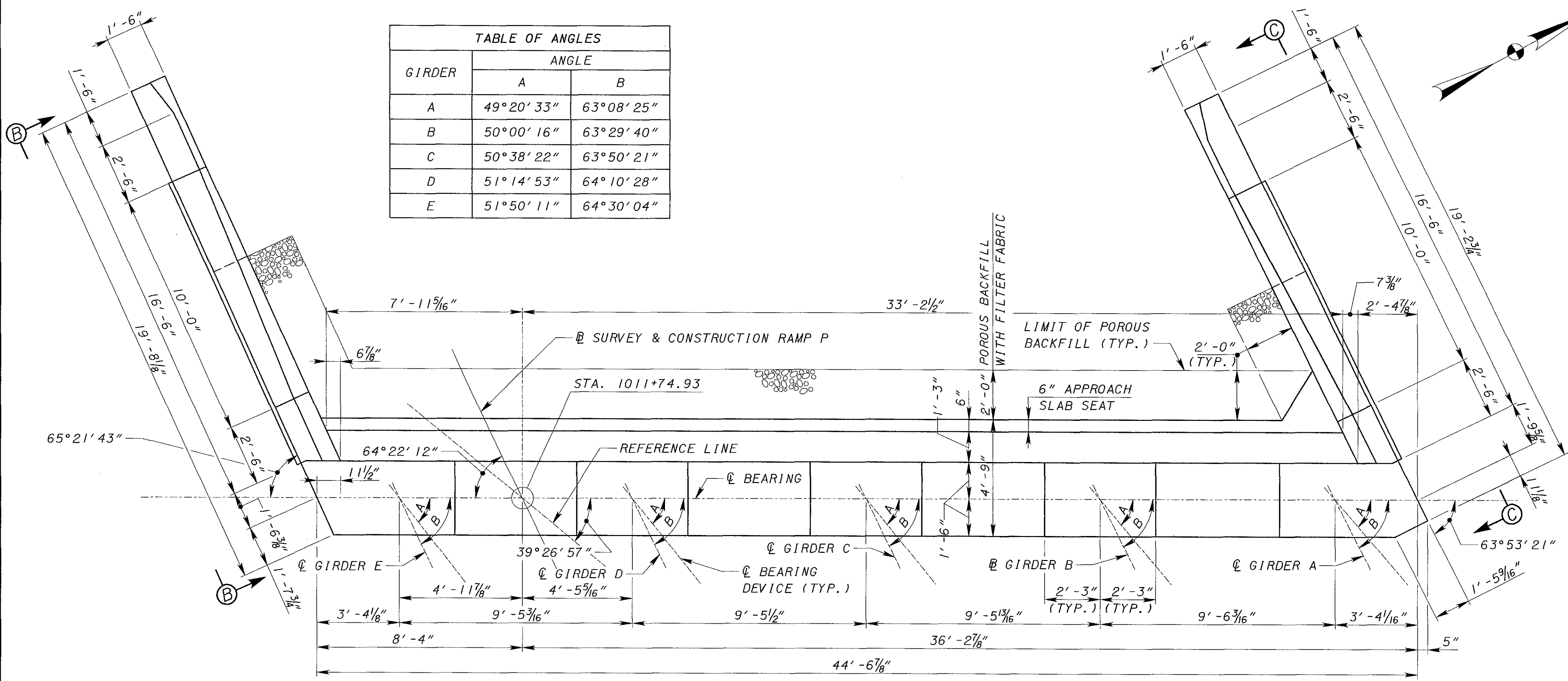


PILE LAYOUT DIAGRAM

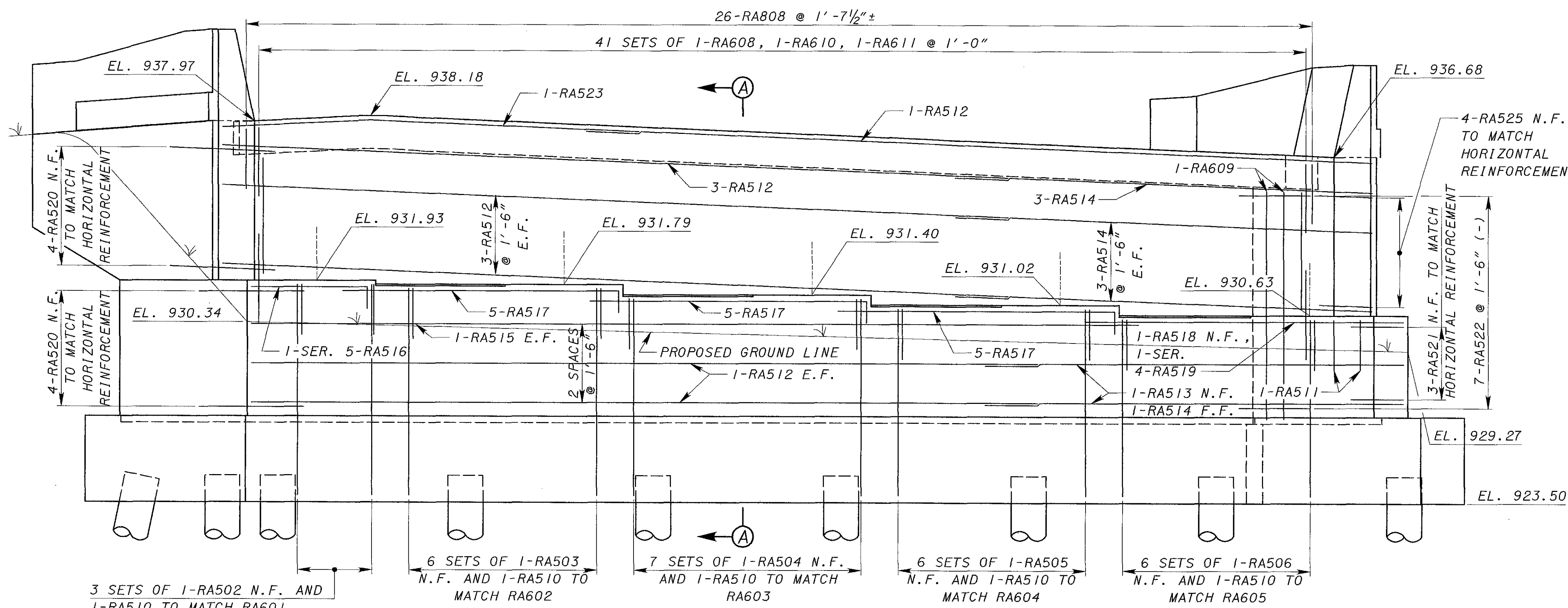
NOTES:

1. FOR DIMENSIONS AND SKEW ANGLES NOT SHOWN, SEE GEOMETRIC LAYOUT SHEET [5/4].
2. ALL PILES ARE 16" DIAMETER CAST-IN-PLACE REINFORCED CONCRETE.
3. ◊ INDICATES BATTER PILE (1:4) IN THE DIRECTION SHOWN.
4. FOR ABUTMENT, WINGWALL AND PIER DETAILS, SEE SHEETS [7/4] TO [20/4].

GIRDER	ANGLE	
	A	B
A	49°20'33"	63°08'25"
B	50°00'16"	63°29'40"
C	50°38'22"	63°50'21"
D	51°14'53"	64°10'28"
E	51°50'11"	64°30'04"



PLAN



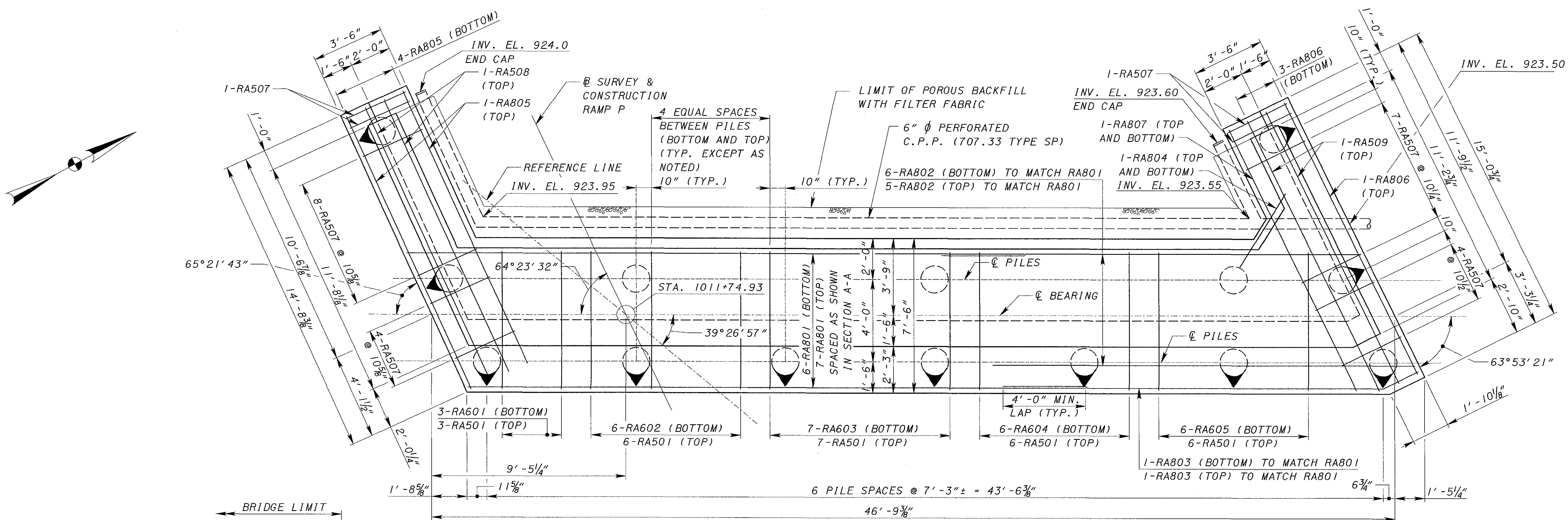
ELEVATION

(FOOTING AND WINGWALL REINFORCING NOT SHOWN FOR CLARITY)

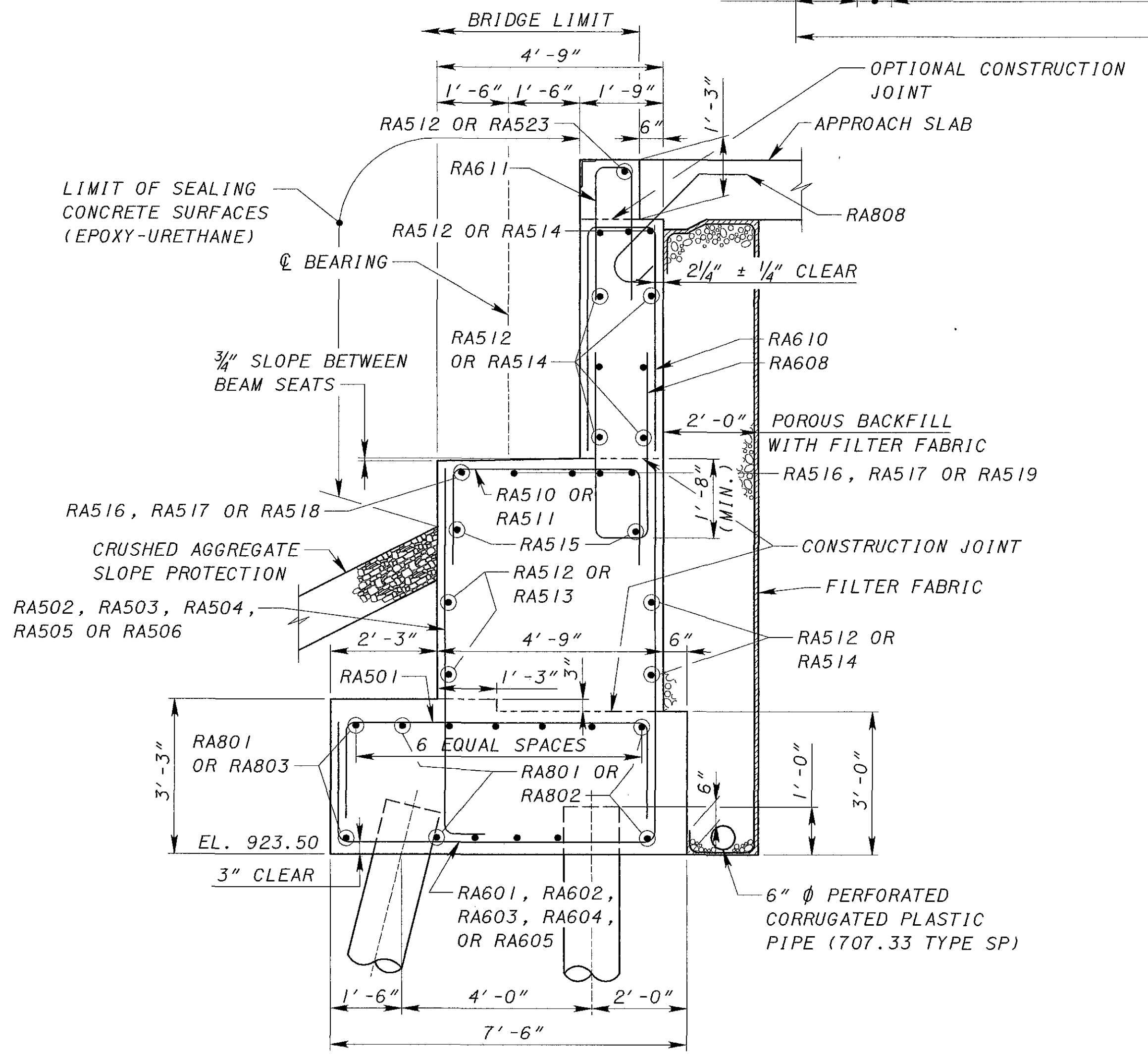
NOTES:

- FOR REINFORCING SCHEDULE, SEE SHEET 38/41.
- FOR FOOTING PLAN AND SECTION A-A, SEE SHEET 8/41.
- FOR VIEW B-B, C-C AND FOR WINGWALL DETAILS, SEE SHEET 9/41.
- POROUS BACKFILL WITH FILTER FABRIC, 2'-0" THICK, SHALL EXTEND UP TO THE PLANE OF THE SUBGRADE, TO 1'-0" BELOW THE EMBANKMENT SURFACE, AND Laterally TO THE ENDS OF THE WINGWALLS.
- BRIDGE SEAT REINFORCING, SETTING ANCHORS: ACCURATELY PLACE REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES OR THE PRE-SETTING OF BEARING ANCHORS.
- BACKWALL CONCRETE: IN ADDITION TO 511.10, DO NOT PLACE BACKWALL CONCRETE ABOVE OPTIONAL CONSTRUCTION JOINT AT THE APPROACH SLAB SEAT UNTIL AFTER THE DECK CONCRETE IN THE SPAN ADJACENT TO THE ABUTMENT HAS BEEN PLACED.
- INSTALLATION OF SEAL: DURING INSTALLATION OF THE SUPPORT/ARMOR FOR THE SUPERSTRUCTURE SIDE OF THE EXPANSION JOINT SEAL, OBSERVE THE SEATING OF THE BEAMS ON BEARINGS TO ASSURE THAT POSITIVE BEARING IS MAINTAINED.
- SEALING OF BEAM SEATS: IF THE BEAM SEATS ARE SEALED WITH AN EPOXY OR NON-EPOXY SEALER PRIOR TO SETTING THE BEARINGS, DO NOT APPLY SEALER TO THE CONCRETE SURFACES UNDER THE PROPOSED BEARING LOCATIONS. IF THESE LOCATIONS ARE SEALED, REMOVE THE SEALER TO THE SATISFACTION OF THE ENGINEER PRIOR TO SETTING THE BEARINGS. THE DEPARTMENT WILL NOT PAY FOR THIS REMOVAL.
- THE FOLLOWING ABBREVIATIONS ARE USED:
 EL. = ELEVATION
 TYP. = TYPICAL
 SER. = SERIES
 CLR. = CLEARANCE
 N.F. = NEAR FACE
 F.F. = FAR FACE
 E.F. = EACH FACE

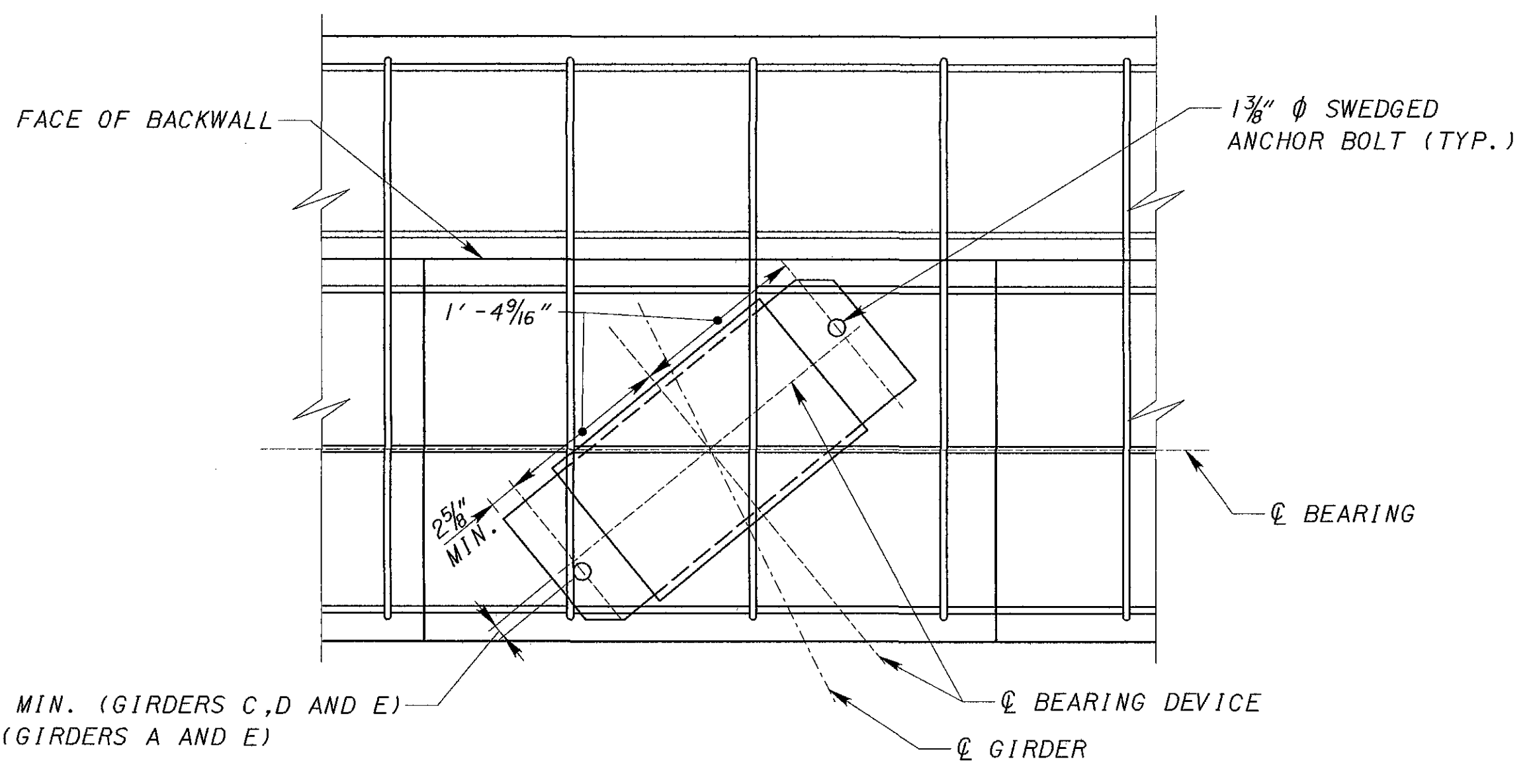
LAP LENGTH TABLE	
BAR	REQUIRED LAP LENGTH
#5	2'-0"
#6	2'-4"
#8	4'-0"



FOOTING PLAN



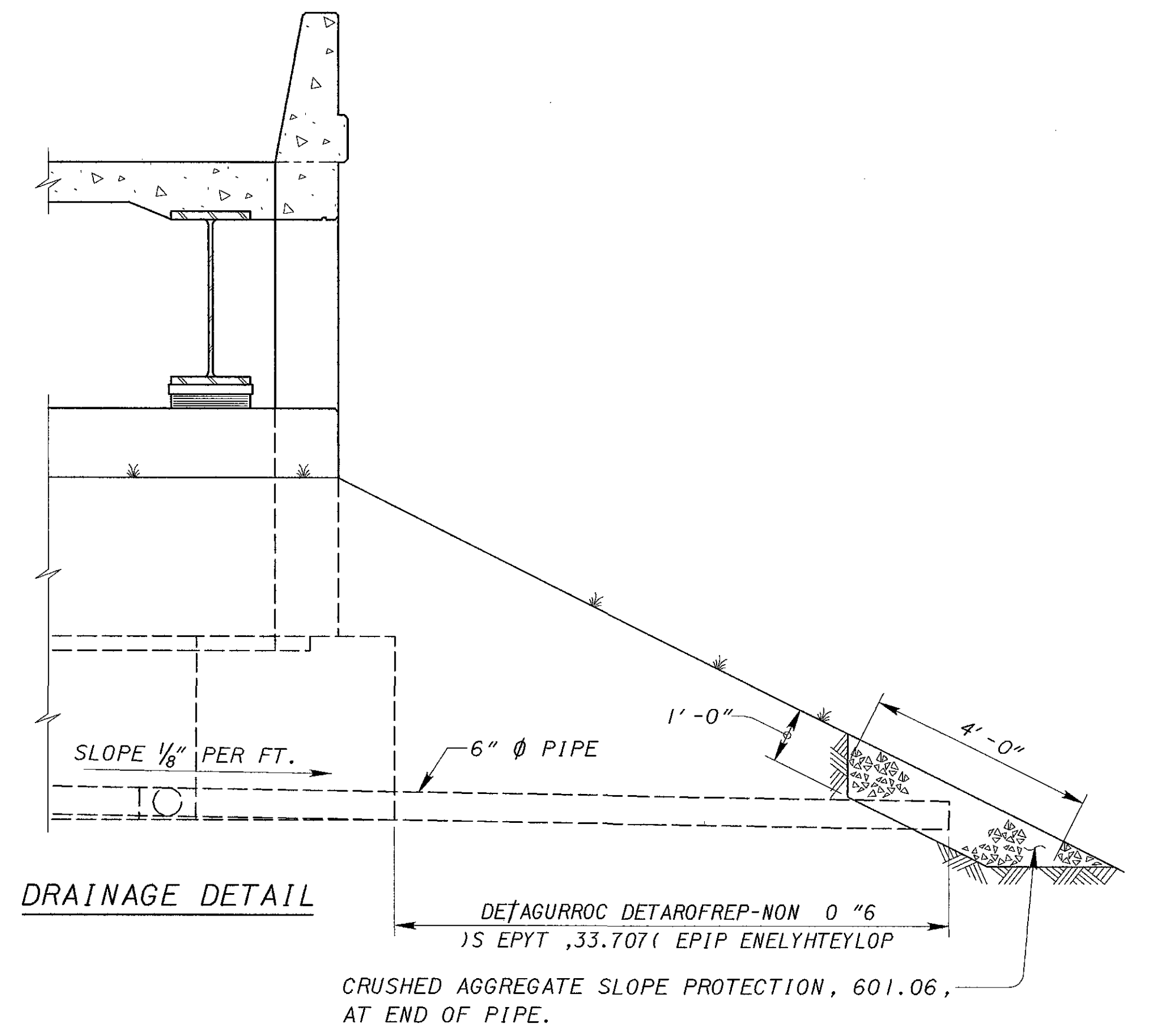
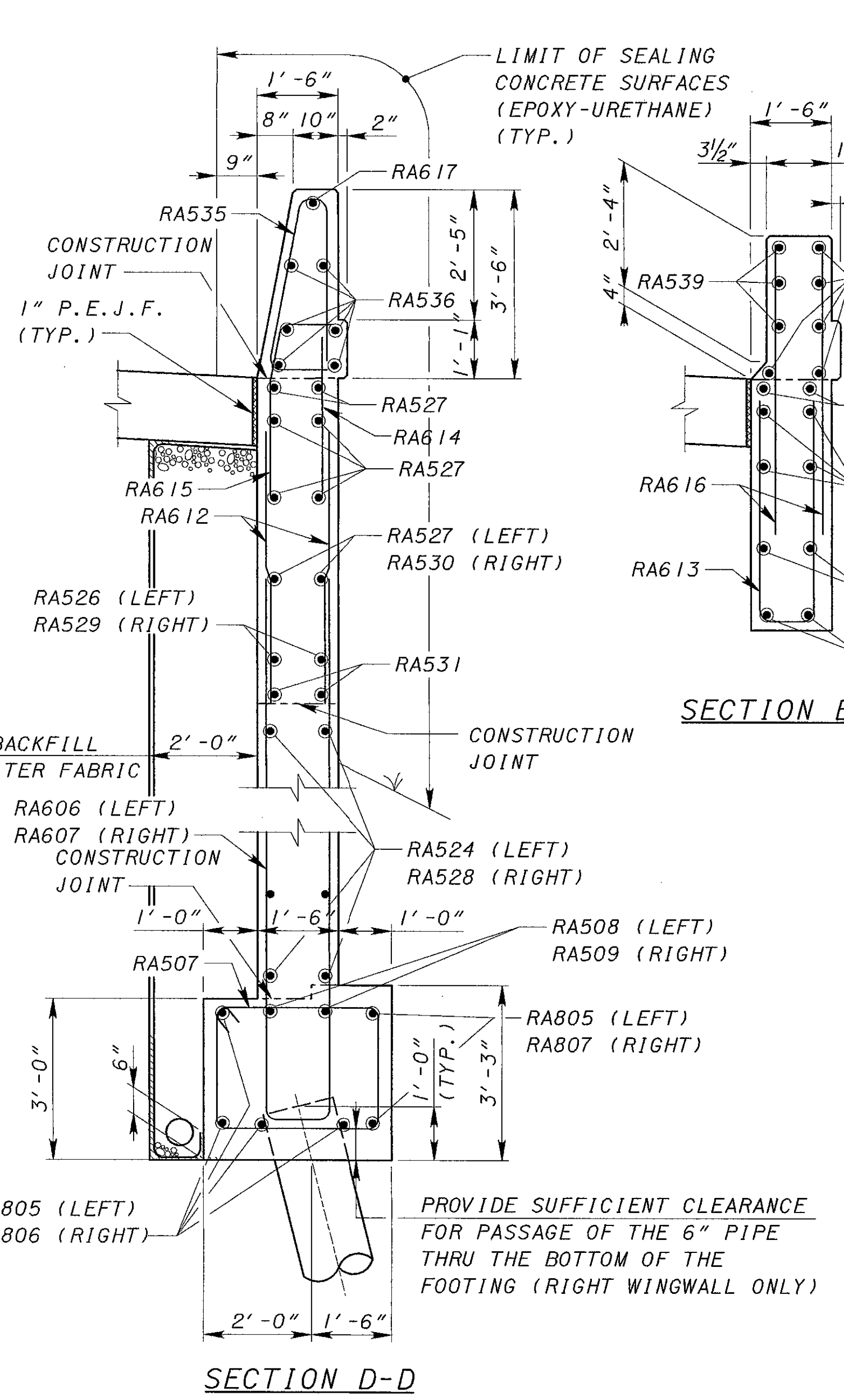
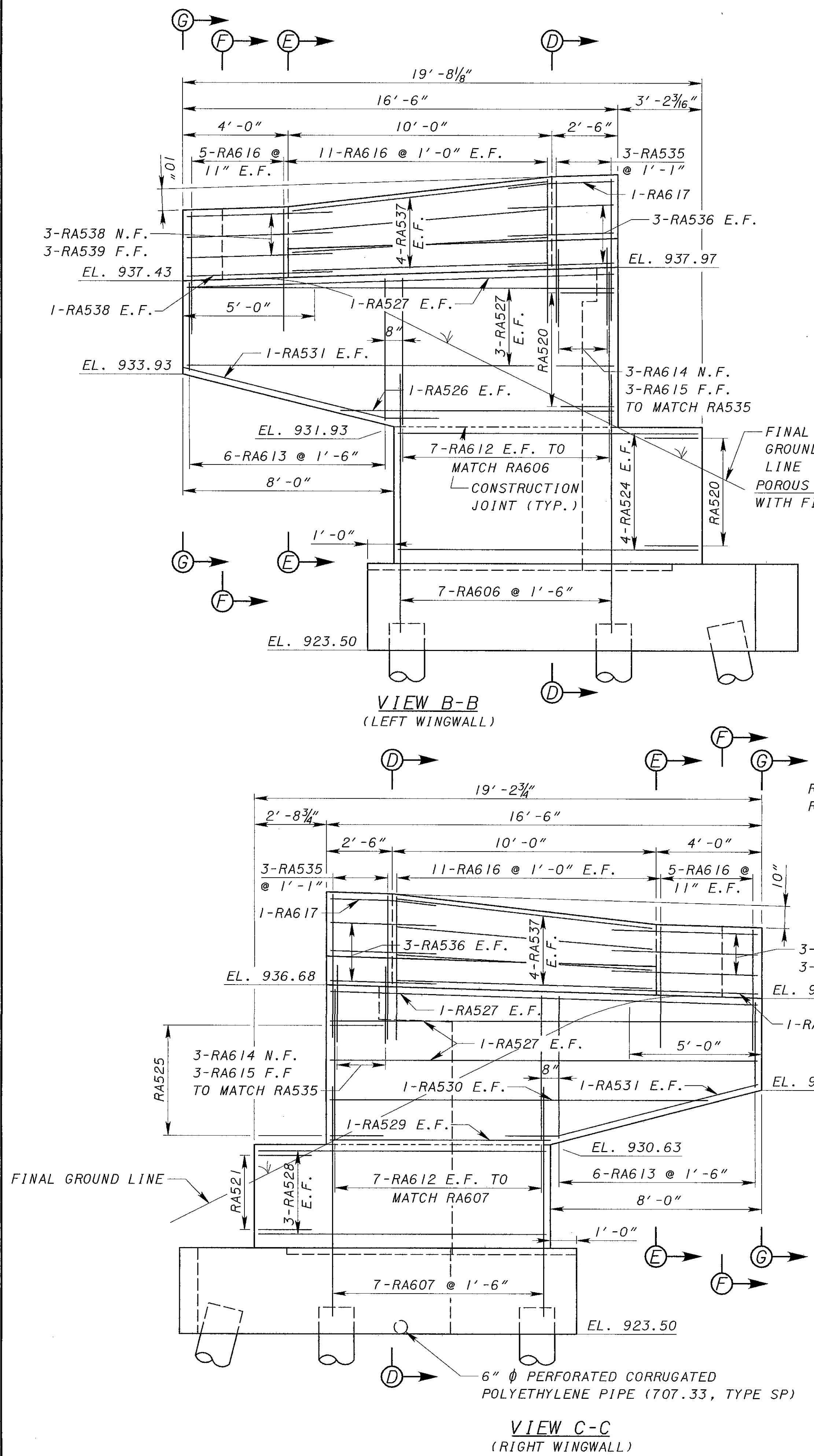
SECTION A-A



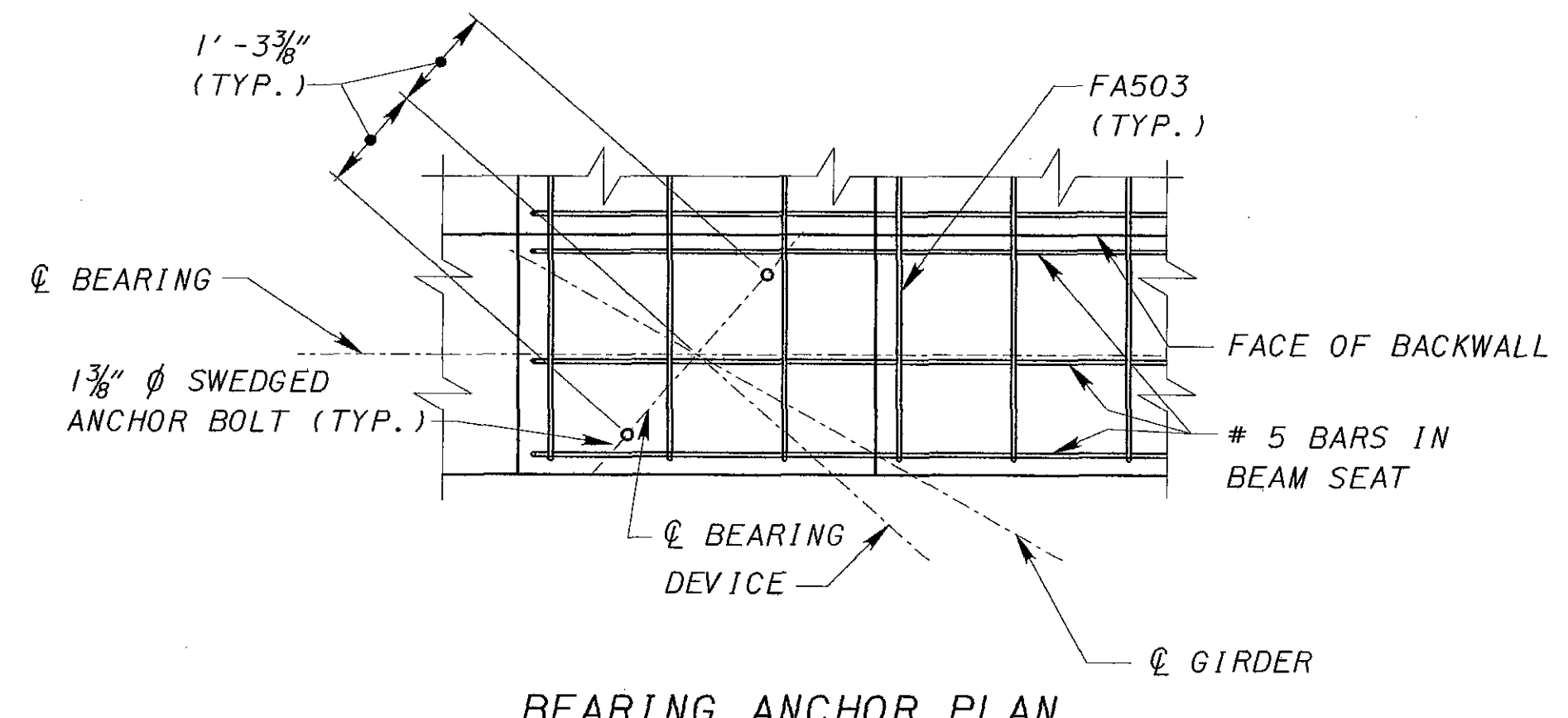
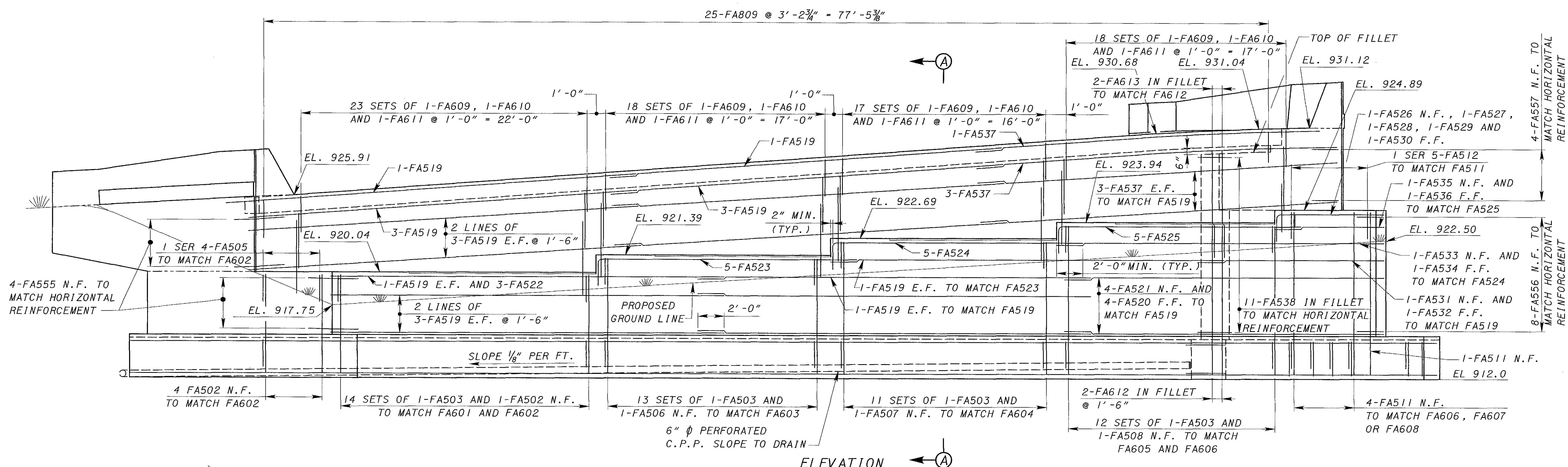
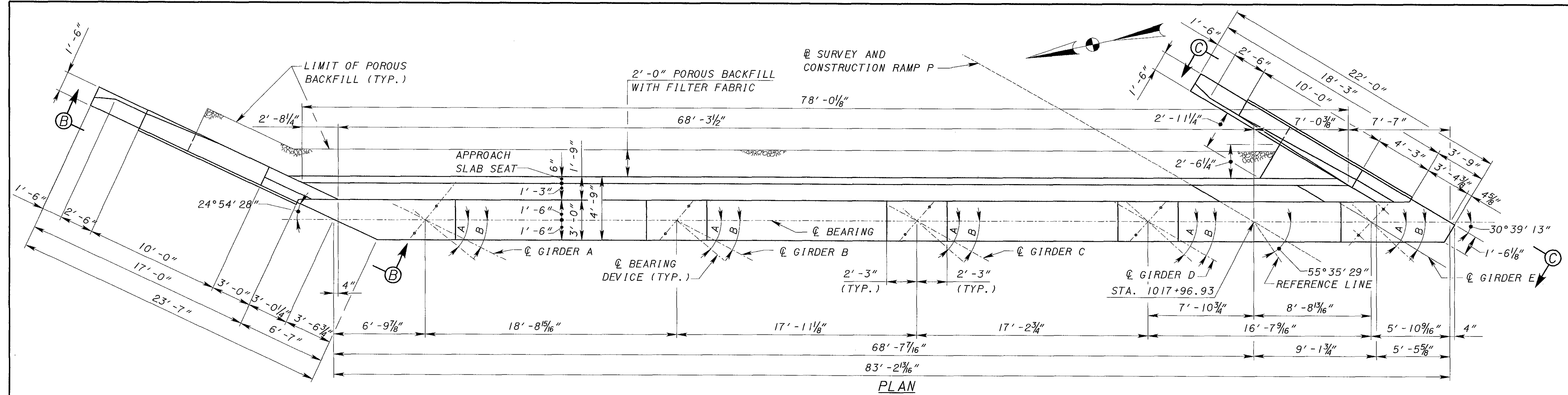
BEARING ANCHOR PLAN
 (SEE NOTE 1)

NOTES:

1. ACCURATELY PLACE REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES OR THE PRE-SETTING OF BEARING ANCHORS. REFER TO BEARING DETAILS SHOWN ON SHEET [294].
2. FOR ABUTMENT PLAN AND ELEVATION, SEE SHEET [74].
3. FOR ADDITIONAL NOTES, SEE SHEET [74].



- NOTES:**
- FOR ABUTMENT DETAILS, SEE SHEETS [7/41] AND [8/41].
 - FOR ADDITIONAL NOTES, SEE SHEET [7/41].



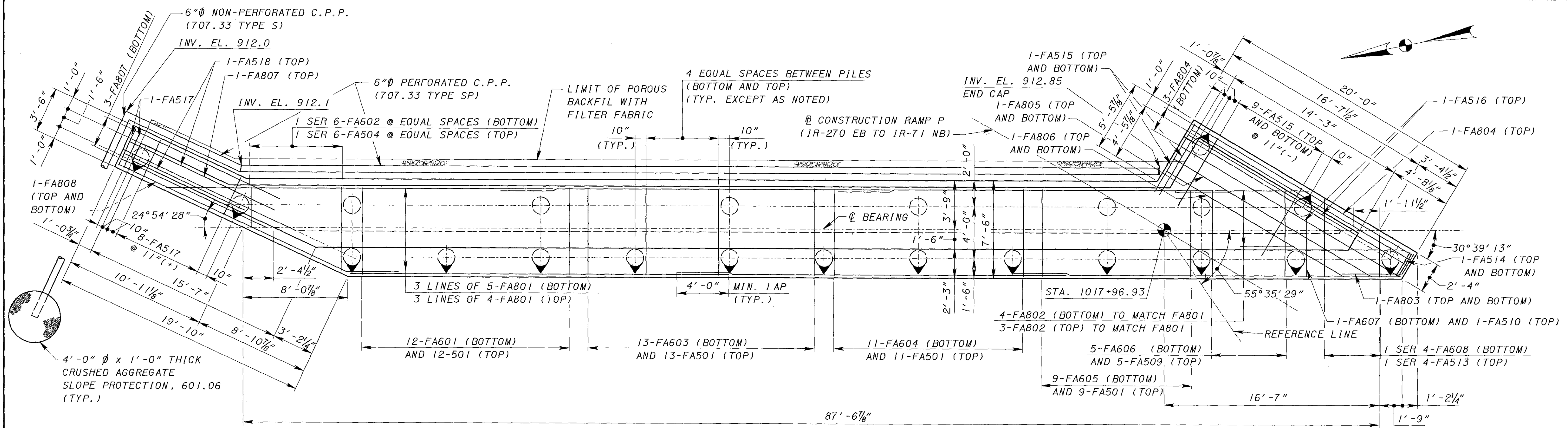
LAP LENGTH TABLE

BAR	REQUIRED LAP LENGTH
#5	2'-0"
#6	2'-4"
#8	4'-0"

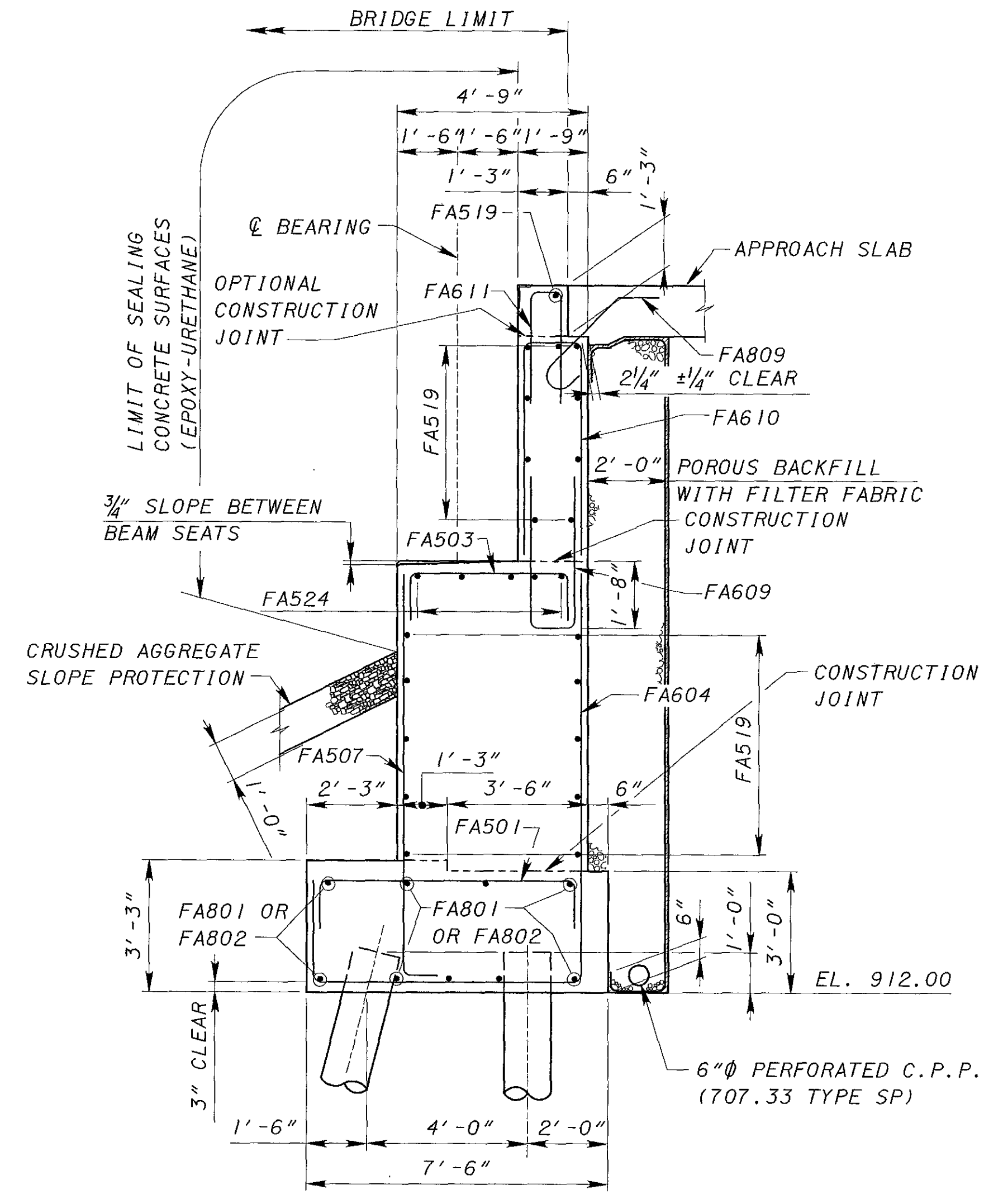
TABLE OF ANGLES

GIRDER	ANGLE	
	A	B
A	38°58'57"	26°16'28"
B	40°09'35"	27°39'35"
C	41°16'06"	28°57'09"
D	42°19'01"	30°09'55"
E	43°18'43"	31°18'31"

- NOTES:
- FOR FOOTING PLAN AND SECTION A-A SEE SHEET [11/41].
 - FOR VIEWS B-B & C-C AND WINGWALL DETAILS, SEE SHEET [12/41].
 - FOR BEARING DEVICE DETAILS SEE SHEET [29/41].
 - THE FOLLOWING ABBREVIATIONS ARE USED:
 EL. = ELEVATION
 SER = SERIES
 TYP. = TYPICAL
 N.F. = NEAR FACE
 F.F. = FAR FACE
 E.F. = EACH FACE
 MIN. = MINIMUM
 CLR. = CLEARANCE
 - FOR ADDITIONAL NOTES SEE SHEET [7/41].



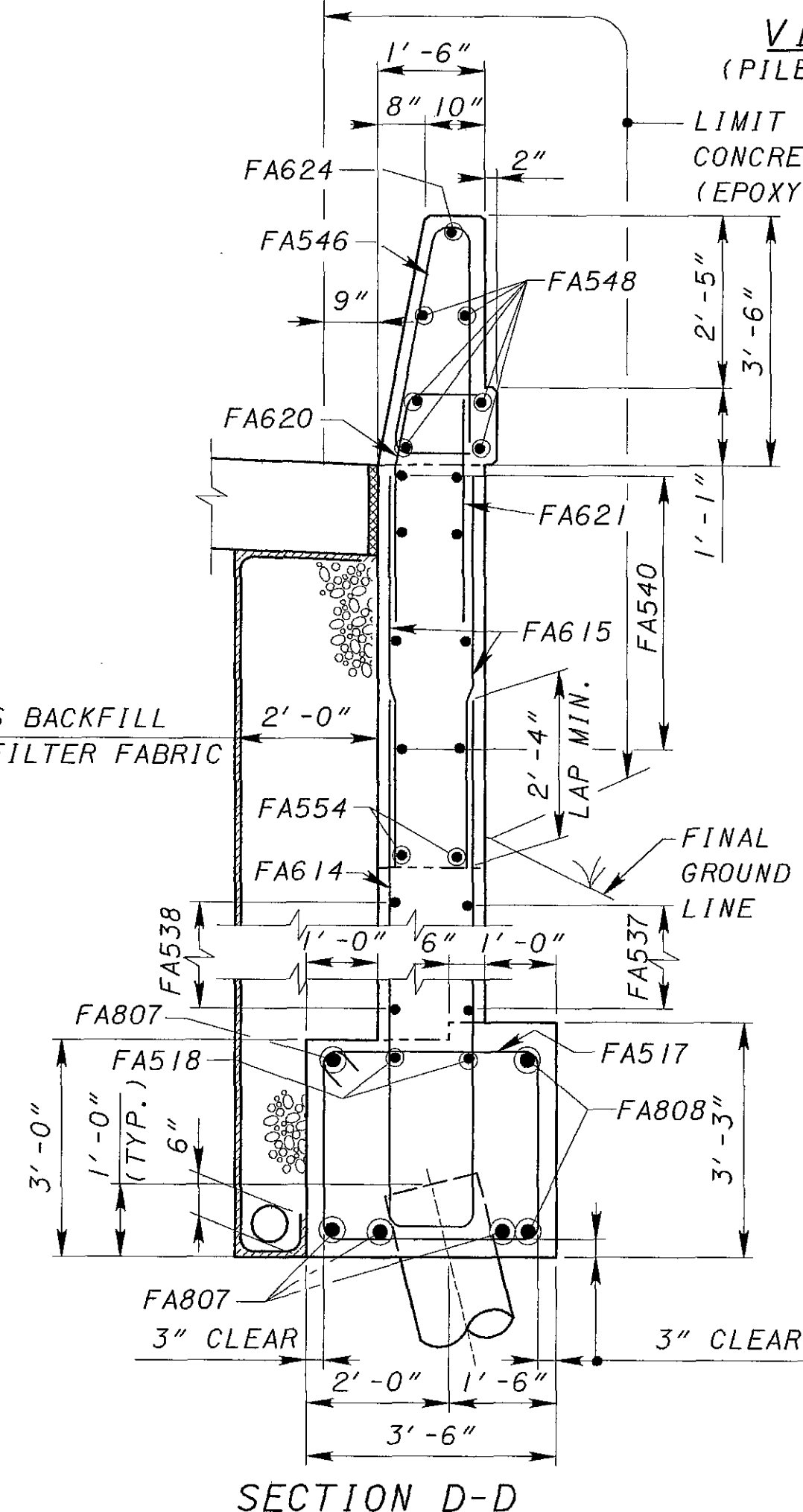
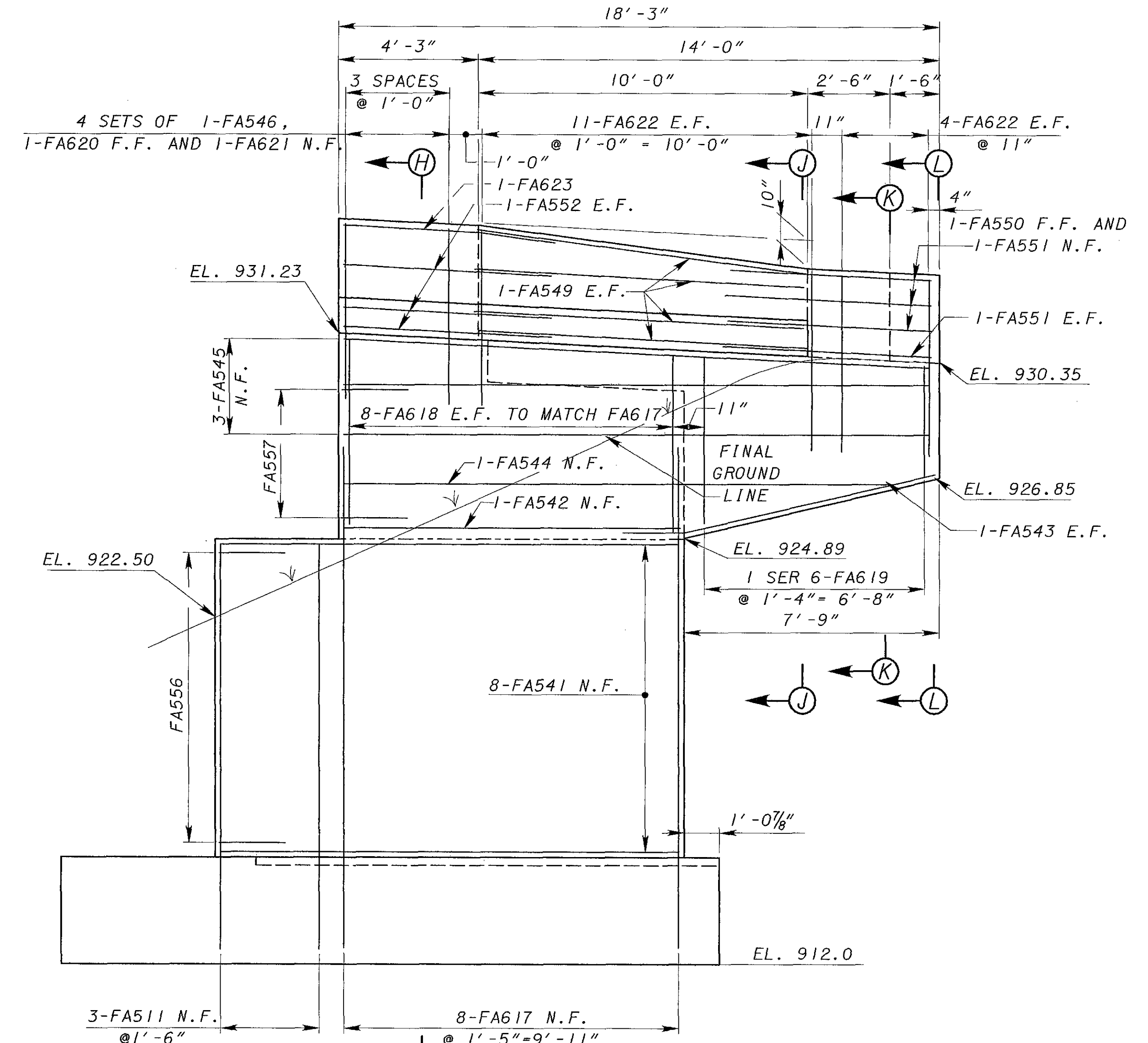
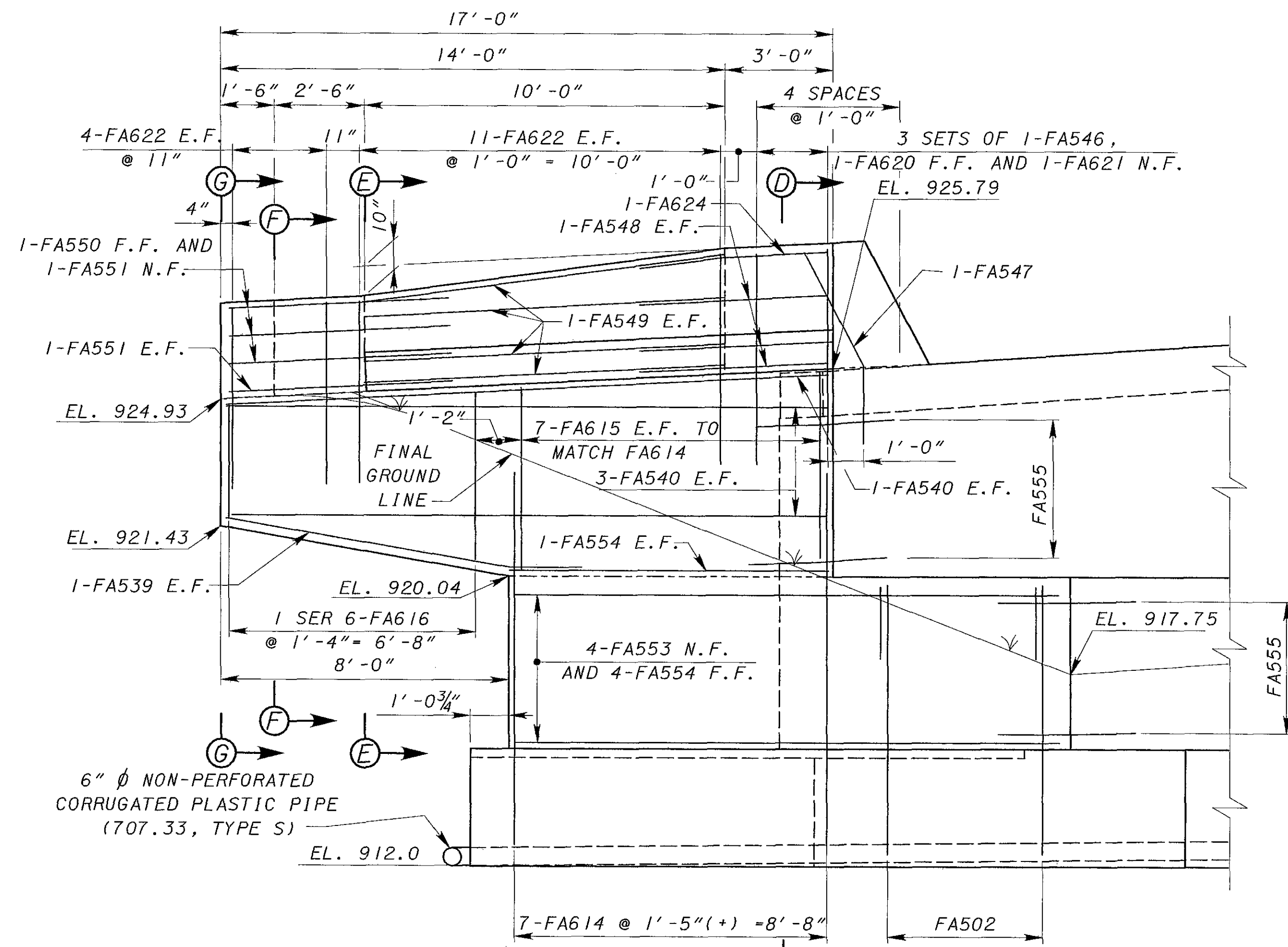
FOOTING PLAN



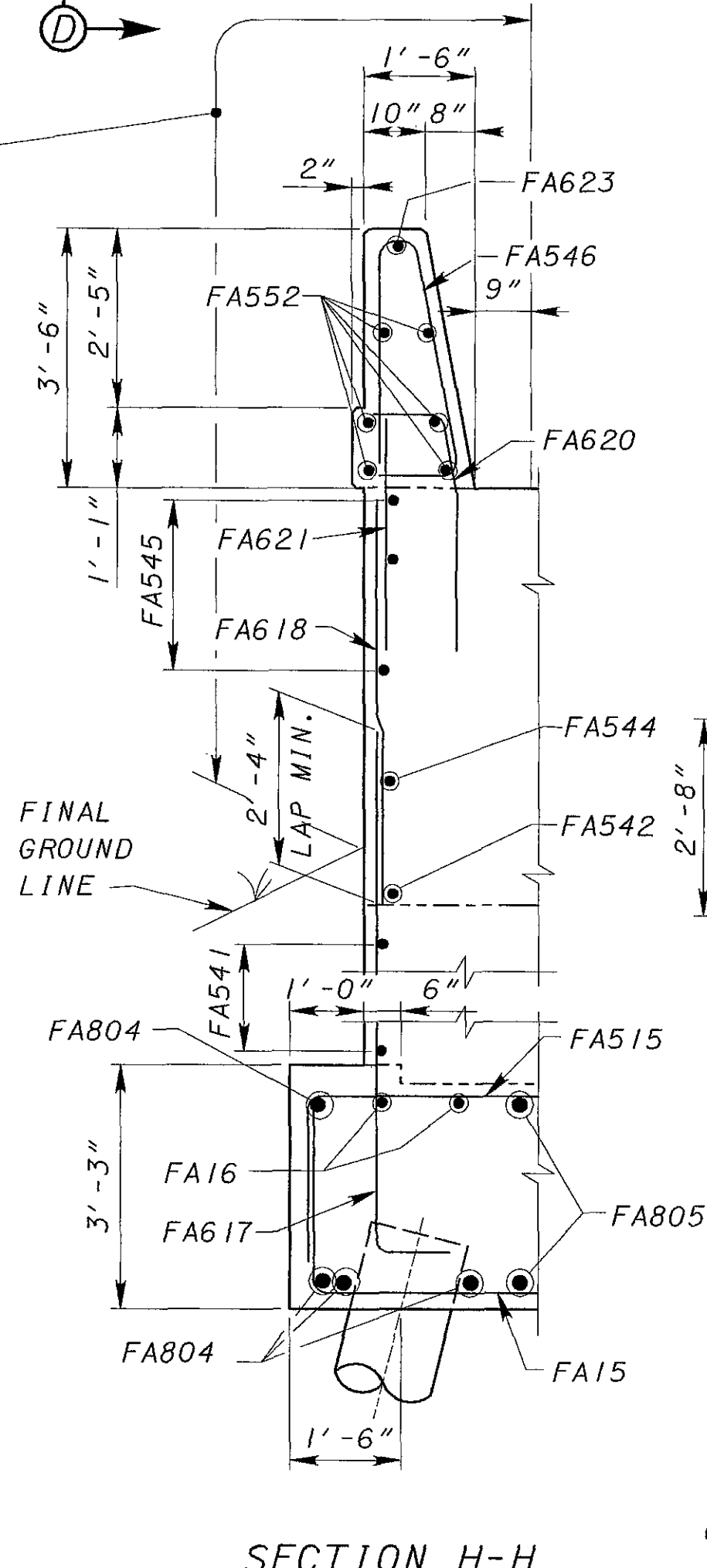
SECTION A-A

NOTES:
 1. FOR ABUTMENT NOTES SEE SHEET 741.

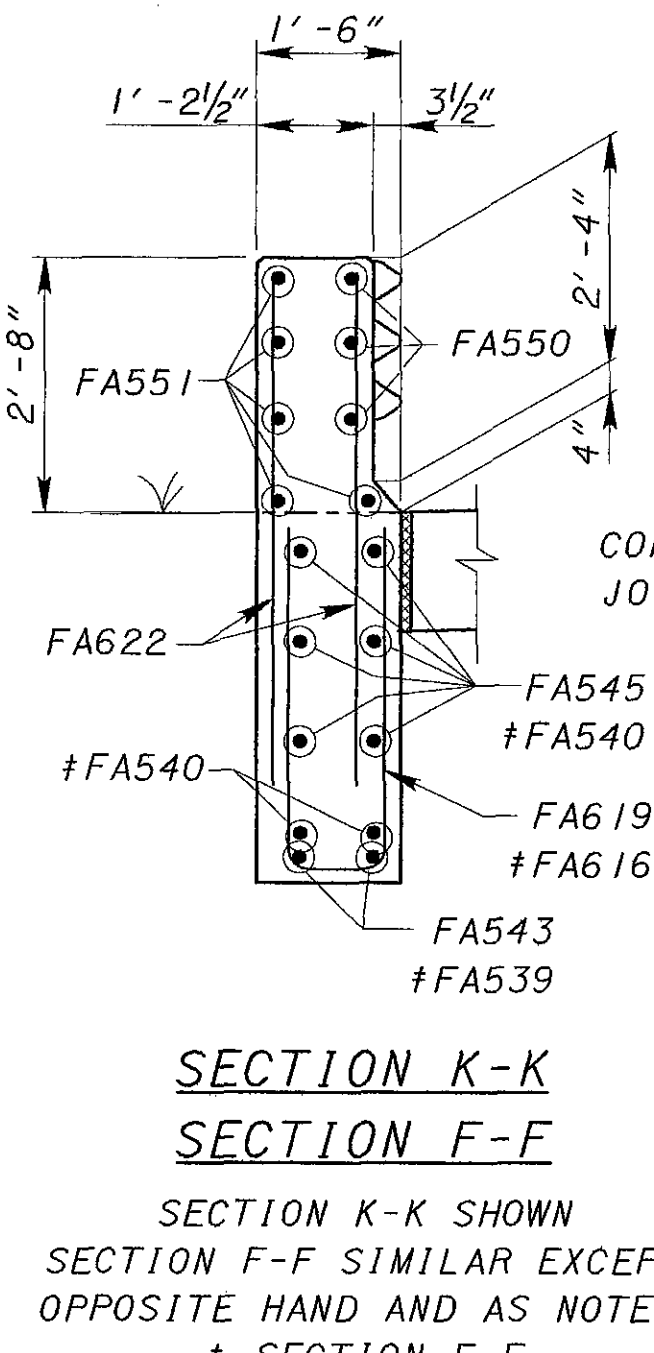
DATE	7/19/05
REVIEWED	REC
STRUCTURE FILE NUMBER	2511452
DRAWN	RCK
REVISION	NFF
DESIGNED	MLR
CHECKED	NFF



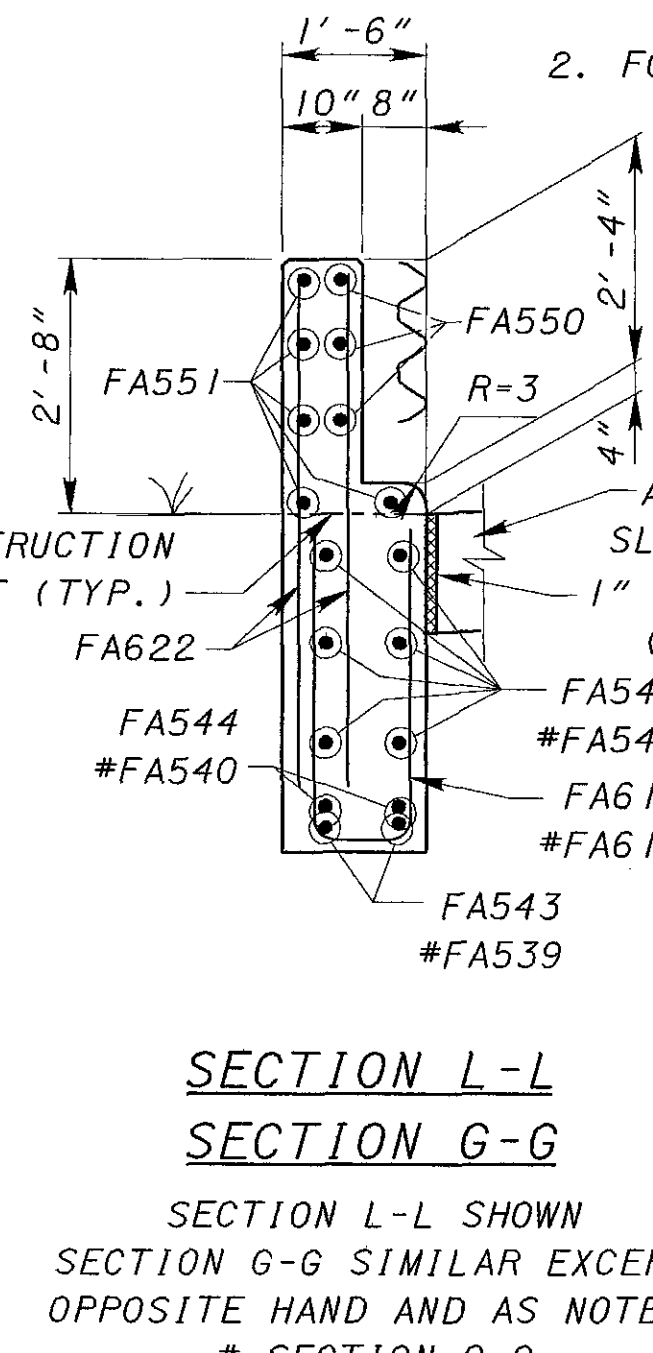
VIEW B-B
(PILES NOT SHOWN)
LIMIT OF SEALING
CONCRETE SURFACES
(EPOXY - URETHANE)



SECTION J-J
SECTION E-E
SECTION J-J SHOWN
SECTION E-E SIMILAR EXCEPT
OPPOSITE HAND AND AS NOTED.
* SECTION E-E

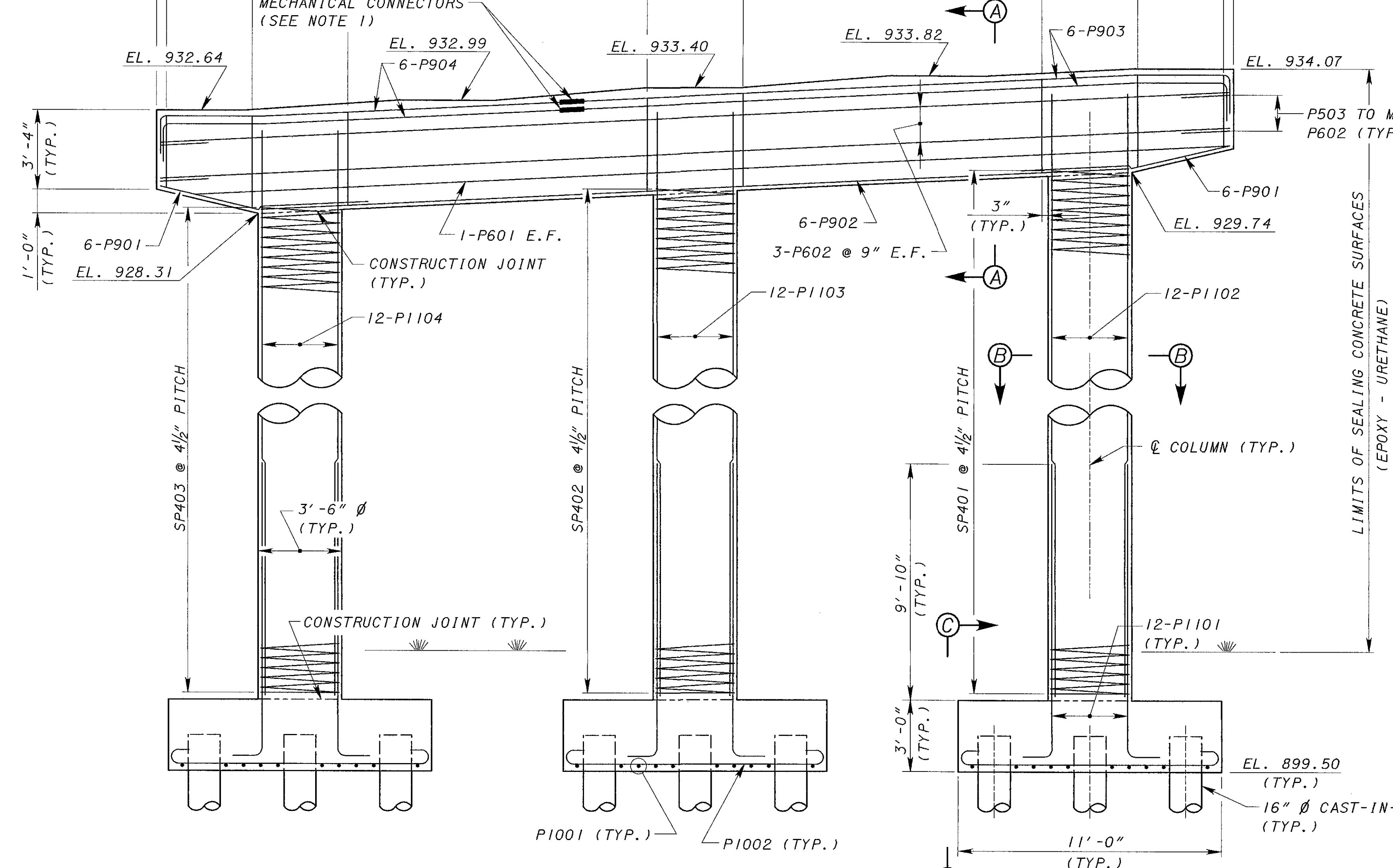
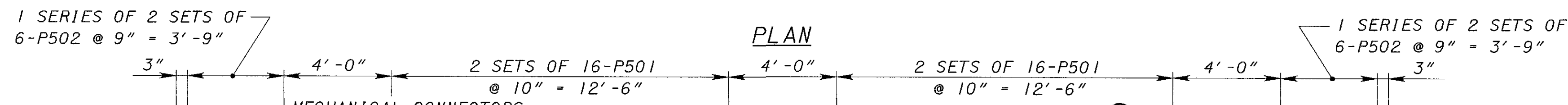
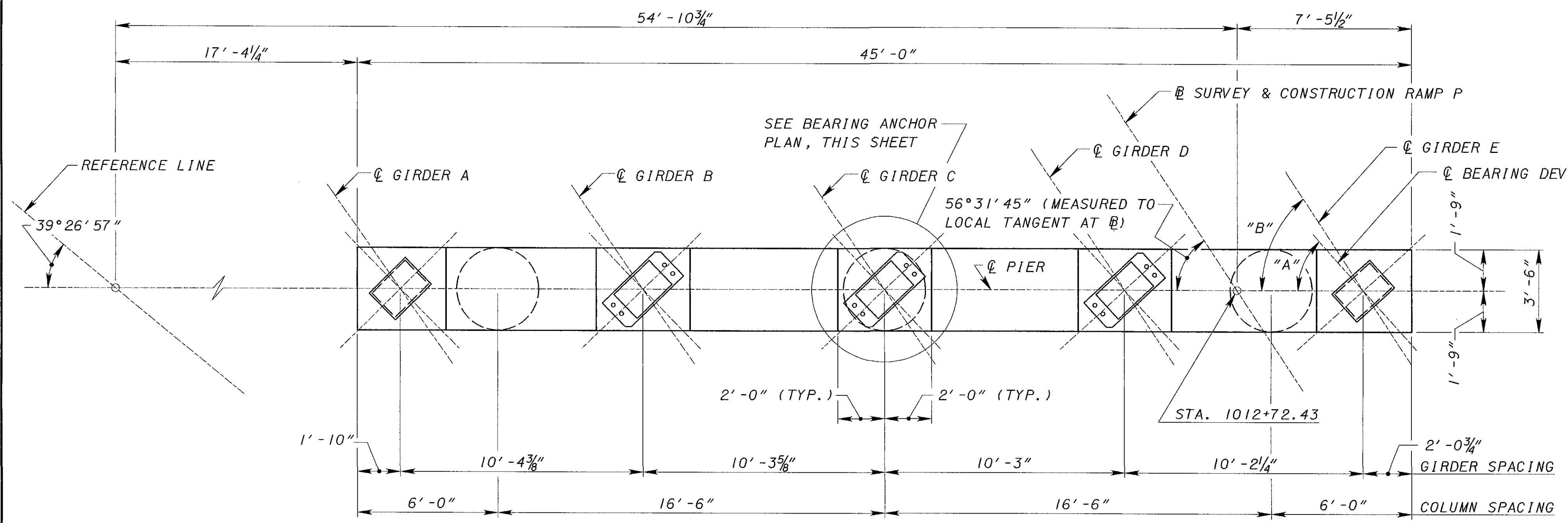


SECTION K-K
SECTION F-F
SECTION K-K SHOWN
SECTION F-F SIMILAR EXCEPT
OPPOSITE HAND AND AS NOTED.
† SECTION F-F

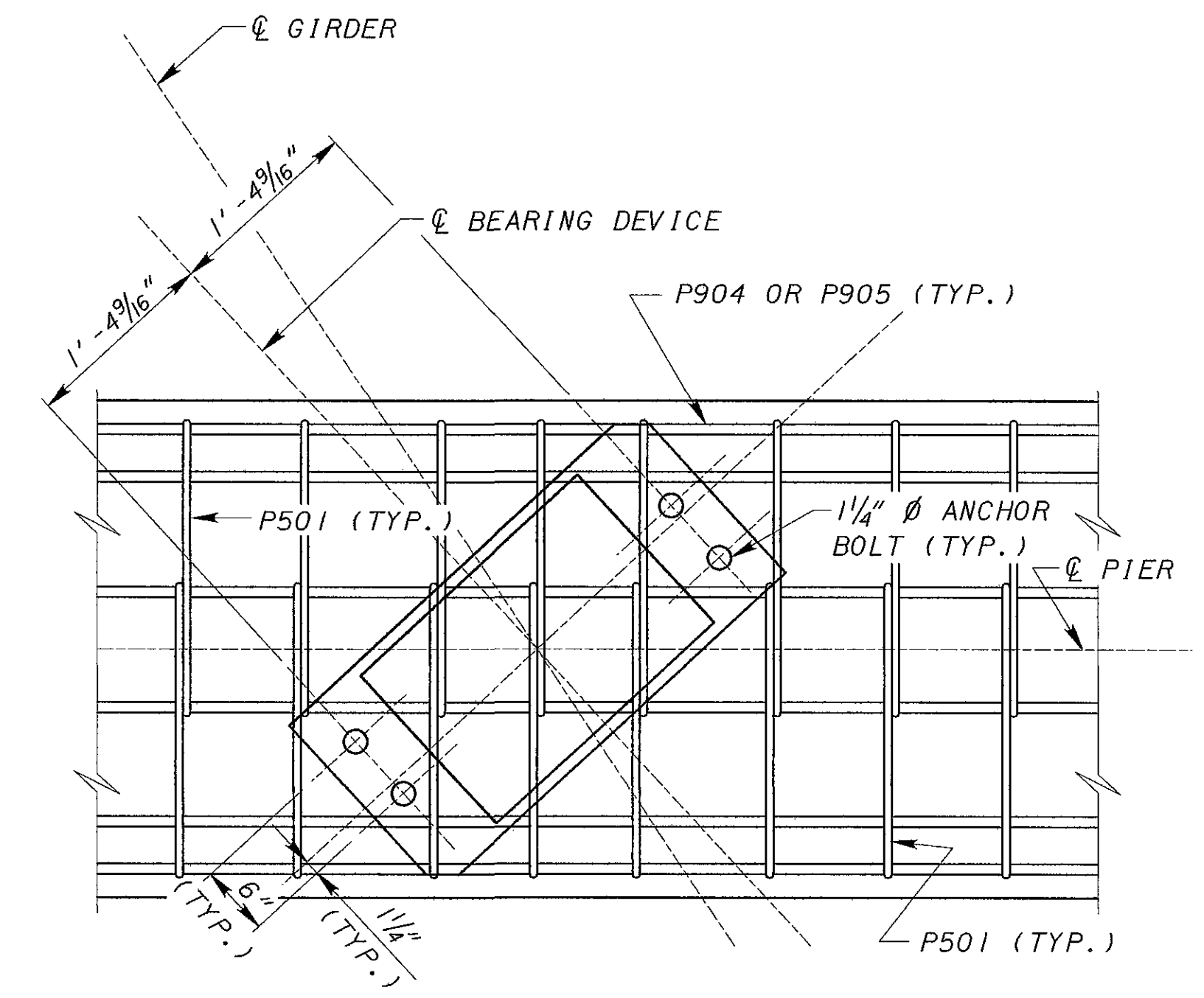


SECTION L-L
SECTION G-G
SECTION L-L SHOWN
SECTION G-G SIMILAR EXCEPT
OPPOSITE HAND AND AS NOTED.
SECTION G-G

- NOTES:**
1. FOR ABUTMENT DETAILS, SEE SHEETS 10/41 AND 11/41.
 2. FOR ADDITIONAL NOTES, SEE SHEET 7/41.



ELEVATION

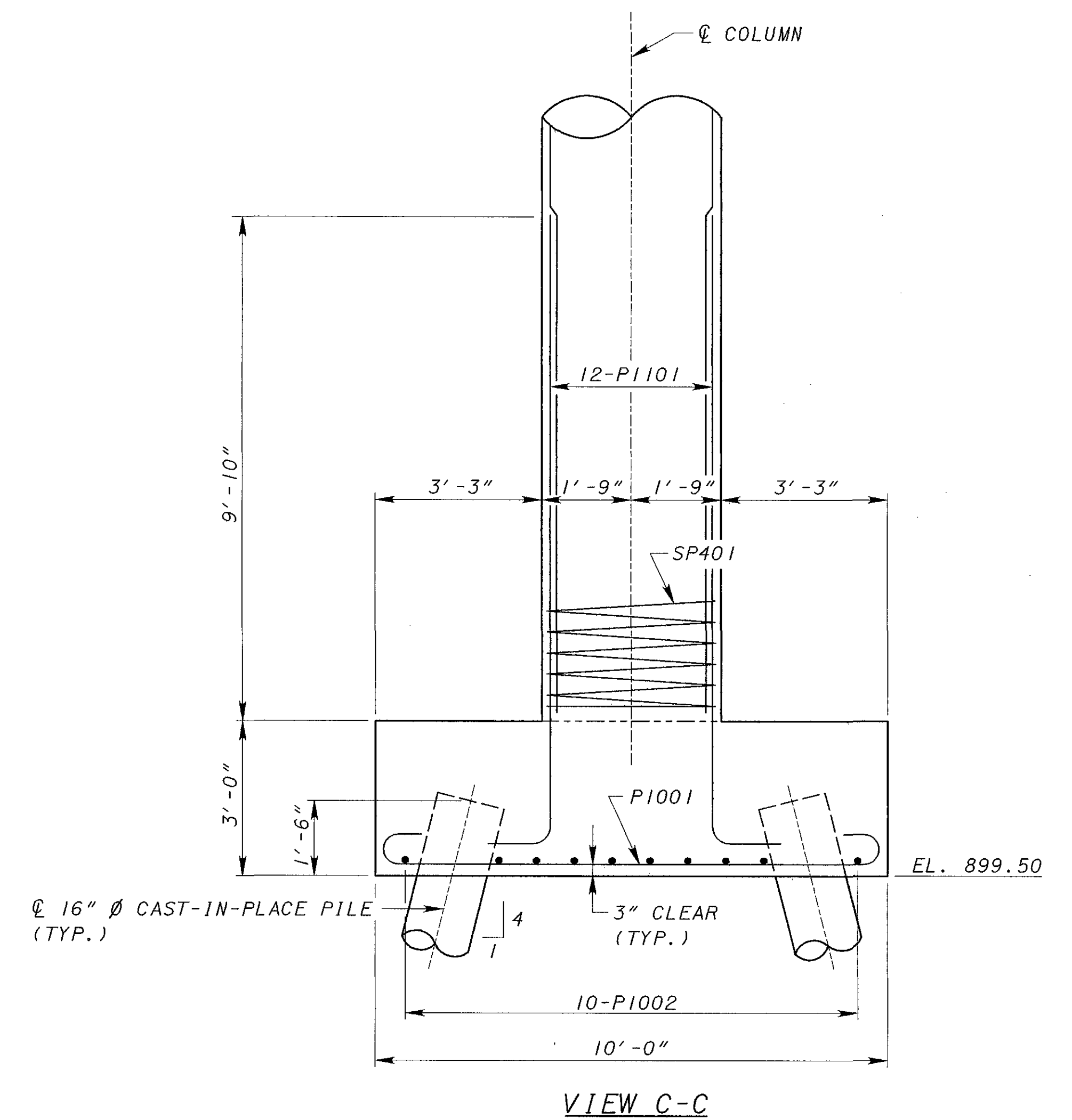
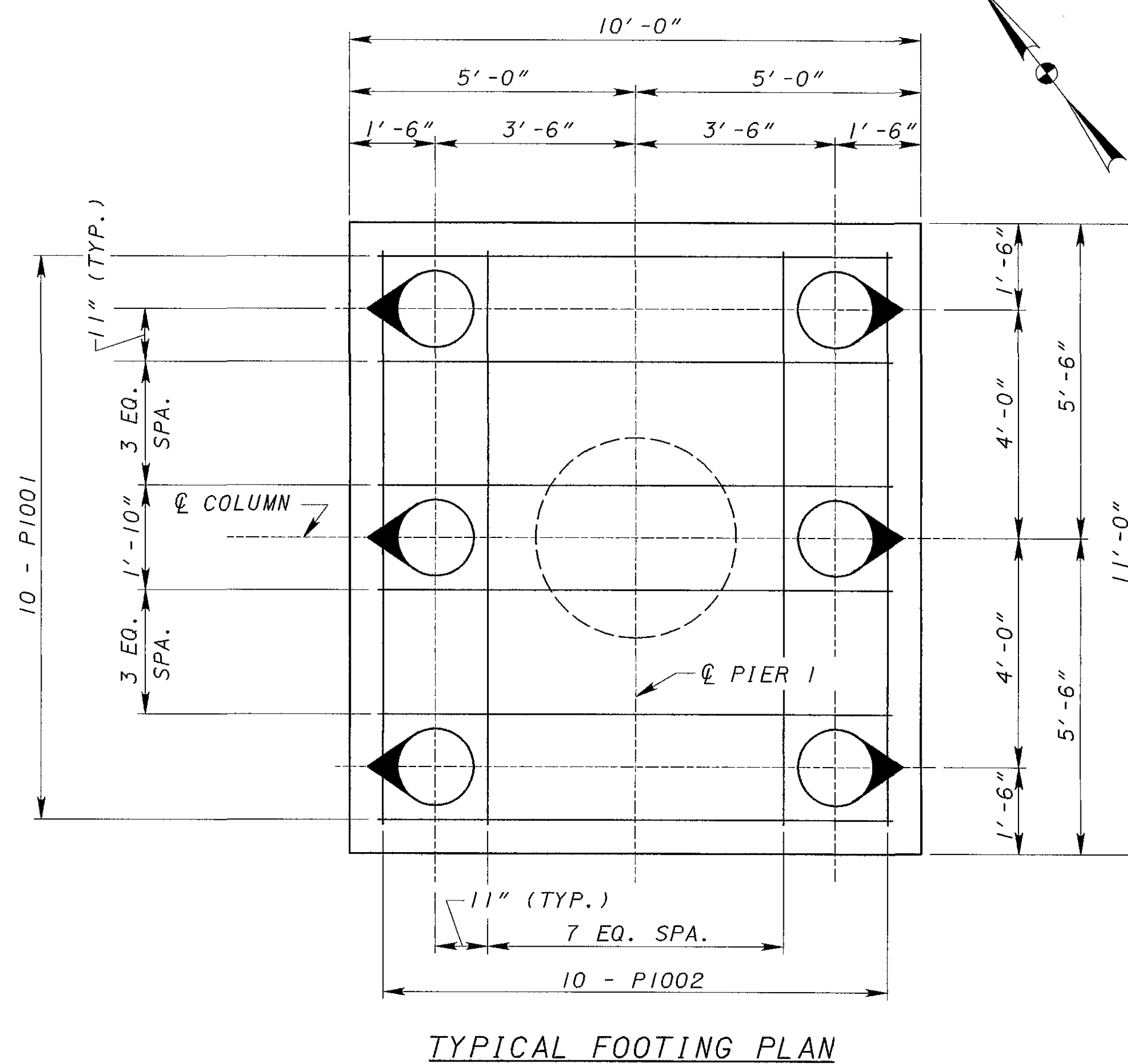
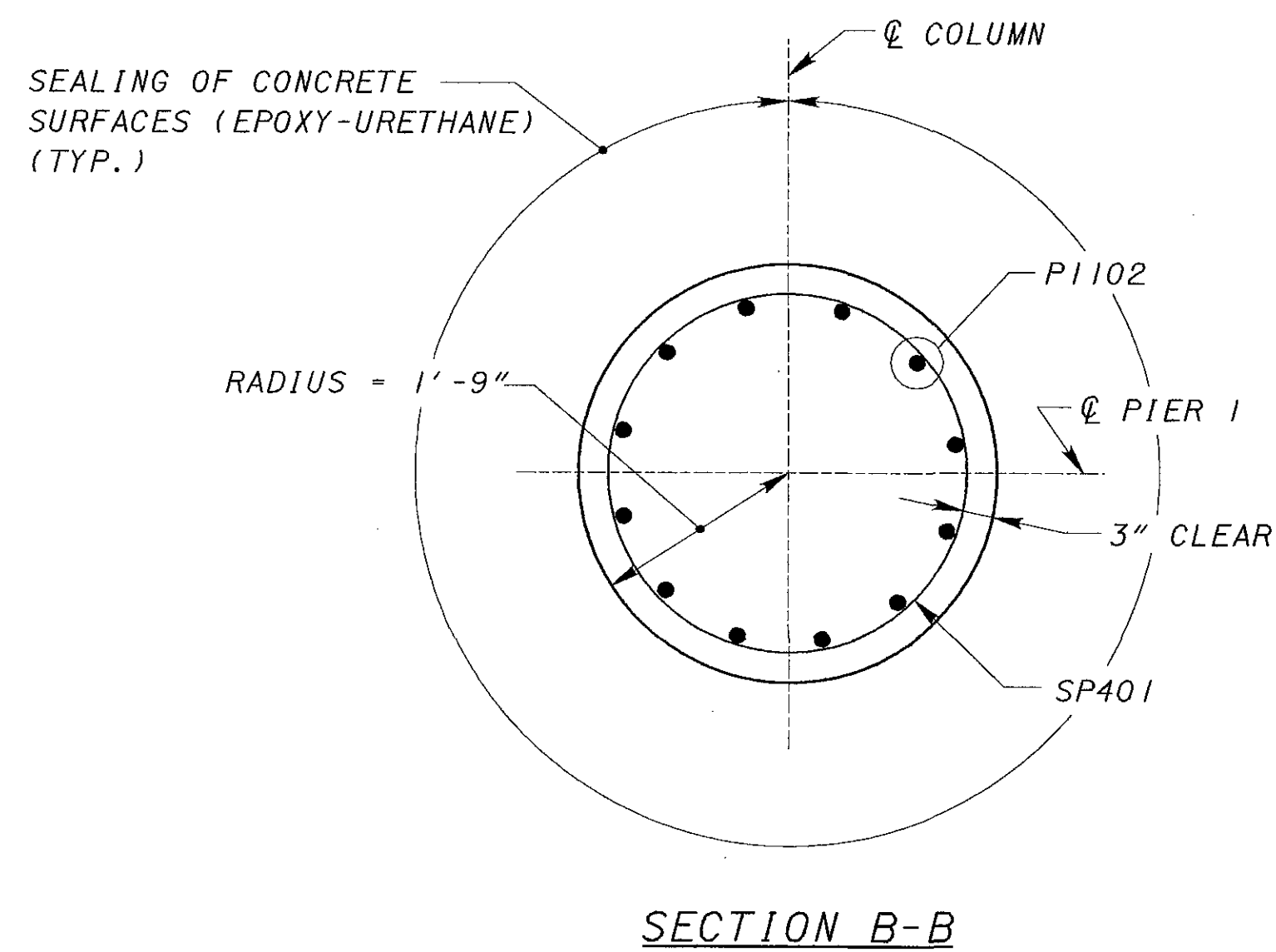
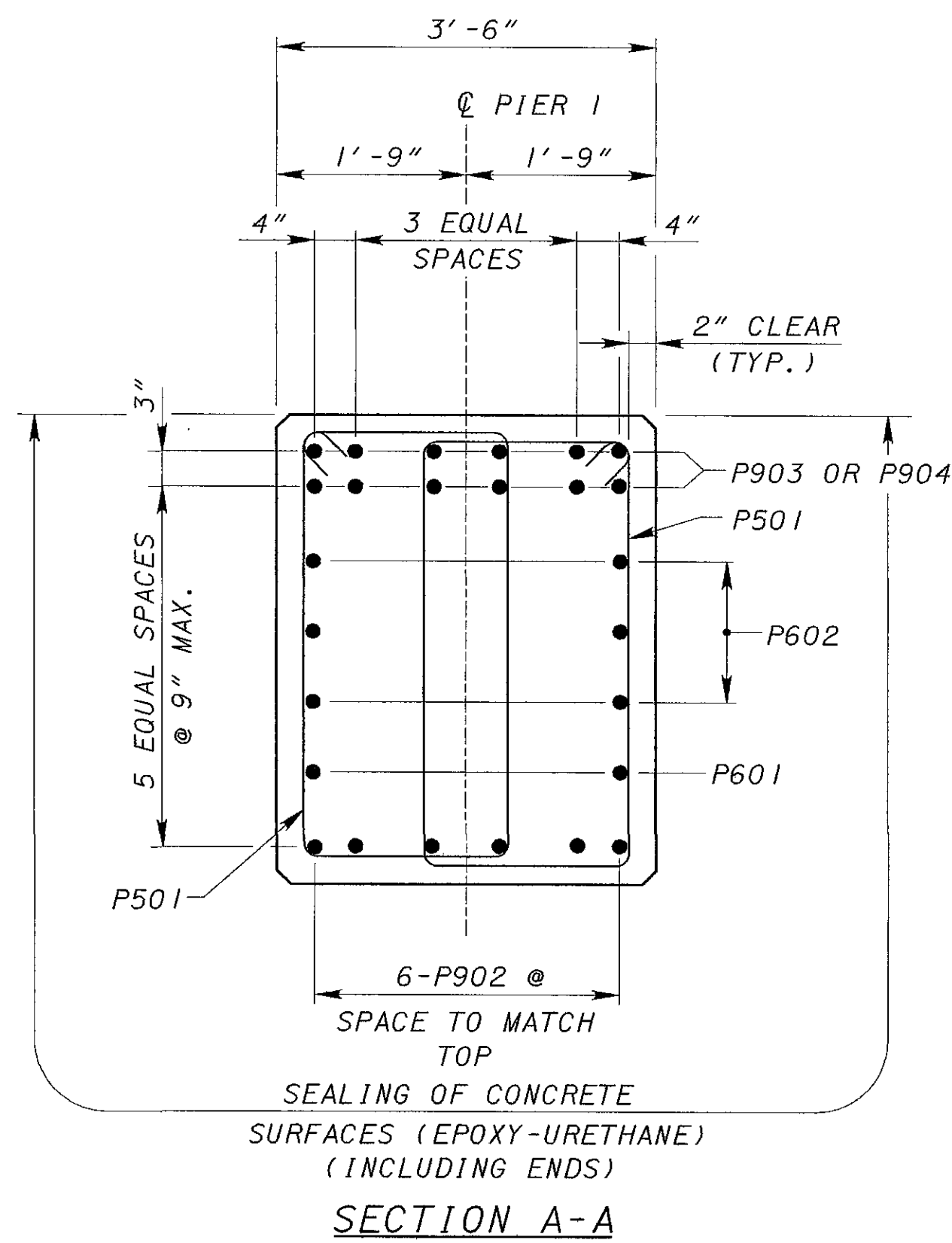


BEARING ANCHOR PLAN (SEE NOTE 2)

BAR	REQUIRED LAP LENGTH
#9	6'-5"

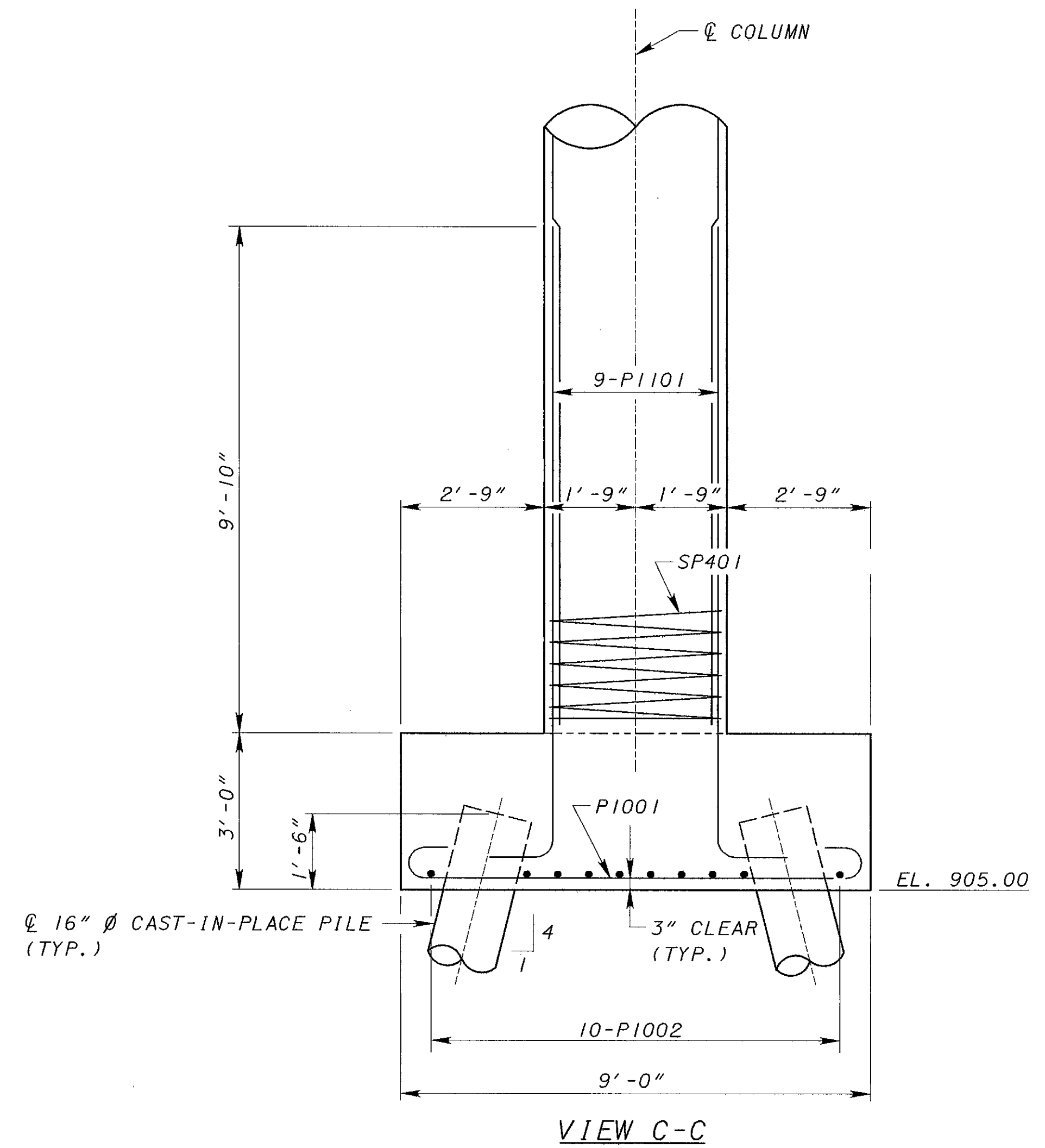
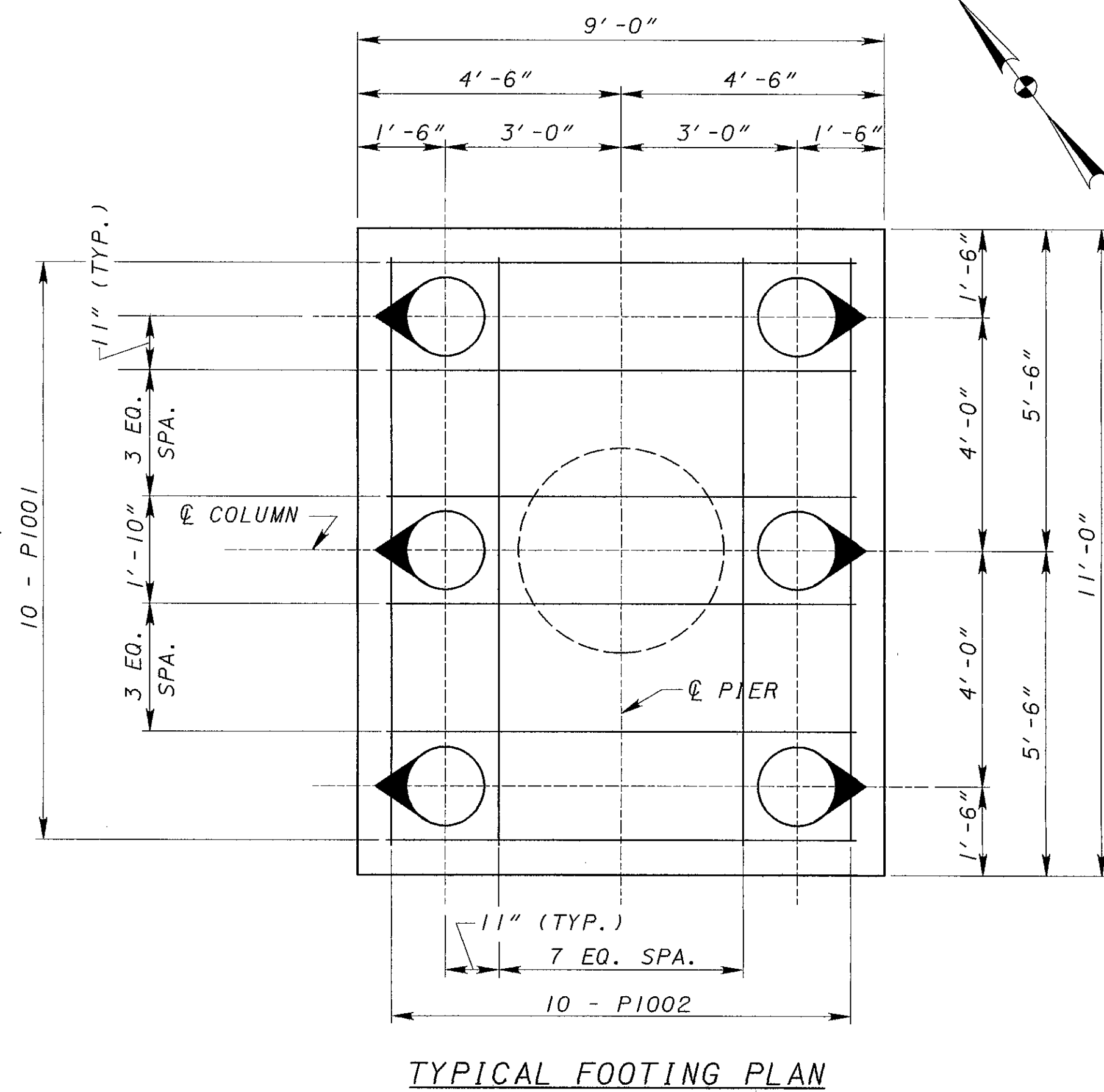
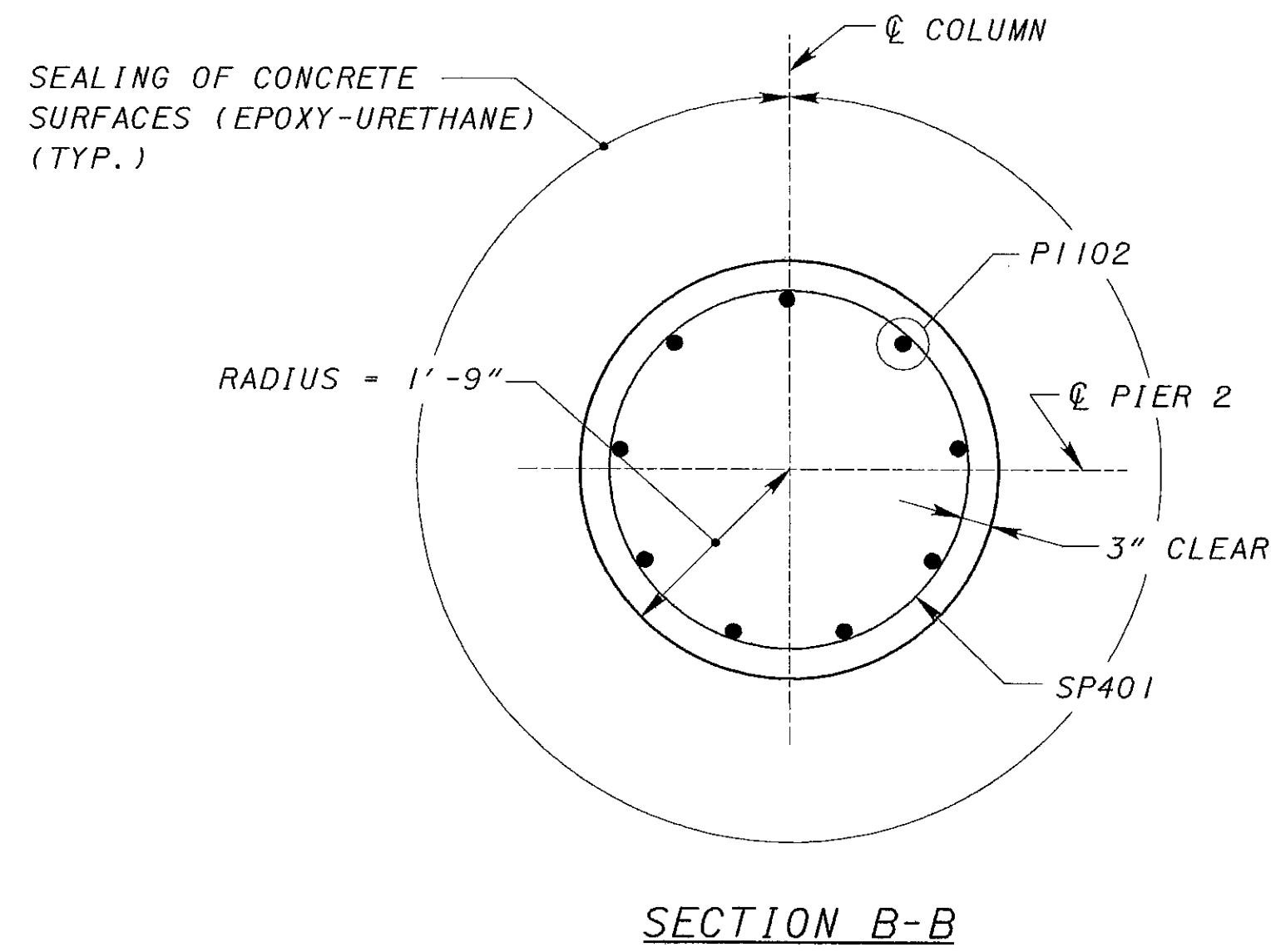
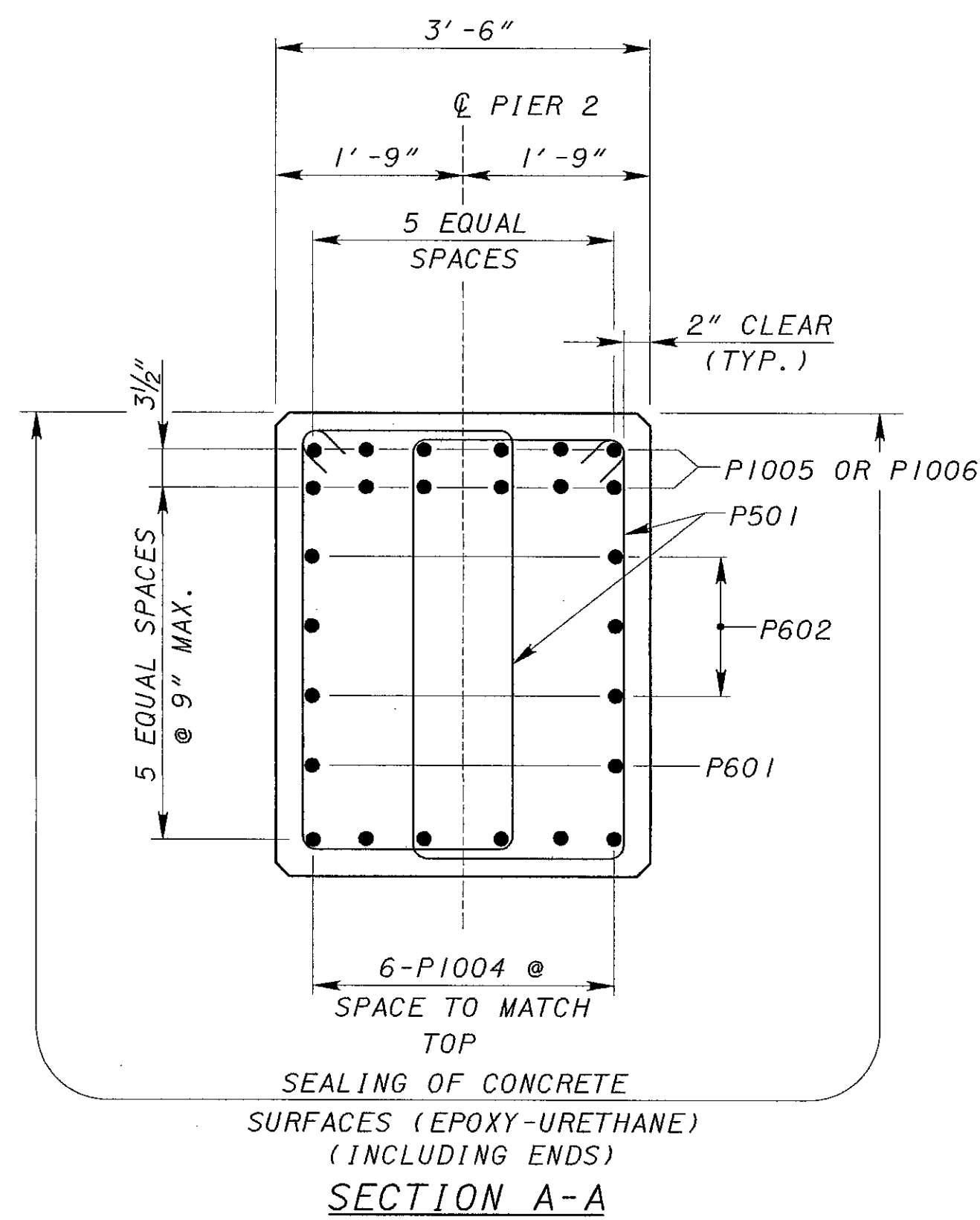
NOTES:

- MECHANICAL CONNECTORS SHALL DEVELOP A MINIMUM ULTIMATE STRENGTH OF 125% OF THE REQUIRED YIELD STRENGTH OF THE REINFORCEMENT THEY CONNECT. MECHANICAL SPLICES SHALL MEET THE REQUIREMENTS OF ITEM 509 AND SHALL BE STAGGERED.
- ACCURATELY PLACE REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES OR THE PRE-SETTING OF BEARING ANCHORS.
- FOR MASONRY PLATE AND BEARING DETAILS, SEE SHEETS [28/41] & [29/41].
- FOR PILE LAYOUT, SEE SHEET [6/41].
- FOR SECTIONS A-A AND B-B, VIEW C-C AND TYPICAL FOOTING PLAN, SEE SHEET [14/41].
- FOR REINFORCING STEEL LIST, SEE SHEET [40/41].
- THE FOLLOWING ABBREVIATIONS ARE USED:
E.F. - EACH FACE
EQ. - EQUAL
MAX. - MAXIMUM
SPA. - SPACES
TYP. - TYPICAL
- TOP OF THE COLUMN SPIRAL REINFORCEMENT SHALL BE EMBEDDED A MINIMUM 2" INTO THE PIER CAP CONCRETE.

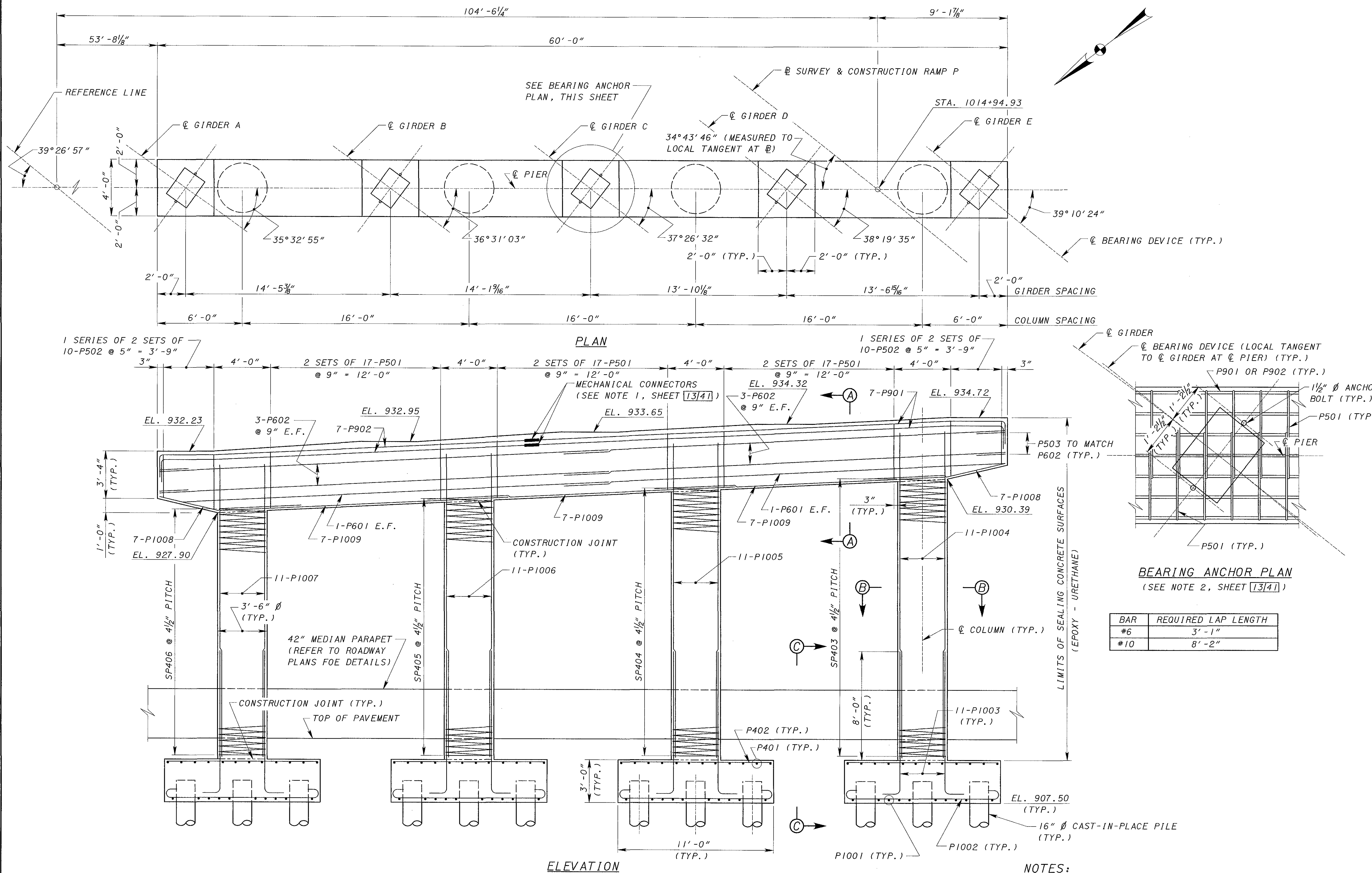


NOTES:

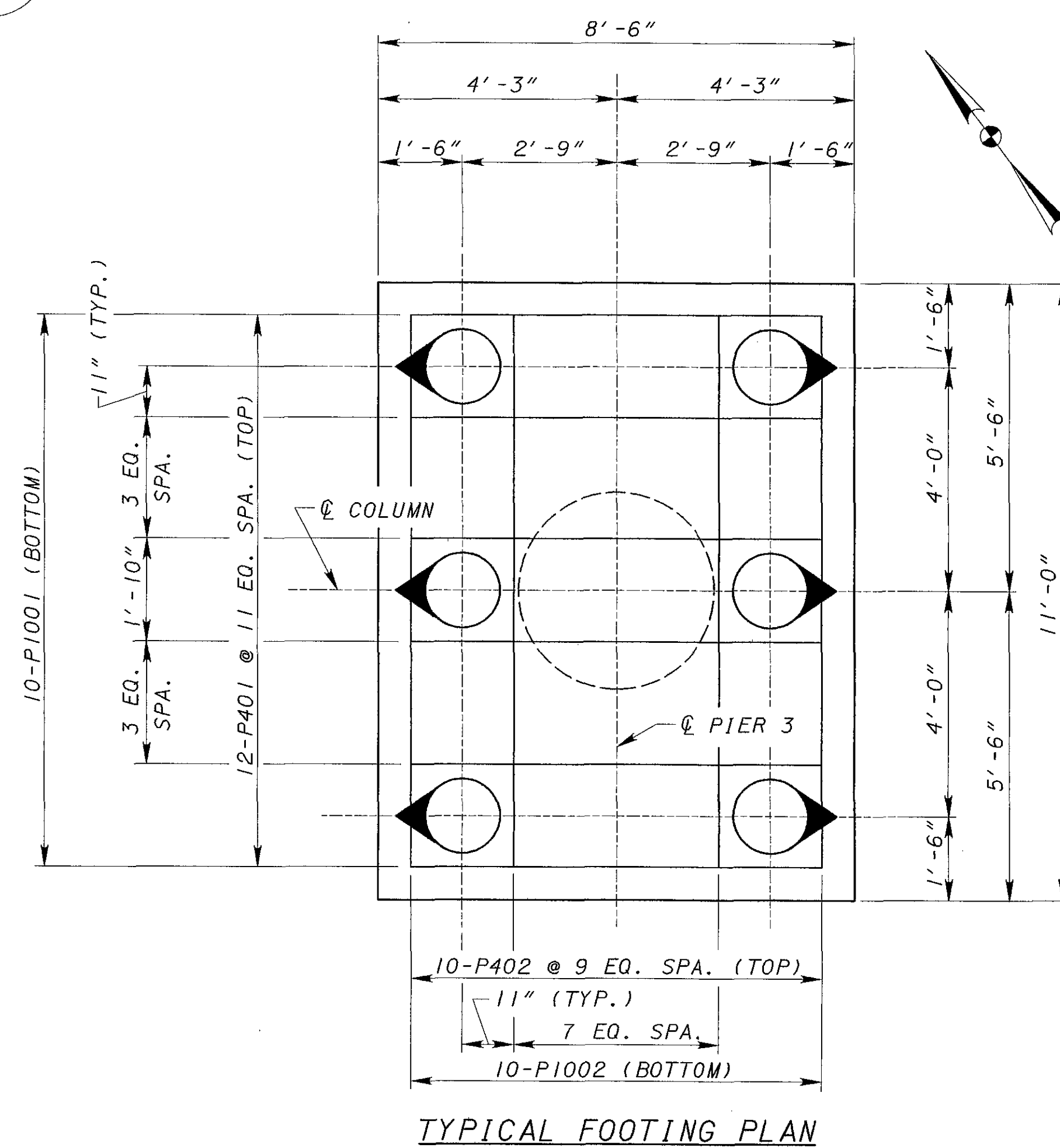
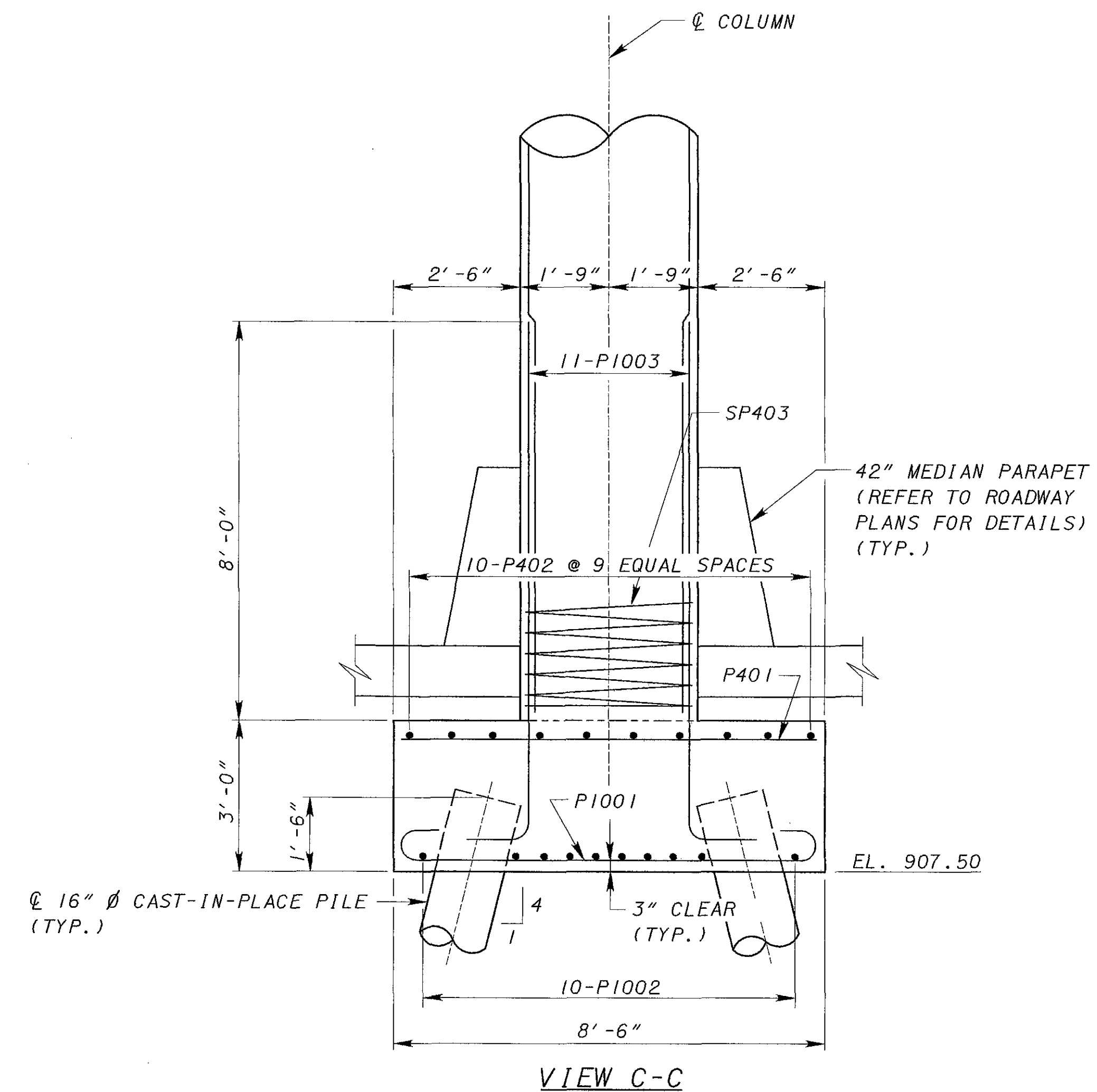
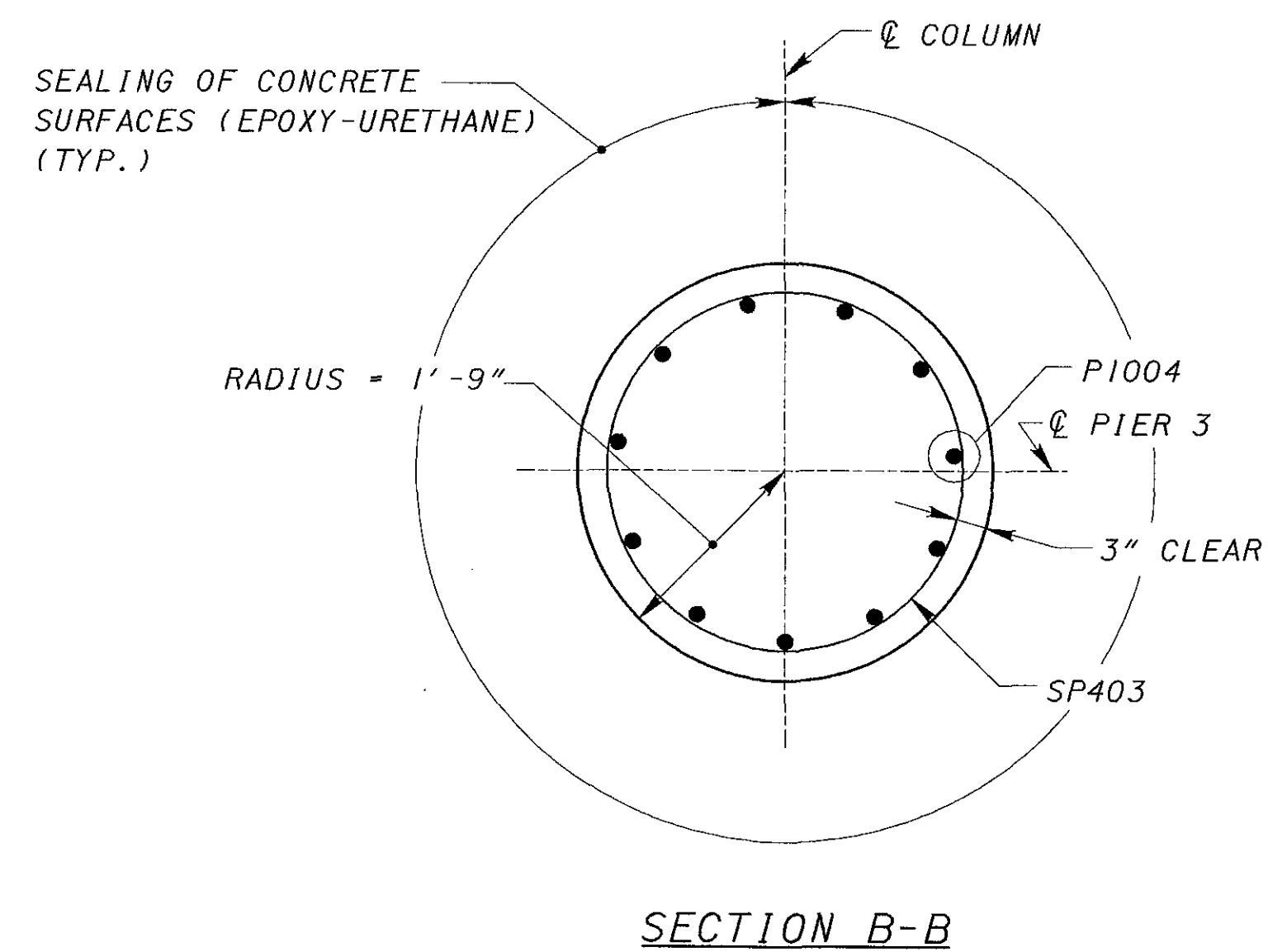
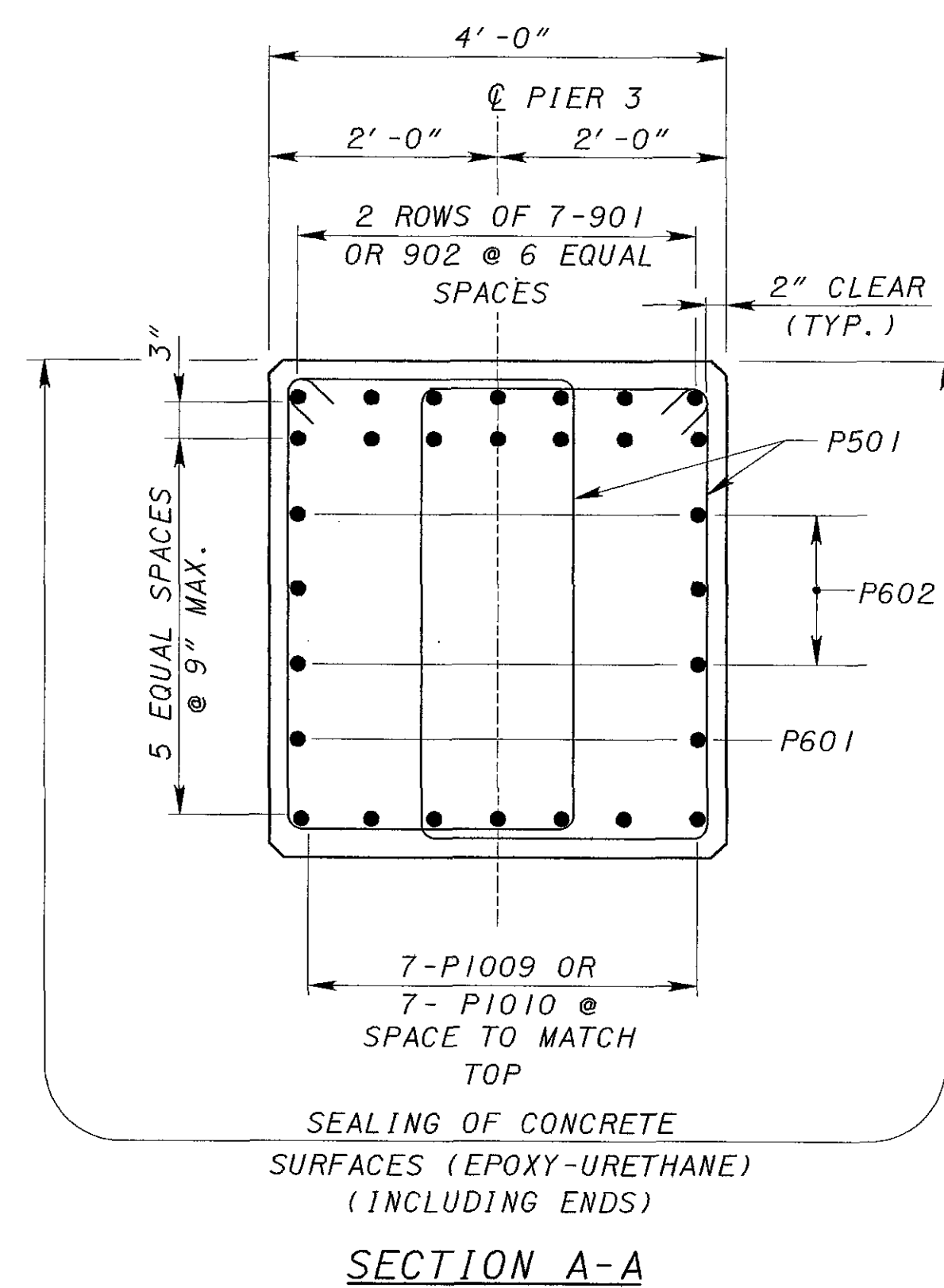
1. FOR ADDITIONAL PIER NOTES, SEE SHEET [1314].



NOTES:
1. FOR ADDITIONAL PIER NOTES, SEE SHEET [13/4].

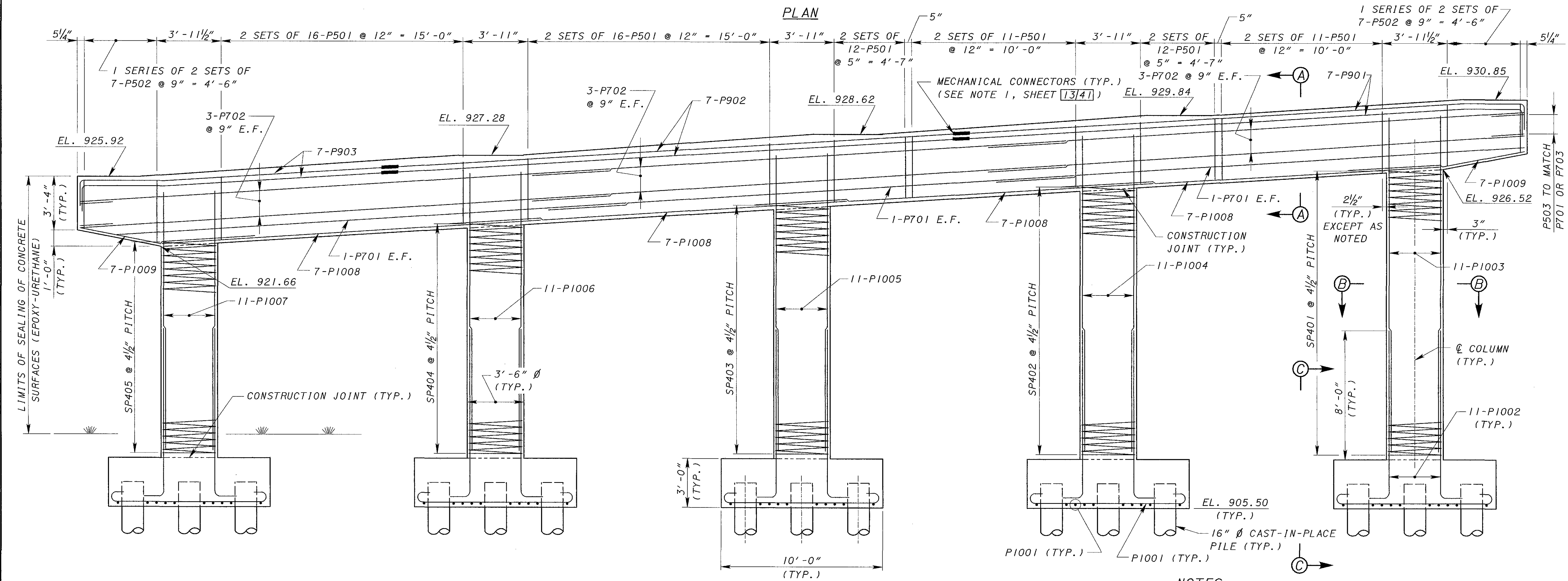
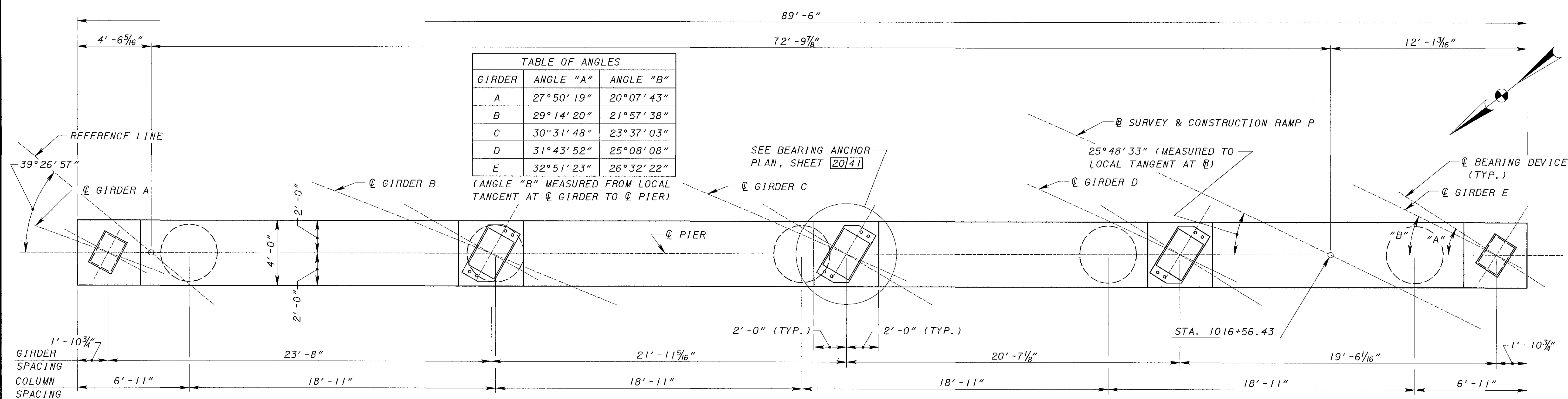


- NOTES:**
- FOR ADDITIONAL PIER NOTES, SEE SHEET 13/41.
 - FOR SECTIONS A-A AND B-B, VIEW C-C AND TYPICAL FOOTING PLAN SEE SHEET 18/41.



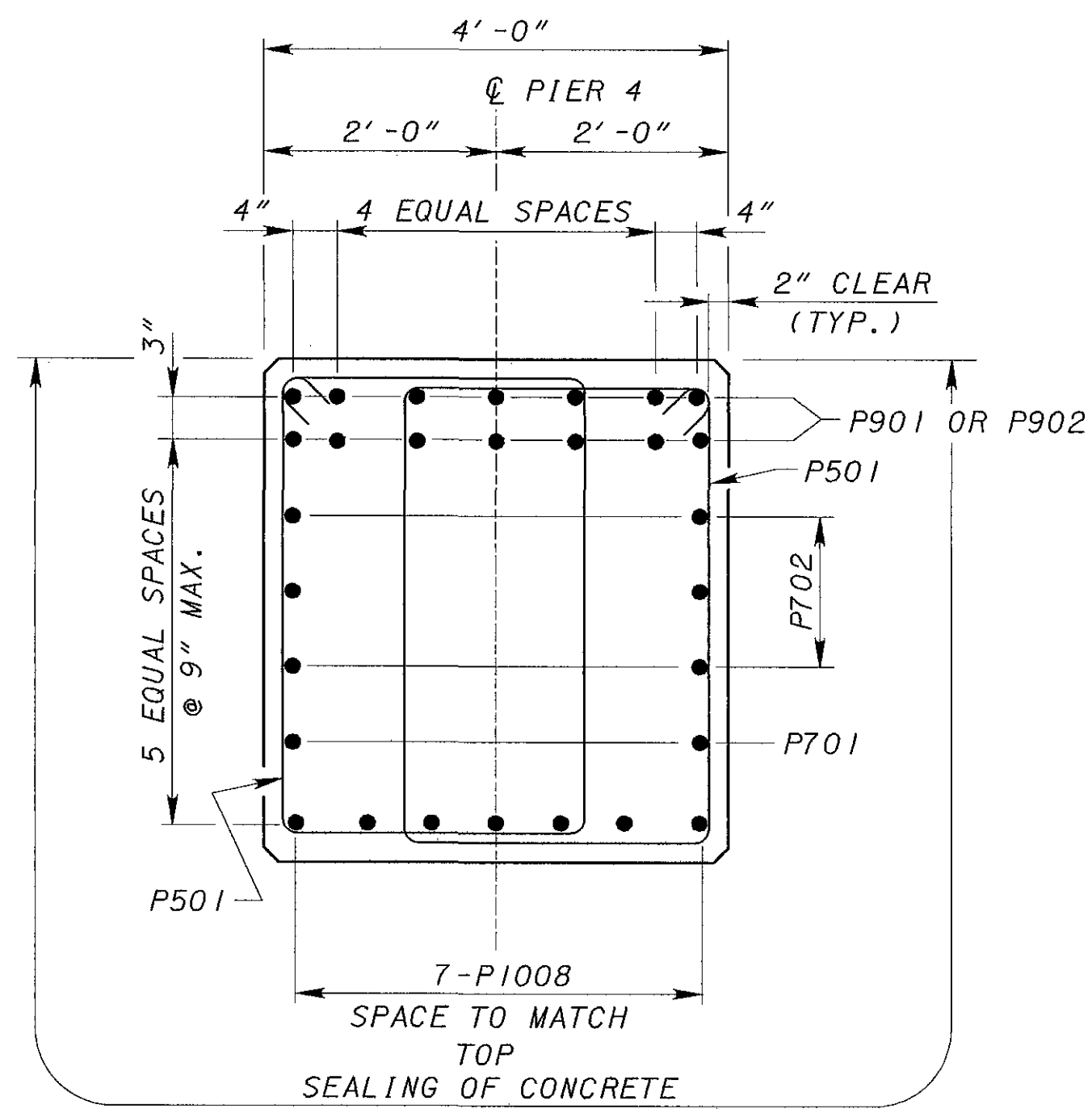
NOTES:

1. FOR ADDITIONAL PIER NOTES, SEE SHEET 13/41.

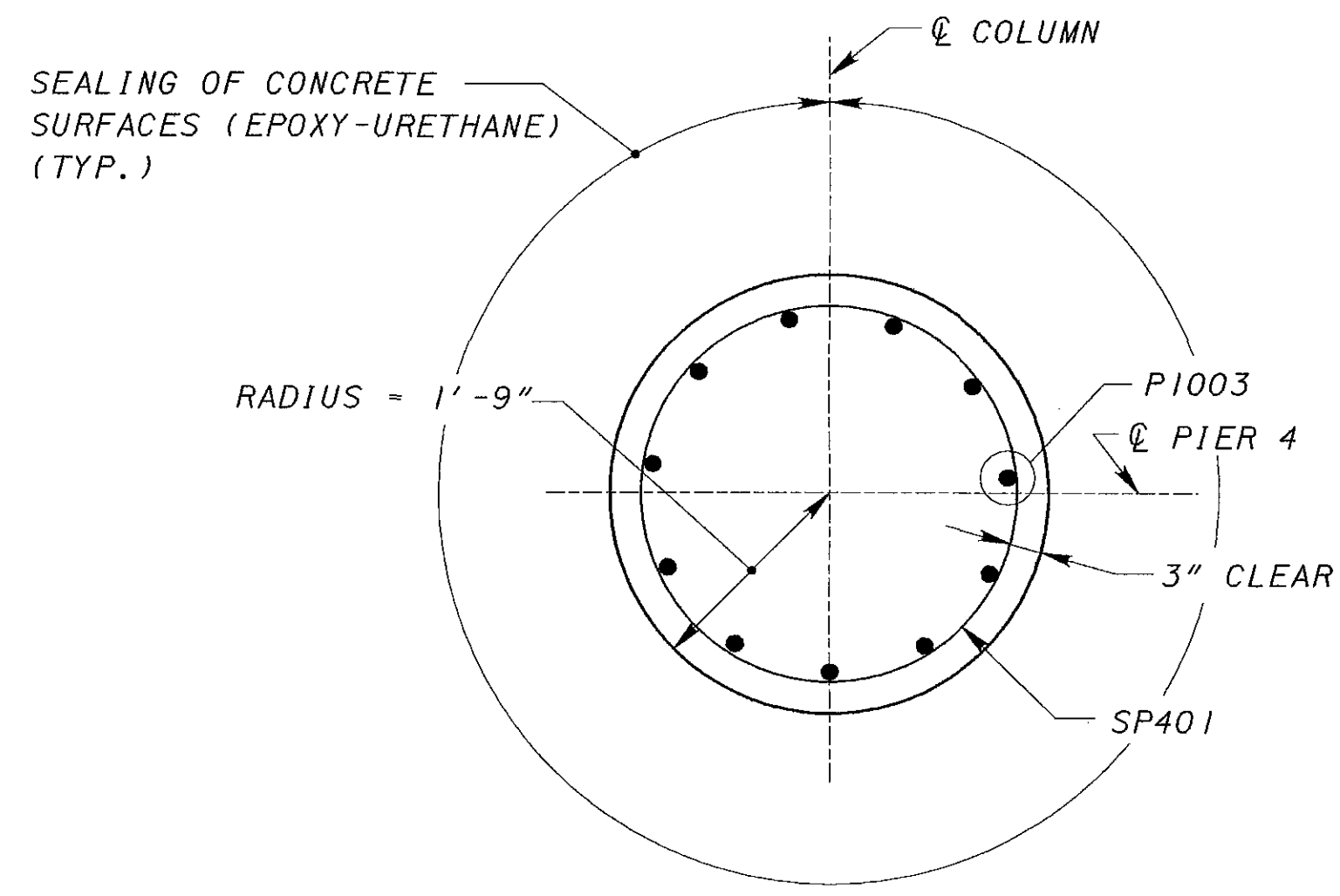


NOTES:

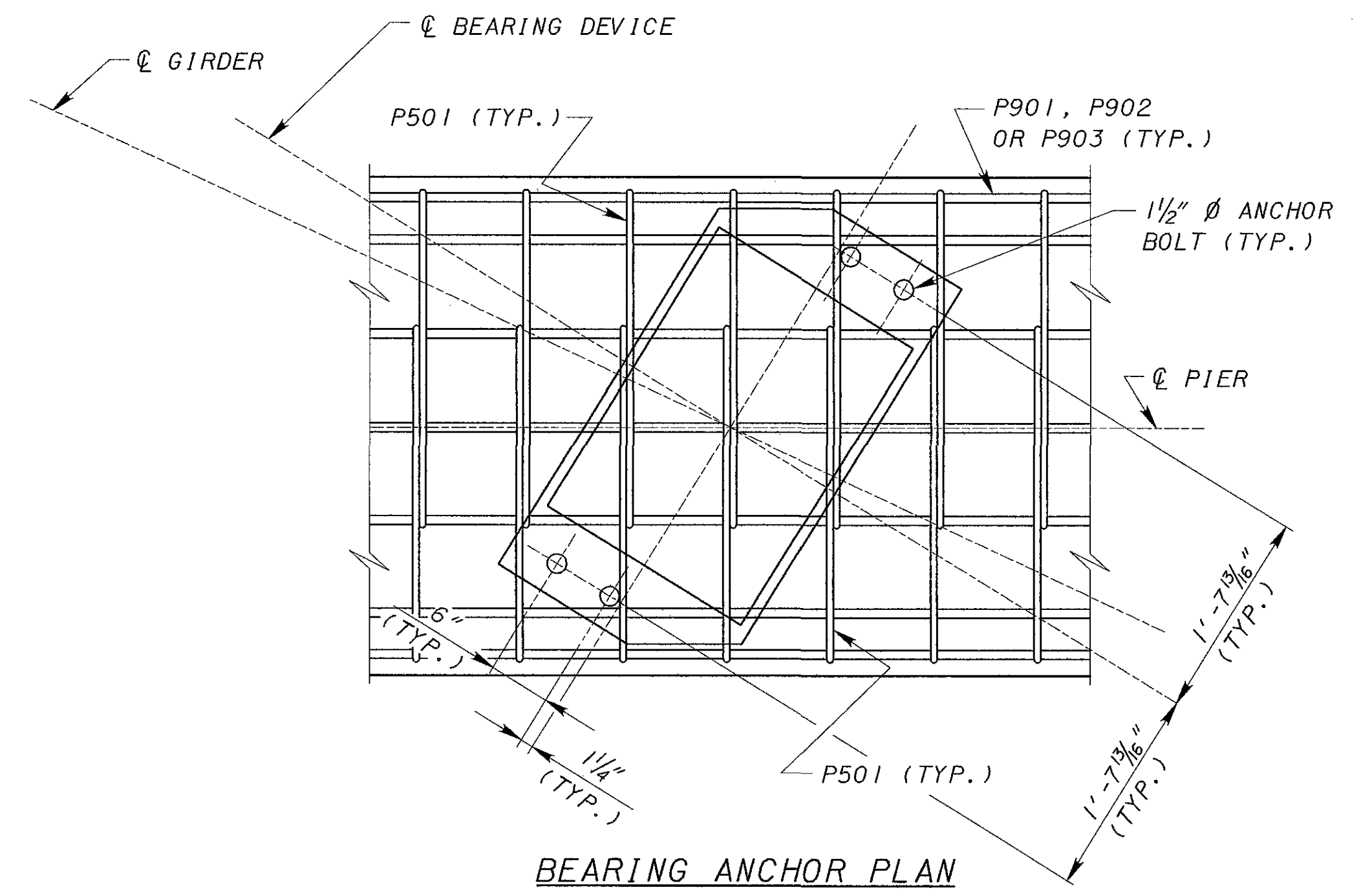
- FOR ADDITIONAL PIER NOTES, SEE SHEET [13]41.
- FOR SECTIONS A-A AND B-B, VIEW C-C AND TYPICAL FOOTING PLAN SEE SHEET [20]41.



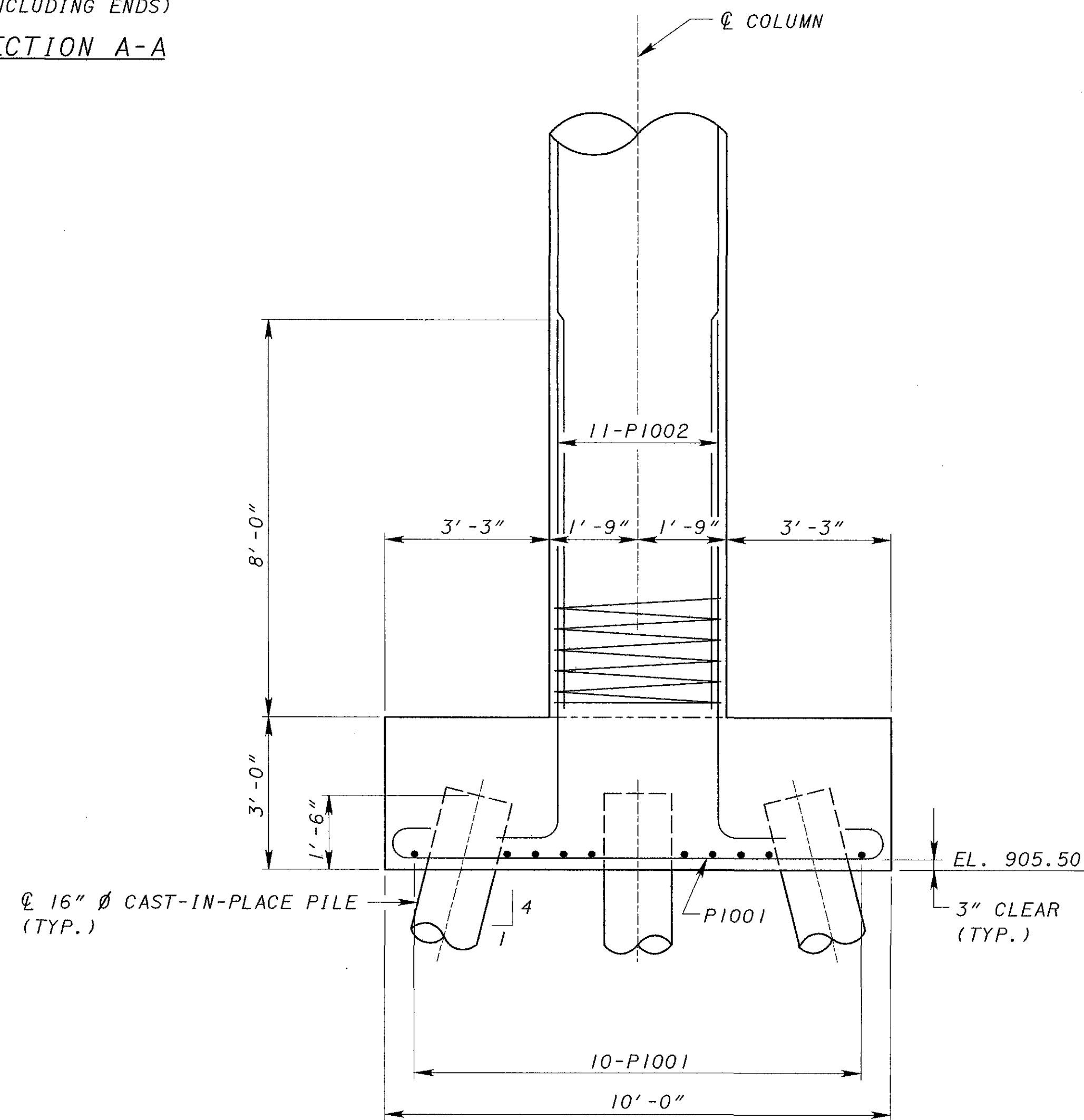
SECTION A-A



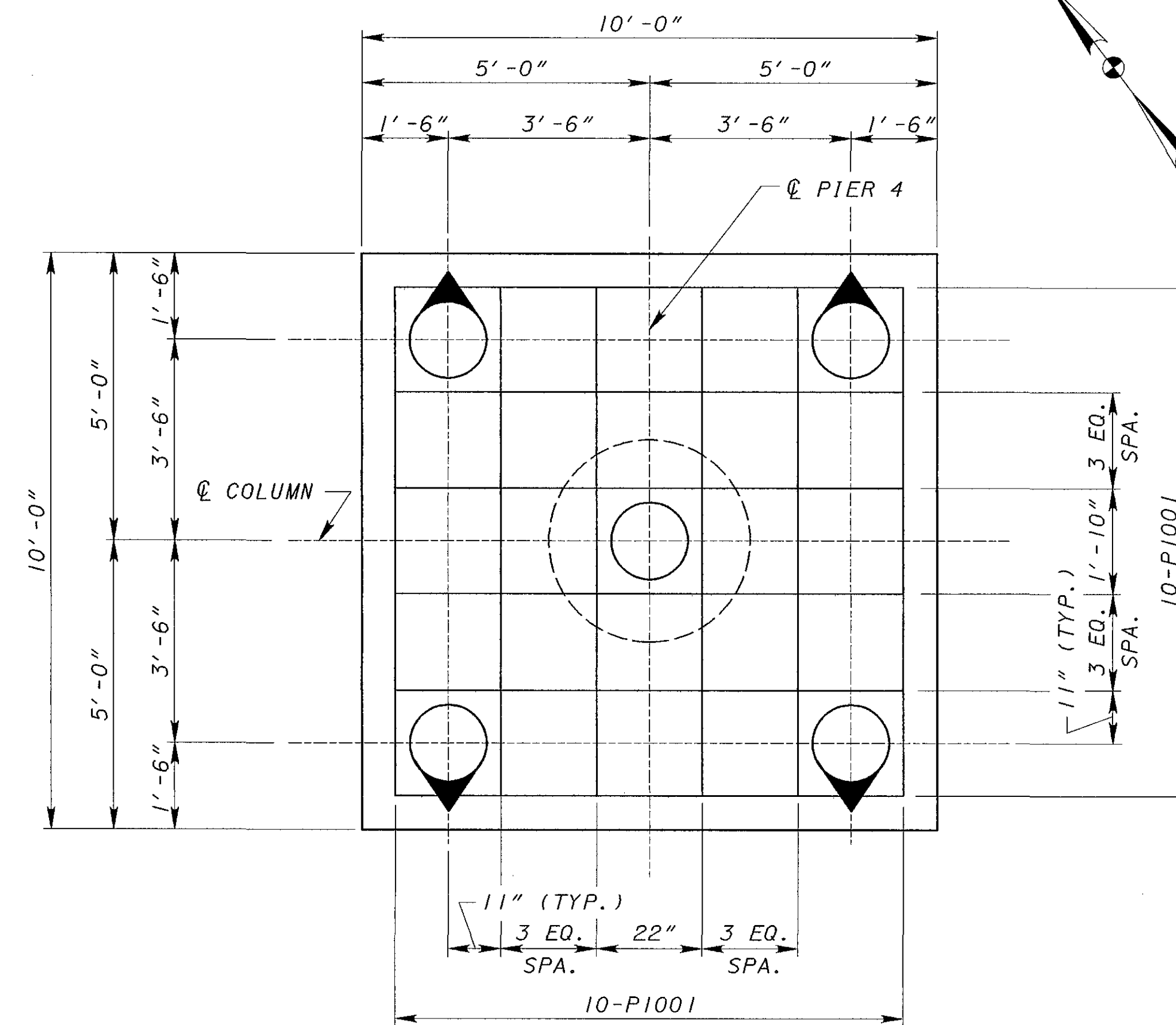
SECTION B-B



BEARING ANCHOR PLAN
(SEE NOTE 2, SHEET [13]41)



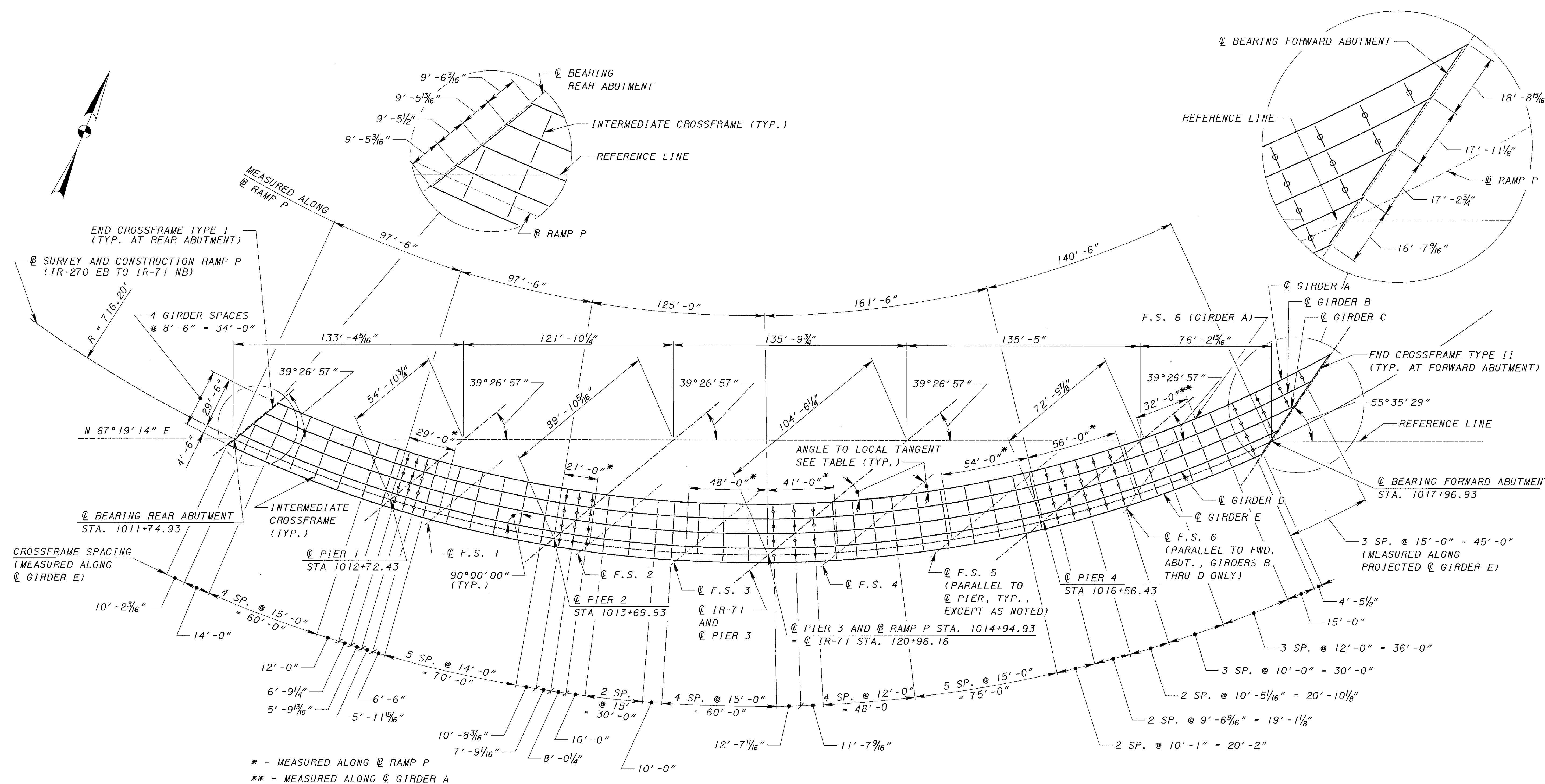
VIEW C-C



TYPICAL FOOTING PLAN

NOTES:

1. FOR ADDITIONAL PIER NOTES, SEE SHEET [13]41.



FRAMING PLAN

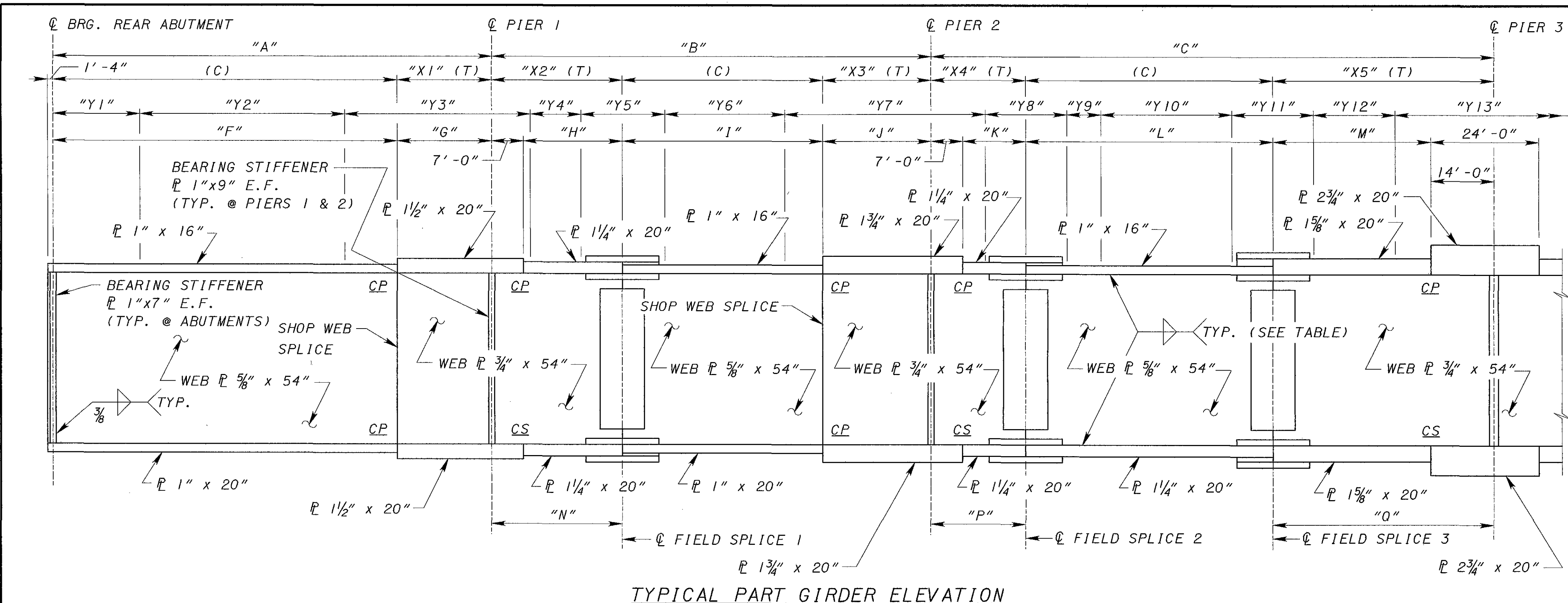
LEGEND:
 | - INDICATES INTERMEDIATE CROSSFRAME TYPE A
 † - INDICATES INTERMEDIATE CROSSFRAME TYPE B

NOTES:
 1. FOR GIRDER DETAILS AND ADDITIONAL NOTES, SEE SHEET [22/41].
 2. FOR CROSSFRAME AND END CROSSFRAME DETAILS, SEE SHEET [23/41].
 3. FOR FIELD SPLICE DETAILS, SEE SHEET [24/41].
 4. ABBREVIATIONS:
 ABUT. - ABUTMENT FWD. - FORWARD
 F.S. - FIELD SPLICE SP. - SPACES

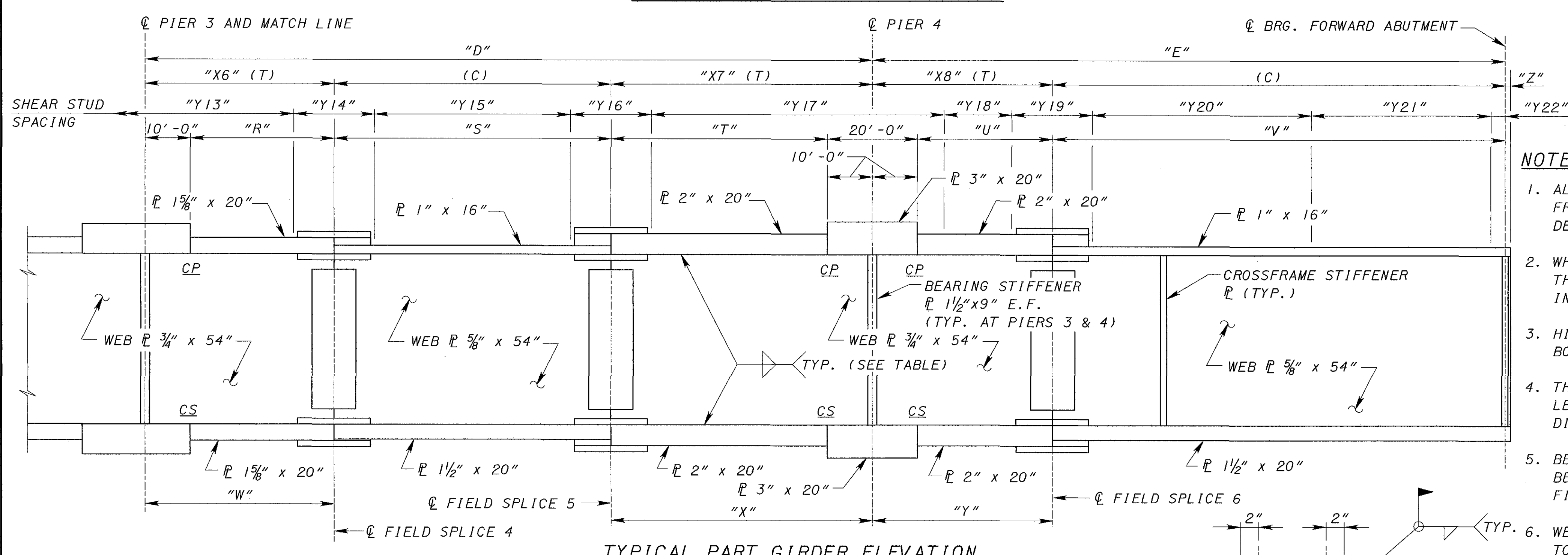
TABLE OF LOCAL TANGENT ANGLES

	℄ BEARING REAR ABUT.	℄ PIER 1	℄ F.S. 1	℄ PIER 2	℄ F.S. 2	℄ F.S. 3	℄ PIER 3	℄ F.S. 4	℄ F.S. 5	℄ PIER 4	℄ F.S. 6	℄ BEARING FWD. ABUT.
GIRDER A	63°08'25"	54°53'10"	52°24'51"	46°31'54"	44°42'47"	39°49'00"	35°32'55"	31°49'45"	25°34'16"	20°07'43"	33°36'03"	26°16'28"
GIRDER B	63°29'40"	55°22'38"	52°57'05"	47°11'31"	45°24'58"	40°38'59"	36°31'03"	32°56'25"	26°59'53"	21°57'38"	35°09'06"	27°39'35"
GIRDER C	63°50'21"	55°51'13"	53°28'19"	47°49'47"	46°05'40"	41°26'57"	37°26'32"	33°59'37"	28°19'35"	23°37'03"	36°07'22"	28°57'09"
GIRDER D	64°10'28"	56°18'59"	53°58'37"	48°26'46"	46°44'56"	42°13'04"	38°19'35"	34°59'41"	29°34'12"	25°08'08"	37°02'59"	30°09'55"
℄ RAMP P	64°19'47"	56°31'45"	54°12'33"	48°43'45"	47°02'57"	42°34'22"	38°43'46"	35°27'01"	30°07'45"	25°48'33"	37°28'17"	30°42'50"
GIRDER E	64°30'04"	56°45'56"	54°28'00"	49°02'33"	47°22'53"	42°57'26"	39°10'24"	35°56'55"	30°44'24"	26°32'22"	37°56'09"	31°18'31"

* - MEASURED ALONG ℄ RAMP P
 ** - MEASURED ALONG ℄ GIRDER A



DIMENSION	GIRDER A	GIRDER B	GIRDER C	GIRDER D	GIRDER E
"A"	98'-11 1/16"	98'-5 1/16"	98'-0 7/16"	97'-8 1/16"	97'-3 5/8"
"B"	100'-1 1/16"	99'-3 3/4"	98'-6 9/16"	97'-9 5/16"	97'-1 3/4"
"C"	131'-7 9/16"	129'-6 1/4"	127'-6 5/16"	125'-9 1/2"	124'-1 1/16"
"D"	184'-9 3/4"	176'-7 9/16"	169'-9 1/2"	163'-11 9/16"	158'-11"
"E"	119'-9 1/16"	126'-8 1/2"	132'-8 3/16"	138'-1 9/16"	143'-0 1/16"
"F"	77'-8 1/16"	77'-3 3/4"	76'-11 1/16"	76'-7 3/4"	76'-4 1/16"
"G"	21'-3 1/16"	21'-2 1/8"	21'-1 1/4"	21'-0 3/8"	20'-11 9/16"
"H"	22'-5 3/16"	22'-4 1/16"	22'-2 5/16"	22'-0 3/4"	21'-11 3/16"
"I"	46'-1 1/8"	45'-7 3/16"	45'-1 5/8"	44'-8 3/8"	44'-3 1/16"
"J"	24'-6 5/8"	24'-4 9/16"	24'-2 5/8"	24'-0 1/8"	23'-1 1/8"
"K"	14'-6 1/2"	14'-4 7/16"	14'-2 9/16"	14'-0 3/8"	13'-1 1/8"
"L"	59'-1 17/16"	58'-8 3/8"	57'-6 1/2"	56'-5 1/16"	55'-5 5/16"
"M"	36'-1 1/8"	35'-5 3/8"	34'-9 7/16"	34'-3 1/16"	33'-8 3/4"
"N"	29'-5 5/16"	29'-4 1/16"	29'-2 5/16"	29'-0 3/4"	28'-11 3/16"
"P"	21'-6 1/2"	21'-4 7/16"	21'-2 9/16"	21'-0 3/8"	20'-11 1/8"
"Q"	50'-1 5/8"	49'-5 3/8"	48'-9 7/16"	48'-3 1/16"	47'-8 3/4"
"R"	33'-9 1/2"	32'-10 7/16"	32'-0 5/8"	31'-3 3/16"	30'-7 7/8"
"S"	78'-3 3/4"	74'-2 7/16"	70'-8 7/8"	67'-9 3/16"	65'-2 1/16"
"T"	52'-8 7/16"	49'-6 5/8"	47'-0"	44'-10 9/16"	43'-1 1/16"
"U"	22'-0"	24'-1"	33'-9 3/16"	42'-3 3/16"	49'-11 9/16"
"V"	87'-9 1/16"	92'-7 1/2"	88'-11 5/8"	85'-10"	83'-1 1/8"
"W"	43'-9 1/2"	42'-10 7/16"	42'-0 5/8"	41'-3 1/16"	40'-7 7/8"
"X"	62'-8 7/16"	59'-6 5/8"	57'-0"	54'-10 9/16"	53'-1 1/16"
"Y"	32'-0"	34'-1"	43'-9 3/16"	52'-3 9/16"	59'-11 9/16"
"Z"	2'-8"	2'-6 1/2"	2'-5 1/4"	2'-4"	2'-3"



DIMENSION	GIRDER A	GIRDER B	GIRDER C	GIRDER D	GIRDER E
"X1"	22'-10"	22'-4"	22'-2"	21'-7"	21'-2"
"X2"	28'-3"	29'-5"	30'-10"	31'-7"	32'-8"
"X3"	27'-9"	26'-8"	25'-11"	25'-9"	25'-8"
"X4"	21'-5"	22'-5"	22'-5"	21'-11"	22'-8"
"X5"	47'-10"	45'-2"	43'-7"	40'-7"	41'-0"
"X6"	36'-1"	37'-3"	38'-0"	38'-6"	41'-11"
"X7"	55'-5"	45'-7"	37'-6"	32'-4"	28'-9"
"X8"	32'-8"	39'-7"	45'-5"	50'-11"	59'-4"

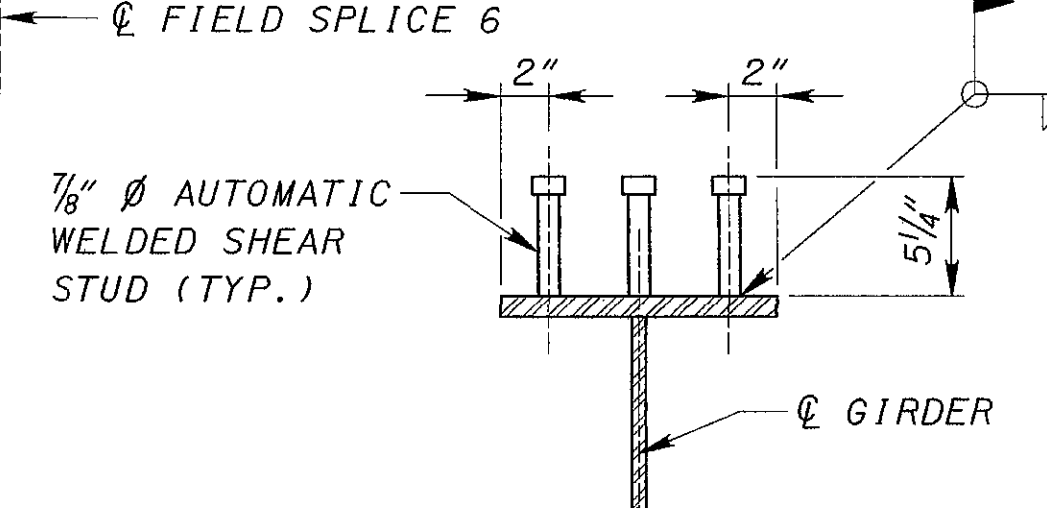
LEGEND:

(T) - DENOTES AREA OF TENSION IN THE TOP FLANGE. THE BOTTOM FLANGE IN THESE AREAS IS IN COMPRESSION.

(C) - DENOTES AREA OF COMPRESSION IN THE TOP FLANGE. THE BOTTOM FLANGE IN THESE AREAS IS IN TENSION.

"CS" - INDICATES BUTT WELD SUBJECT TO COMPRESSIVE STRESSES ONLY.

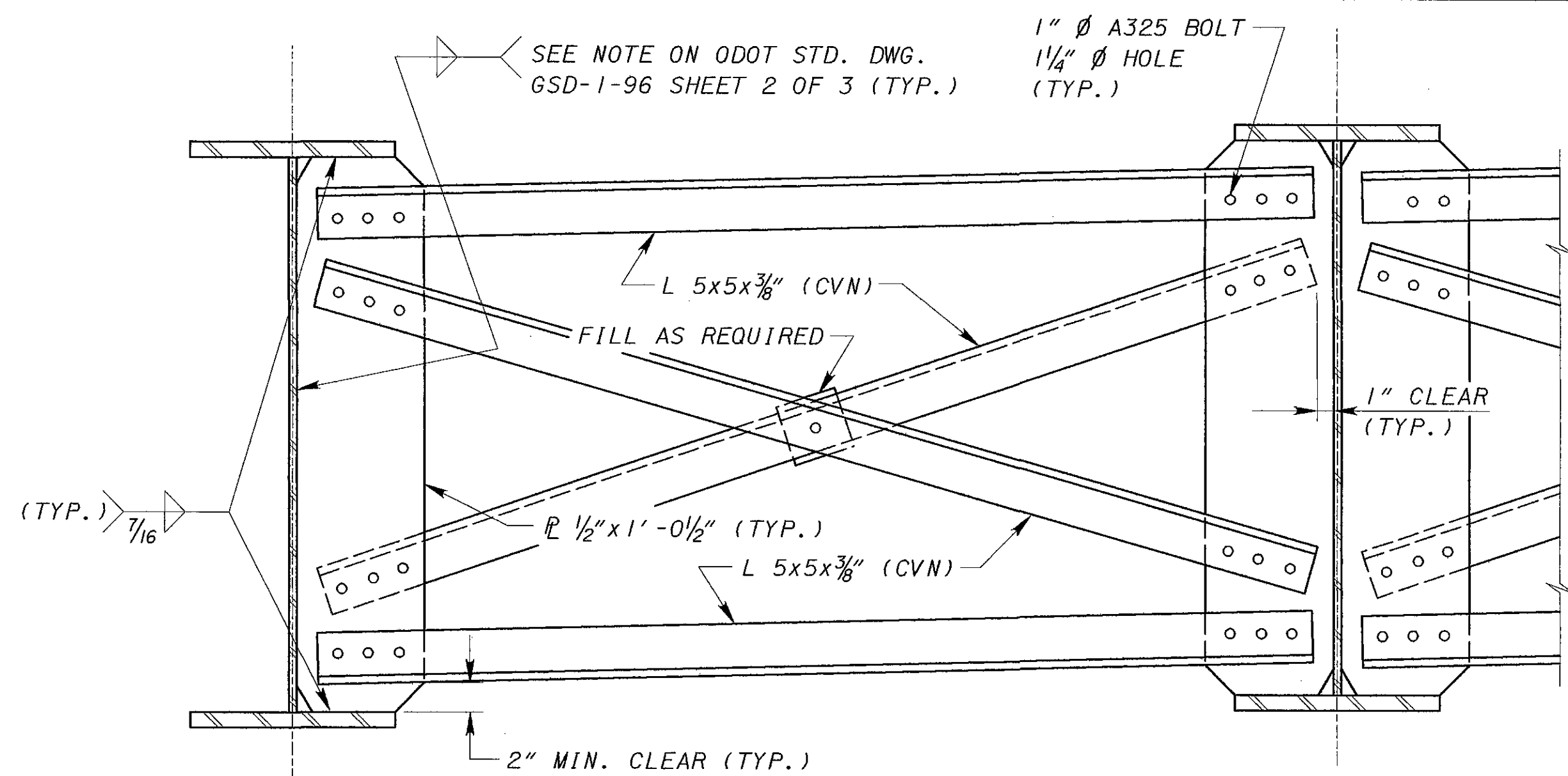
"CP" - INDICATES COMPLETE JOINT PENETRATION WELD.



NOTE: STUD PLACEMENT ON FLANGE SPLICE PLATES IS NOT PERMITTED. ADJUST SPACINGS TO AVOID INTERFERENCE WITH SPLICE PLATES AND CONNECTION BOLTS

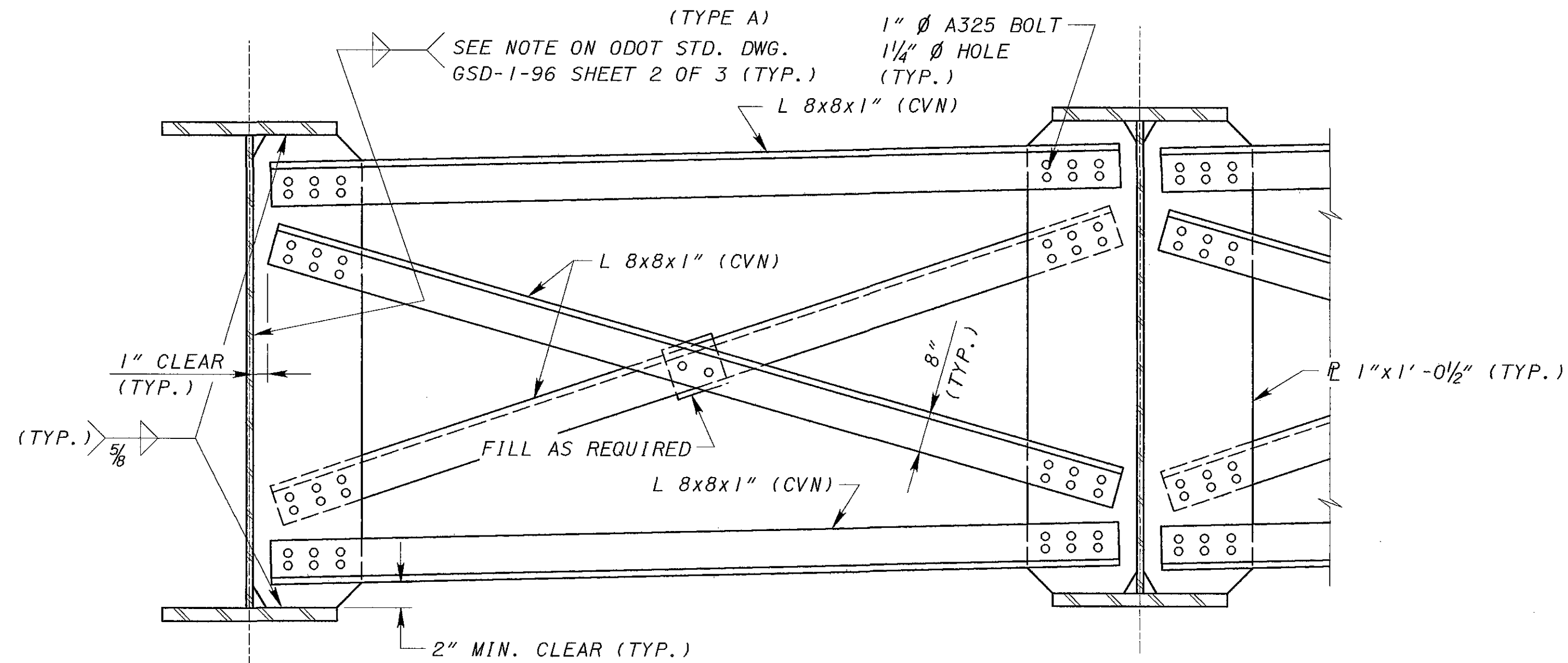
FLANGE PLATE THICKNESS	FILLET WELD SIZE
7/8" THRU 1 1/2"	5/16"
1 5/8" THRU 2 1/2"	3/8"

- NOTES:**
- ALL FLANGE AND WEB PLATES, INCLUDING FIELD SPLICE PLATES, CROSS FRAME MEMBERS, AND CROSS FRAME CONNECTION STIFFENERS, SHALL BE DESIGNATED "CVN".
 - WHERE A SHAPE OR PLATE IS DESIGNATED (CVN), FURNISH MATERIAL THAT MEETS THE MINIMUM NOTCH TOUGHNESS REQUIREMENTS AS SPECIFIED IN 711.01.
 - HIGH STRENGTH BOLTS SHALL BE 1" DIAMETER A325 GALVANIZED TYPE I BOLTS UNLESS OTHERWISE NOTED.
 - THE WEB PLATES MAY BE SHOP SPLICED AS REQUIRED BY AVAILABLE PLATE LENGTH. THE LOCATION OF WEB SHOP SPLICES SHALL BE SUBMITTED TO THE DIRECTOR FOR APPROVAL PRIOR TO ORDERING OF MATERIAL.
 - BEARING STIFFENERS SHALL BE PLACED IN PAIRS ON ALL GIRDERS AND SHALL BE SET VERTICAL WITH FULL BEARING ON THE BOTTOM FLANGE AND A TIGHT FIT AT THE TOP FLANGE.
 - WELD ATTACHMENT OF SUPPORTS FOR CONCRETE DECK FINISHING MACHINE TO AREAS OF THE FASCIA STRINGER FLANGES DESIGNATED "COMPRESSION (C)". DO NOT WELD ATTACHMENTS TO AREAS DESIGNATED "TENSION (T)". FILLET WELDS TO COMPRESSION FLANGES SHALL BE AT LEAST 1" FROM EDGE OF FLANGE, BE NO MORE THAN 2" LONG, AND BE AT LEAST 1/4" FOR THICKNESS UP TO 3/4" OR 5/16" FOR GREATER THAN 3/4" THICK.
 - BUTT WELDS AT SHOP SPLICES SHALL BE COMPLETE PENETRATION WELDS (CP). WELD REINFORCEMENT SHALL BE REMOVED BY GRINDING IN THE DIRECTION OF THE MAIN STRESSES.
 - FOR INTERMEDIATE CROSSFRAMES AND END CROSSFRAME DETAILS, SEE SHEET [23/41].
 - FOR FIELD SPLICE DETAILS, SEE SHEET [24/41].
 - FOR BEARING DETAILS, SEE SHEET [28/41] & [29/41].
 - FOR CAMBER AND DEFLECTIONS, SEE SHEET [25/41] THRU [27/41].
 - FOR TABLE OF SHEAR STUD SPACINGS, SEE SHEET [23/41].



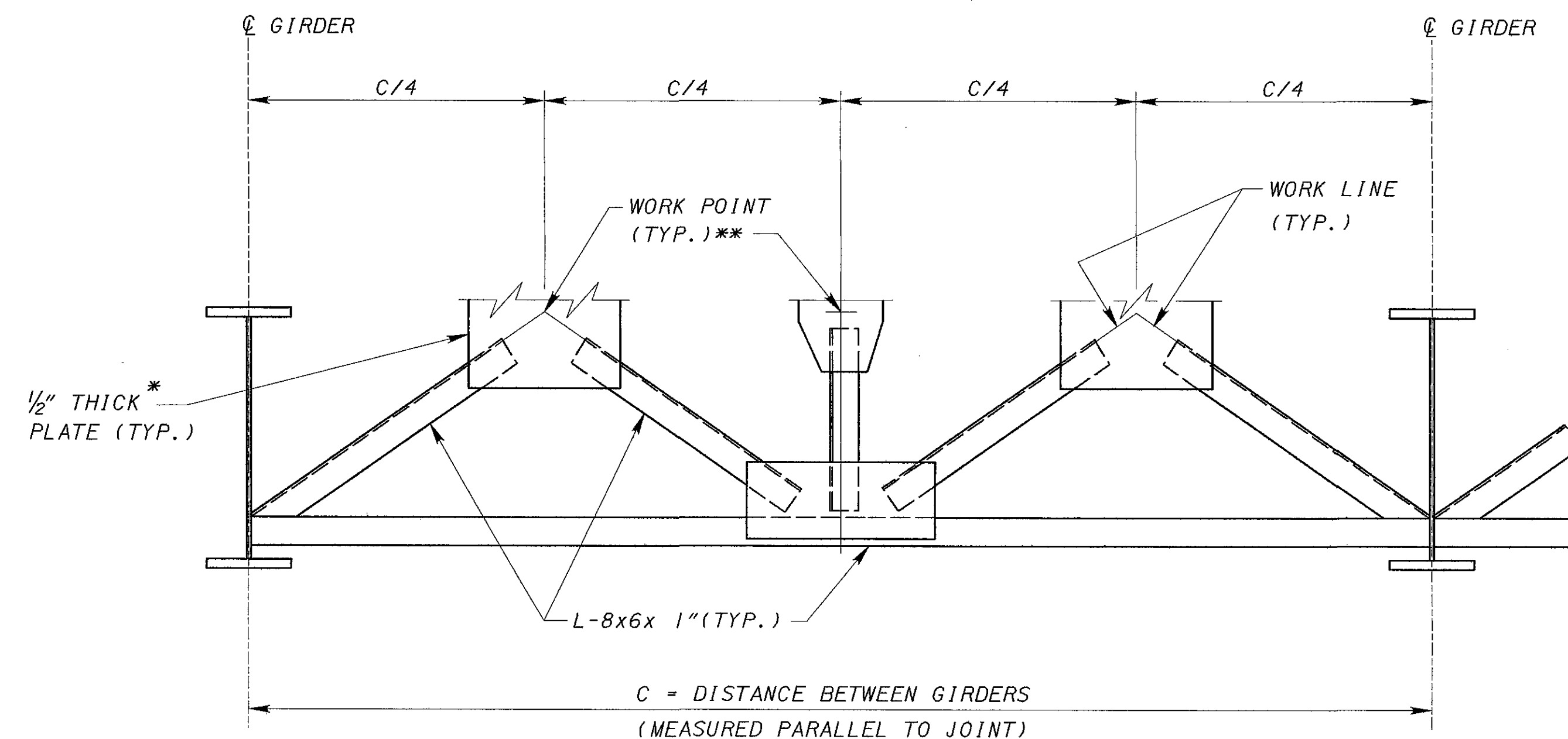
INTERMEDIATE CROSSFRAME

(TYPE A)



INTERMEDIATE CROSSFRAME

(TYPE B)

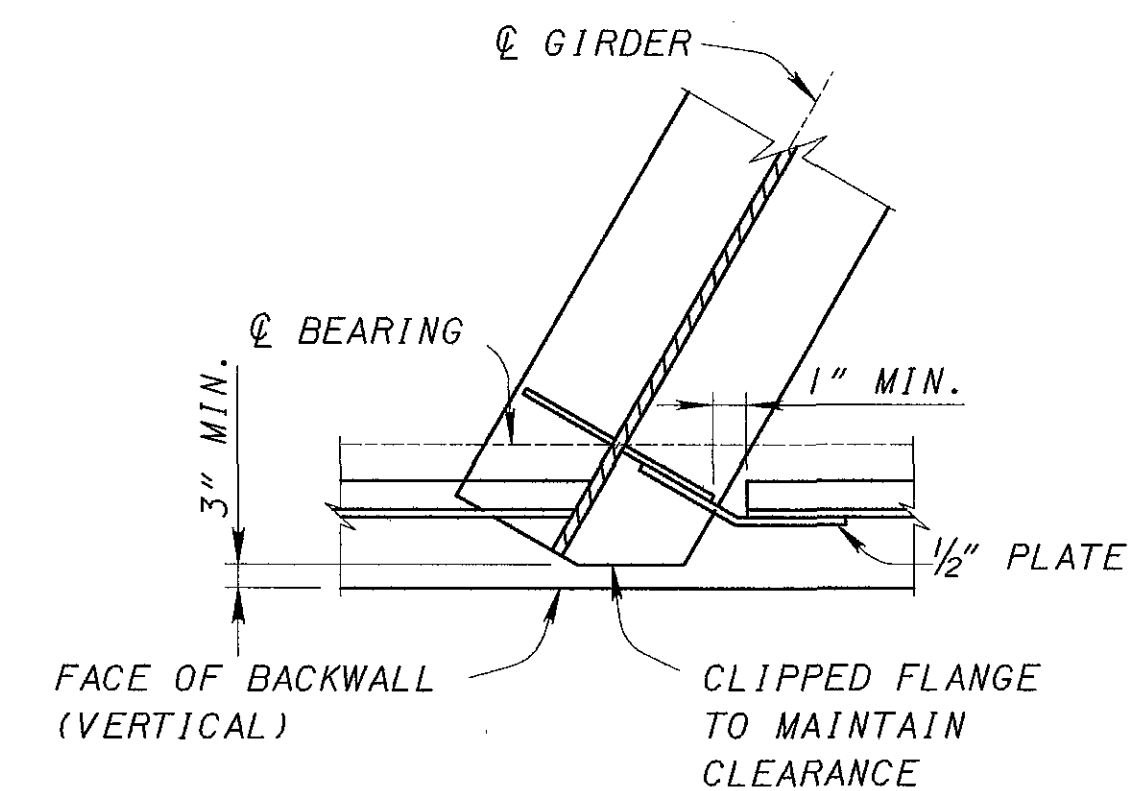


END CROSSFRAME DETAILS

(TYPE 11 SHOWN, FOR TYPE 1 REFER TO O.D.O.T. STD. DRAWING GSD-1-96, FOR 8'-0" TO 12'-0" GIRDER SPACING)

* - THE 1/2" THICK PLATE IS PART OF THE EXPANSION JOINT SYSTEM. SEE THE APPROPRIATE EXPANSION JOINT STANDARD BRIDGE DRAWING FOR DETAILS INCLUDING MATERIAL AND COATING REQUIREMENTS.

** - THE WORK LINE SHALL BE THE INSIDE FACE OF THE PROTRUDING ANGLE LEG EXTENDED AS SHOWN. THE WORK POINT SHALL BE AT THE INTERSECTION OF THESE LINES

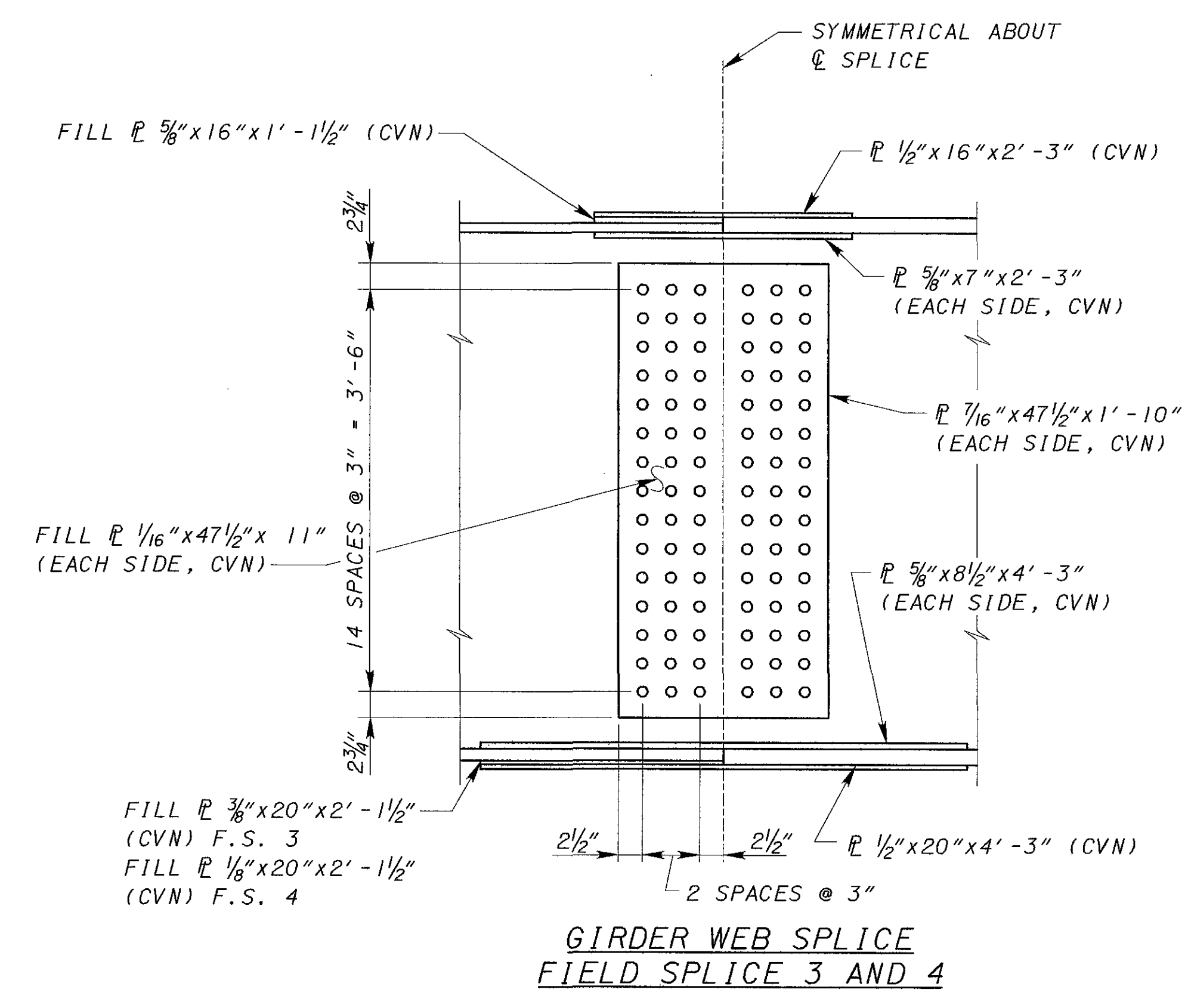
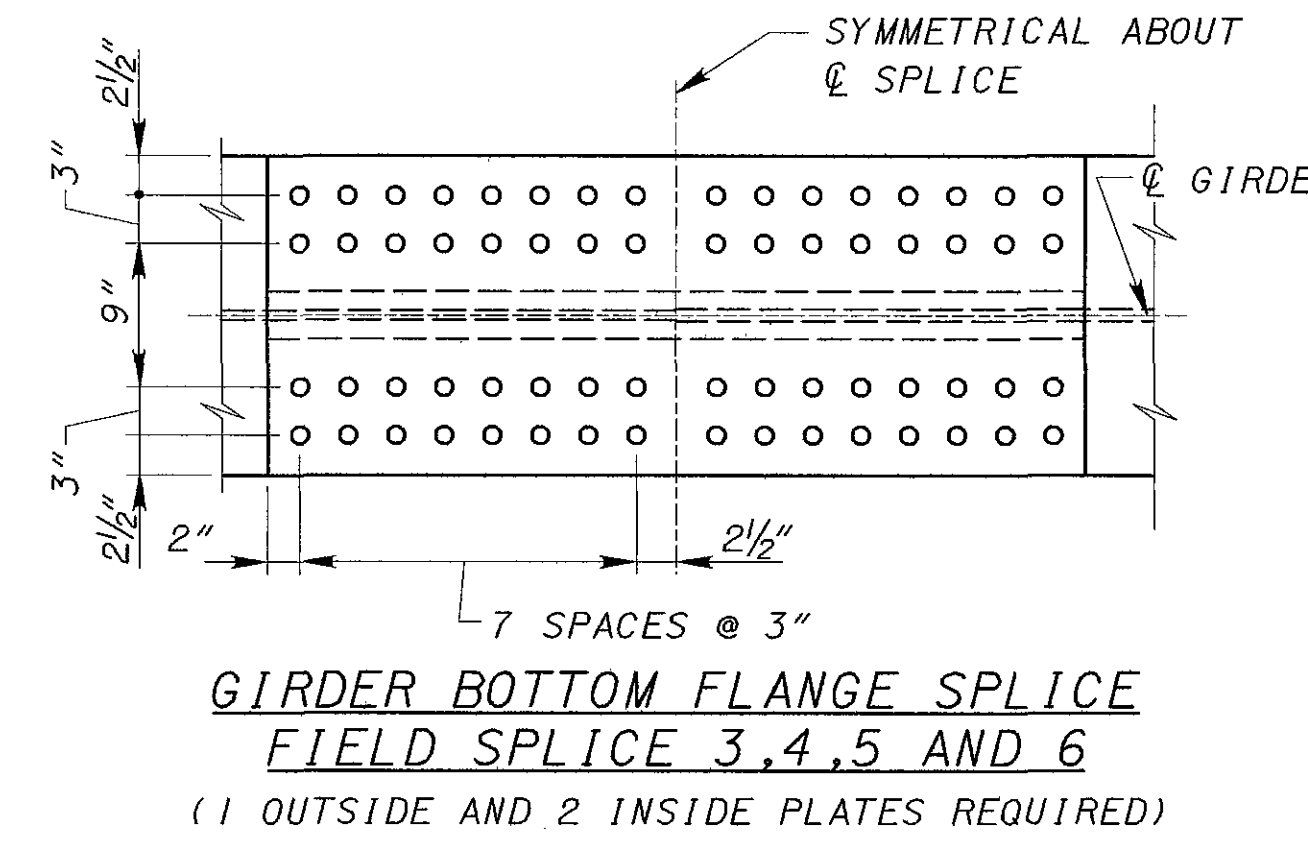
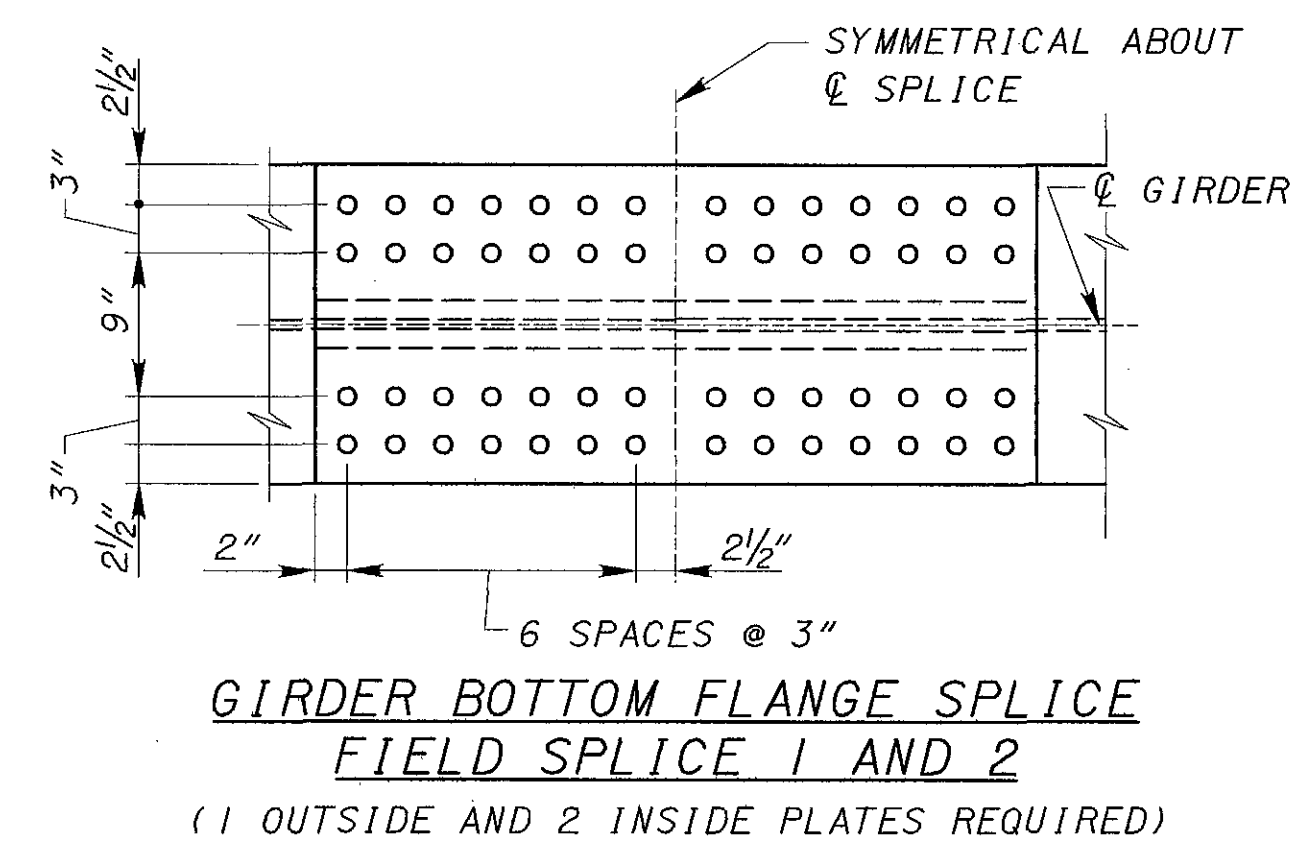
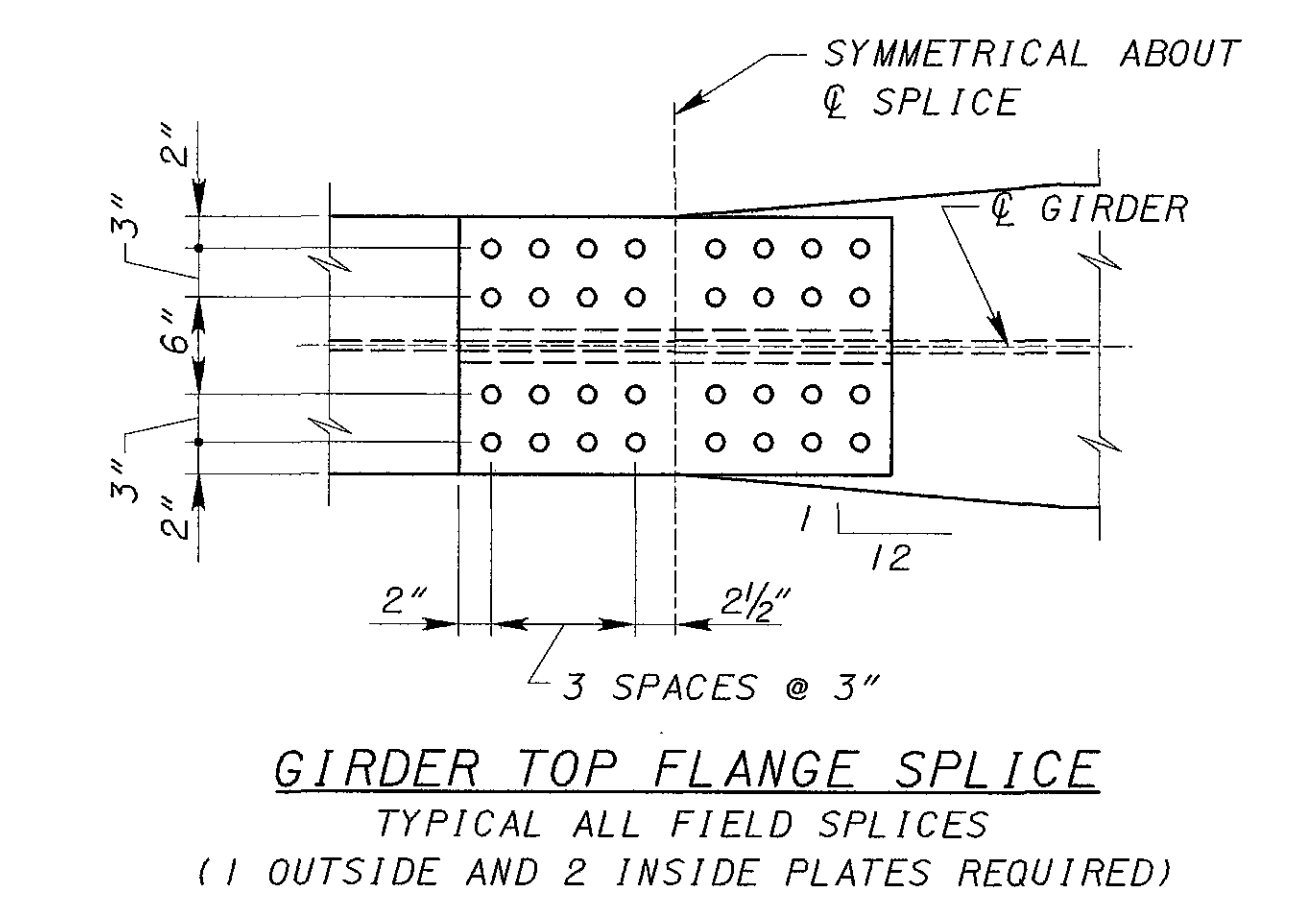
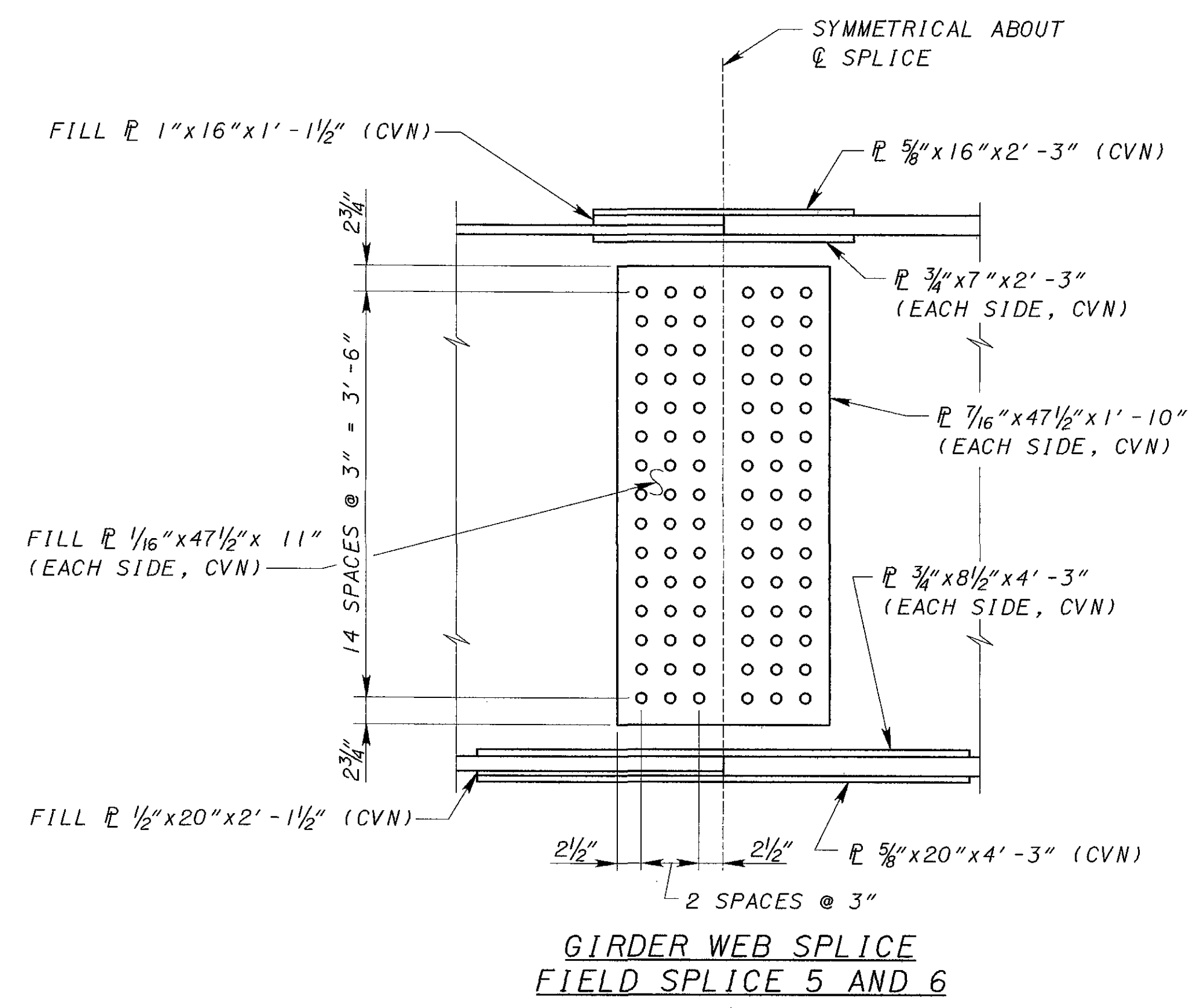
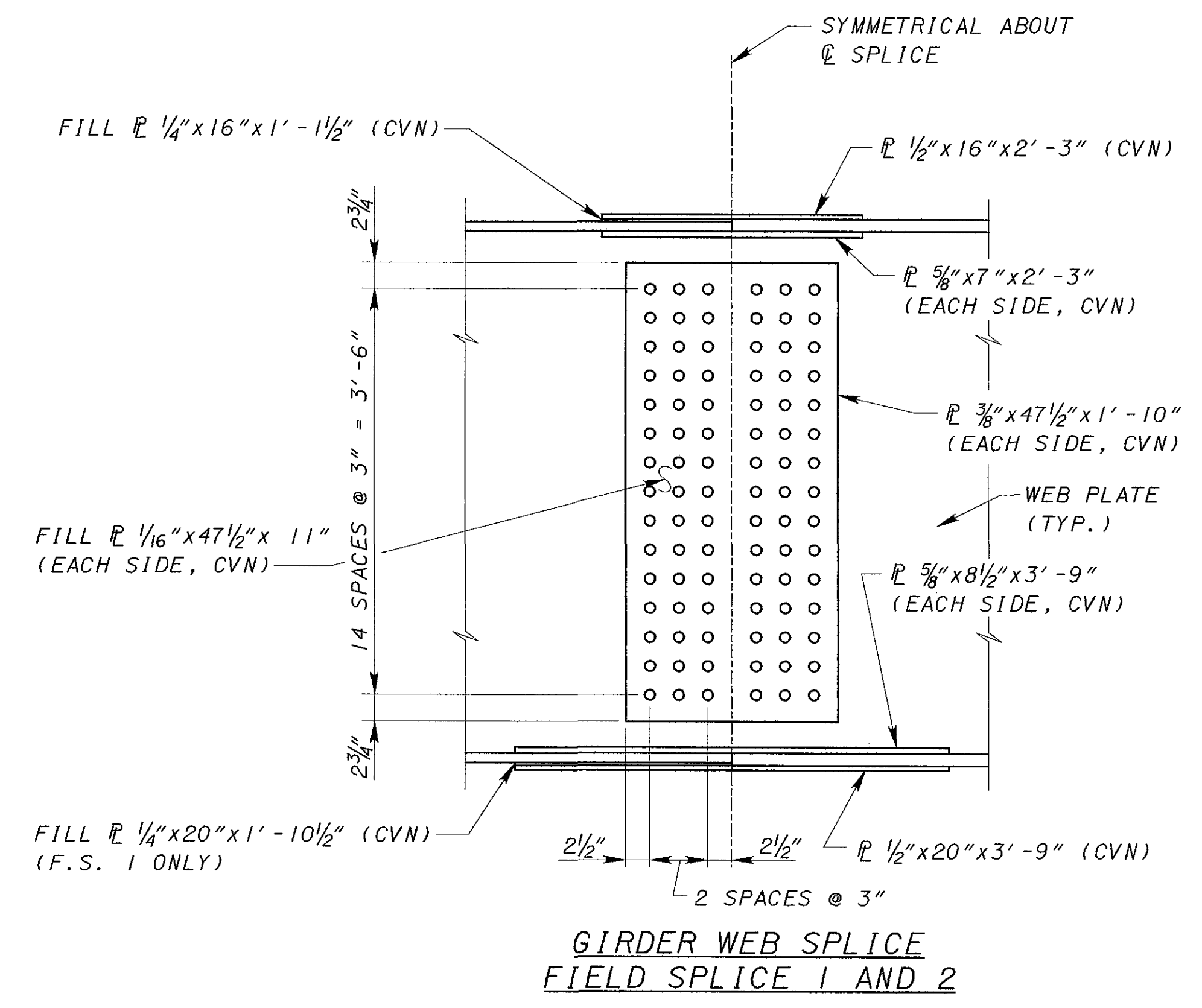


CLIPPED FLANGE DETAIL

NOTES:

- WHERE A SHAPE OR PLATE IS DESIGNATED (CVN), FURNISH MATERIAL THAT MEETS THE MINIMUM NOTCH TOUGHNESS REQUIREMENTS AS SPECIFIED IN 711.01.
- FOR GIRDER NOTES, SEE SHEET [22/4].
- FOR ADDITIONAL CROSS FRAME DETAILS NOT SHOWN, REFER TO ODOT STANDARD DRAWING GSD-1-96.
- FOR CROSS FRAME LOCATIONS, REFER TO FRAMING PLAN, SHEET [21/4].

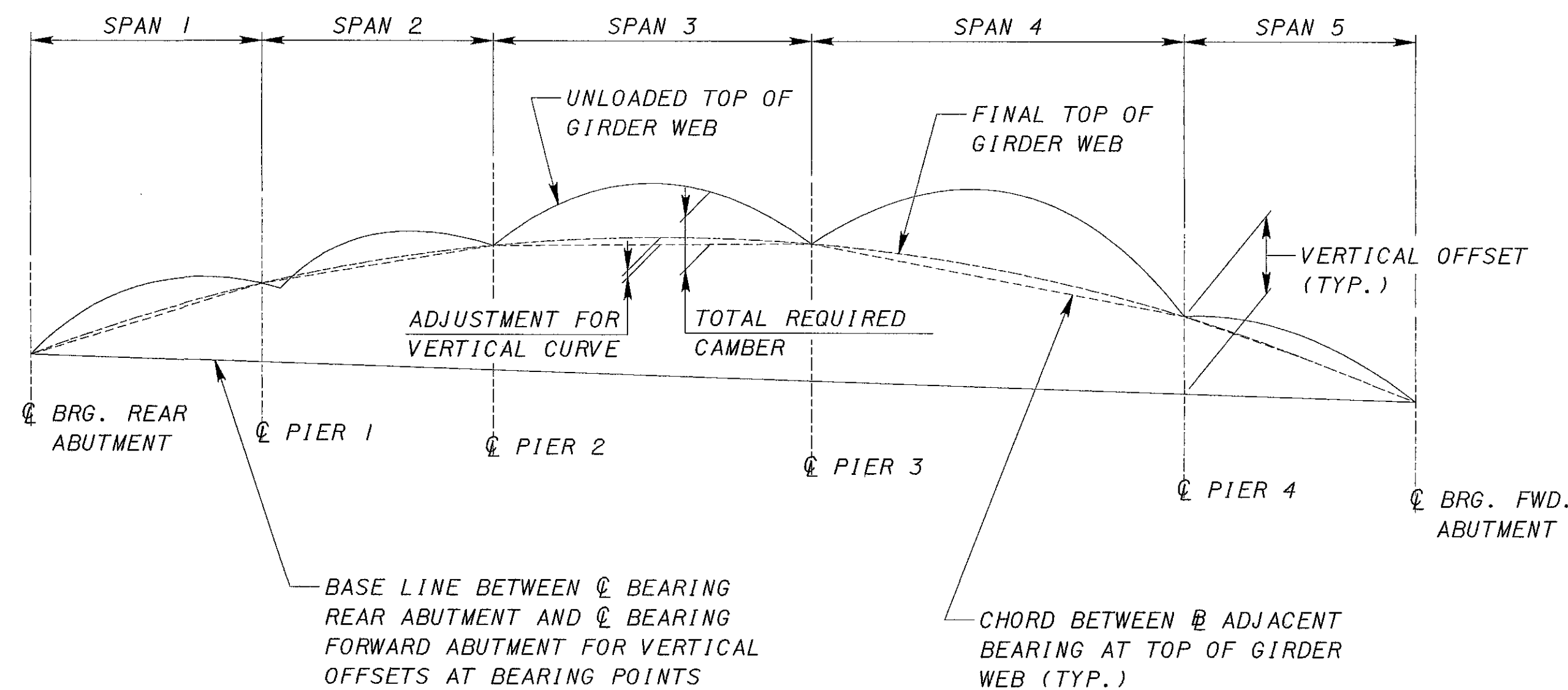
DIMENSION	GIRDER A	GIRDER B	GIRDER C	GIRDER D	GIRDER E
"Y1"	14 SPA. @ 9" = 10'-6"	14 SPA. @ 9" = 10'-6"	14 SPA. @ 9" = 10'-6"	14 SPA. @ 9" = 10'-6"	14 SPA. @ 9" = 10'-6"
"Y2"	74 SPA. @ 11" = 67'-10"	53 SPA. @ 11" = 48'-7"	52 SPA. @ 11" = 47'-8"	52 SPA. @ 11" = 47'-8"	74 SPA. @ 11" = 67'-10"
"Y3"	41 SPA. @ 9" = 30'-9"	77 SPA. @ 9" = 57'-9"	80 SPA. @ 9" = 60'-0"	79 SPA. @ 9" = 59'-3"	38 SPA. @ 9" = 28'-6"
"Y4"	21 SPA. @ 10" = 17'-6"	11 SPA. @ 10" = 9'-2"	9 SPA. @ 10" = 7'-6"	9 SPA. @ 10" = 7'-6"	21 SPA. @ 10" = 17'-6"
"Y5"	3'-6"	3'-6"	3'-6"	3'-6"	3'-8"
"Y6"	60 SPA. @ 10" = 50'-0"	59 SPA. @ 10" = 49'-2"	46 SPA. @ 10" = 38'-4"	58 SPA. @ 10" = 48'-4"	57 SPA. @ 10" = 47'-6"
"Y7"	58 SPA. @ 8" = 38'-8"	58 SPA. @ 8" = 38'-8"	73 SPA. @ 8" = 48'-8"	57 SPA. @ 8" = 38'-0"	57 SPA. @ 8" = 38'-0"
"Y8"	3'-6"	3'-6"	3'-6"	3'-6"	3'-6"
"Y9"	5 SPA. @ 8" = 3'-4"	43 SPA. @ 8" = 28'-8"	41 SPA. @ 8" = 27'-4"	41 SPA. @ 8" = 27'-4"	58 SPA. @ 8" = 38'-8"
"Y10"	64 SPA. @ 10" = 53'-4"	32 SPA. @ 10" = 26'-8"	32 SPA. @ 10" = 26'-8"	31 SPA. @ 10" = 25'-10"	16 SPA. @ 10" = 13'-4"
"Y11"	3'-6"	3'-6"	3'-6"	3'-6"	3'-6"
"Y12"	42 SPA. @ 10" = 35'-0"	41 SPA. @ 10" = 34'-2"	44 SPA. @ 10" = 36'-8"	40 SPA. @ 10" = 33'-4"	41 SPA. @ 10" = 34'-2"
"Y13"	83 SPA. @ 8" = 55'-4"	82 SPA. @ 8" = 54'-8"	76 SPA. @ 8" = 50'-8"	79 SPA. @ 8" = 52'-8"	76 SPA. @ 8" = 50'-8"
"Y14"	3'-6"	3'-6"	3'-6"	3'-6"	3'-6"
"Y15"	112 SPA. @ 8" = 74'-8"	106 SPA. @ 8" = 70'-8"	101 SPA. @ 8" = 67'-4"	96 SPA. @ 8" = 64'-0"	93 SPA. @ 8" = 62'-0"
"Y16"	3'-6"	3'-6"	3'-6"	3'-8"	3'-6"
"Y17"	111 SPA. @ 8" = 74'-0"	104 SPA. @ 8" = 69'-4"	102 SPA. @ 8" = 68'-0"	101 SPA. @ 8" = 67'-4"	100 SPA. @ 8" = 66'-8"
"Y18"	21 SPA. @ 10" = 17'-6"	27 SPA. @ 10" = 22'-6"	36 SPA. @ 10" = 30'-0"	44 SPA. @ 10" = 36'-8"	51 SPA. @ 10" = 42'-6"
"Y19"	3'-6"	3'-6"	3'-6"	3'-6"	3'-6"
"Y20"	46 SPA. @ 10" = 38'-4"	62 SPA. @ 10" = 51'-8"	25 SPA. @ 10" = 20'-10"	35 SPA. @ 10" = 29'-2"	12 SPA. @ 10" = 10'-0"
"Y21"	70 SPA. @ 8" = 46'-8"	55 SPA. @ 8" = 36'-8"	97 SPA. @ 8" = 64'-8"	81 SPA. @ 8" = 54'-0"	106 SPA. @ 8" = 70'-8"
"Y22"	10 1/16"	9 5/16"	10 3/4"	7 1/16"	10 1/16"



NOTES:
 FOR GIRDER NOTES, SEE SHEET 22/41.

CAMBER AND DEFLECTIONS

GIRDER	POINT	CL BRG. R. A.	SPAN 1									CL PIER 1
			0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	
GIRDER A	DEFLECTION DUE TO WEIGHT OF STEEL	0	1/16"	3/16"	1/4"	1/4"	1/4"	3/16"	3/16"	1/8"	1/16"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	3/8"	1/16"	7/8"	1"	15/16"	13/16"	5/8"	3/8"	3/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/4"	15/16"	13/16"	2 1/16"	2 3/16"	2 1/16"	1 5/16"	1 7/16"	3/4"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/8"	1/4"	5/16"	5/16"	5/16"	1/4"	3/16"	1/8"	1/16"	0
	TOTAL CAMBER	0	1 5/16"	2 3/8"	3 3/16"	3 5/8"	3 1/16"	3 3/8"	2 3/16"	2 1/16"	1"	0
GIRDER B	DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/16"	1/4"	1/4"	1/4"	3/16"	1/8"	1/16"	0	
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	3/8"	1/16"	7/8"	1"	1"	7/8"	5/8"	7/16"	3/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/4"	1 3/16"	1 1/16"	1 5/16"	1 5/16"	2 1/16"	1 1/16"	1 5/16"	3/4"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/8"	3/16"	5/16"	5/16"	5/16"	1/4"	3/16"	1/8"	1/16"	0
	TOTAL CAMBER	0	1 5/16"	2 1/4"	3 1/8"	3 1/2"	3 1/2"	3 3/8"	2 1/16"	1 5/16"	1"	0
GIRDER C	DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/16"	1/4"	1/4"	1/4"	3/16"	1/8"	1/16"	0	
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	3/8"	1/16"	15/16"	1 1/16"	1"	7/8"	1 1/16"	7/16"	3/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/4"	1 3/16"	1 1/16"	1 5/16"	1 5/16"	1 5/16"	1 1/16"	1 5/16"	5/8"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/8"	3/16"	1/4"	5/16"	5/16"	1/4"	3/16"	1/8"	1/16"	0
	TOTAL CAMBER	0	1 5/16"	2 5/16"	3 1/8"	3 1/2"	3 1/2"	3 5/16"	2 3/4"	2"	7/8"	0
GIRDER D	DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/16"	1/4"	1/4"	1/4"	3/16"	1/8"	1/16"	0	
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	3/8"	3/4"	15/16"	1 1/16"	1 1/16"	5/16"	1 1/16"	7/16"	3/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	5/8"	1 3/16"	1 9/16"	1 5/16"	1 5/16"	1 9/16"	1 5/16"	3/4"	0	
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/8"	3/16"	1/4"	5/16"	5/16"	1/4"	3/16"	1/8"	1/16"	0
	TOTAL CAMBER	0	1 3/16"	2 5/16"	3 1/16"	3 9/16"	3 9/16"	3 3/8"	2 5/8"	2"	1"	0
GIRDER E	DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/16"	1/4"	5/16"	5/16"	1/4"	3/16"	1/8"	1/16"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	7/16"	3/4"	1 1/16"	1 3/16"	1 3/16"	1"	3/4"	1/2"	3/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	5/8"	1 1/16"	1 9/16"	1 3/16"	1 5/16"	1 13/16"	1 9/16"	1 3/16"	3/4"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/8"	3/16"	1/4"	5/16"	5/16"	1/4"	3/16"	1/8"	1/16"	0
	TOTAL CAMBER	0	1 1/4"	2 1/4"	3 1/8"	3 9/16"	3 5/8"	3 5/16"	2 3/4"	1 5/16"	1 1/16"	0



CAMBER DIAGRAM
(GIRDER C SHOWN)

POINT	BEARING POINT			
	PIER 1	PIER 2	PIER 3	PIER 4
GIRDER A	3' - 9 3/8"	6' - 1 13/16"	7' - 0 5/8"	4' - 0 3/8"
GIRDER B	3' - 8 1/16"	5' - 11 3/4"	6' - 10 7/16"	4' - 2 3/4"
GIRDER C	3' - 6 13/16"	5' - 9 5/8"	6' - 8 1/4"	4' - 4 5/16"
GIRDER D	3' - 5 1/2"	5' - 7 9/16"	6' - 6 1/16"	4' - 5 1/4"
GIRDER E	3' - 4 5/16"	5' - 5 5/8"	6' - 4 1/16"	4' - 5 3/4"

NOTES:

1. NEGATIVE VALUES FOR DEFLECTIONS INDICATE DEFLECTIONS ABOVE CHORD LINE.
2. DEFLECTIONS AND ADJUSTMENT FOR VERTICAL CURVES ARE GIVEN TO THE NEAREST 1/16th INCH.
3. FOR CAMBER AND DEFLECTIONS, SPANS 2 THRU 5, SEE SHEETS [26/47] AND [27/47].

CAMBER AND DEFLECTIONS

GIRDER	POINT	SPAN 2											SPAN 3												
		☉ PIER 1	0.1	0.2	F.S. 1	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ PIER 2	0.1	F.S. 2	0.2	0.3	0.4	0.5	0.6	F.S. 3	0.7	0.8	0.9	☉ PIER 3
GIRDER A	DEFLECTION DUE TO WEIGHT OF STEEL	0	0	0	0	0	1/16"	1/16"	1/16"	0	0	0	0	1/16"	1/16"	1/16"	1/8"	1/8"	1/8"	1/16"	1/16"	0	-1/16"	-1/16"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	-1/16"	0	1/16"	1/16"	1/8"	1/8"	1/8"	1/16"	0	0	0	1/8"	5/16"	3/8"	9/16"	5/8"	1/2"	5/16"	5/16"	1/8"	-1/16"	-1/8"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/16"	17/16"	15/16"	15/16"	23/16"	23/16"	21/16"	15/16"	17/16"	3/4"	0	17/16"	21/16"	23/16"	31/8"	35/8"	33/4"	35/8"	33/4"	31/8"	23/8"	15/16"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	-1/16"	0	1/16"	1/16"	1/8"	1/8"	1/8"	1/16"	0	0	0	1/16"	1/8"	1/8"	3/16"	3/16"	3/16"	1/8"	1/16"	0	-1/16"	-1/16"	0
	TOTAL CAMBER	0	3/4"	13/8"	2"	2"	23/8"	27/16"	25/16"	21/16"	11/2"	1/16"	0	11/16"	21/2"	25/16"	4"	49/16"	49/16"	41/8"	41/8"	31/4"	21/4"	11/16"	0
GIRDER B	DEFLECTION DUE TO WEIGHT OF STEEL	0	0	0	0	0	0	1/16"	1/16"	0	0	0	0	1/16"	1/16"	1/16"	1/8"	1/8"	1/8"	1/16"	1/16"	0	0	-1/16"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	-1/16"	-1/16"	0	0	1/16"	1/8"	1/8"	1/16"	0	0	0	1/8"	1/4"	5/16"	1/2"	9/16"	1/2"	3/8"	5/16"	1/8"	-1/16"	-1/8"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/4"	15/16"	11/16"	13/16"	21/16"	21/16"	15/16"	11/16"	15/16"	3/4"	0	15/16"	15/16"	21/4"	3"	33/8"	35/8"	31/2"	31/2"	3"	21/4"	13/16"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	-1/16"	-1/16"	0	0	1/16"	1/8"	1/8"	1/16"	0	0	0	1/16"	1/16"	1/8"	3/16"	3/16"	3/16"	1/8"	1/8"	1/16"	0	-1/16"	0
	TOTAL CAMBER	0	3/16"	11/4"	11/16"	13/16"	23/16"	25/16"	21/8"	13/16"	13/8"	1/16"	0	19/16"	23/8"	23/16"	33/16"	45/16"	47/16"	41/16"	4"	33/16"	23/16"	1"	0
GIRDER C	DEFLECTION DUE TO WEIGHT OF STEEL	0	0	0	0	0	0	0	0	0	0	0	0	1/16"	1/16"	1/16"	1/8"	1/8"	1/8"	1/16"	1/16"	0	0	-1/16"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	-1/16"	-1/16"	0	0	1/16"	1/16"	1/16"	1/16"	0	0	0	1/8"	1/4"	5/16"	1/2"	9/16"	9/16"	3/8"	3/8"	3/16"	0	-1/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	5/8"	13/16"	11/16"	11/16"	15/16"	21/16"	15/16"	11/16"	15/16"	3/4"	0	13/16"	15/16"	23/16"	23/4"	31/4"	33/8"	31/4"	31/4"	23/4"	23/16"	11/16"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	-1/16"	-1/16"	0	0	1/16"	1/16"	1/16"	1/16"	0	0	0	1/16"	1/16"	1/8"	3/16"	3/16"	3/16"	1/8"	1/8"	1/16"	0	0	0
	TOTAL CAMBER	0	7/16"	11/16"	15/8"	15/8"	21/16"	21/4"	21/8"	13/16"	15/16"	1/16"	0	17/16"	25/16"	21/16"	39/16"	43/16"	41/4"	37/8"	33/16"	3"	21/8"	15/16"	0
GIRDER D	DEFLECTION DUE TO WEIGHT OF STEEL	0	0	0	0	0	0	0	0	0	0	0	0	1/16"	1/16"	1/16"	1/8"	1/8"	1/8"	1/16"	1/16"	0	0	0	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	-1/16"	-1/16"	-1/16"	-1/16"	0	1/16"	1/16"	1/16"	0	0	0	1/8"	1/4"	5/16"	1/2"	5/8"	9/16"	7/16"	3/8"	3/16"	0	-1/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/4"	15/16"	19/16"	11/16"	15/16"	15/16"	13/16"	11/16"	13/16"	3/4"	0	13/16"	13/16"	21/16"	23/4"	31/8"	31/8"	31/8"	3"	23/4"	21/16"	11/16"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/16"	1/16"	1/16"	1/16"	0	-1/16"	-1/16"	-1/16"	0	0	0	1/16"	1/8"	1/8"	3/16"	3/16"	3/16"	1/8"	1/8"	1/16"	0	0	0
	TOTAL CAMBER	0	1/16"	15/16"	19/16"	11/16"	15/16"	15/16"	13/16"	11/16"	13/16"	1/16"	0	17/16"	21/4"	29/16"	39/16"	41/16"	4"	33/4"	35/8"	31/16"	21/16"	15/16"	0
GIRDER E	DEFLECTION DUE TO WEIGHT OF STEEL	0	0	0	0	0	0	0	0	0	0	0	0	1/16"	1/16"	1/16"	1/8"	3/16"	1/8"	1/8"	1/16"	0	0	0	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	-1/8"	-1/8"	-1/16"	-1/16"	0	0	0	0	0	0	0	1/8"	5/16"	3/8"	9/16"	5/8"	5/8"	7/16"	7/16"	1/4"	1/16"	-1/16"	0
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	3/4"	11/16"	19/16"	19/16"	13/16"	13/16"	11/16"	19/16"	13/16"	5/8"	0	11/16"	11/16"	15/16"	21/2"	27/8"	3"	3"	23/4"	21/2"	15/16"	11/16"	0
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/16"	1/16"	1/16"	1/16"	0	0	0	0	0	0	0	1/16"	1/8"	1/8"	3/16"	3/16"	3/16"	1/8"	1/8"	1/16"	0	0	0
	TOTAL CAMBER	0	1/16"	1"	11/2"	11/2"	13/16"	13/16"	11/16"	19/16"	13/16"	9/16"	0	15/16"	21/8"	21/2"	33/8"	37/8"	35/16"	31/16"	37/16"	27/8"	2"	1"	0

NOTES:
 1. FOR CAMBER AND DEFLECTIONS, SPANS 1 AND SPANS 4 THRU 5, SEE SHEETS [25/41] AND [27/41].

DESIGN AGENCY
TANS SYSTEMS CORPORATION
 58 BARBERS CHURCH DRIVE - SUITE 400
 SUDBURY, ONTARIO M3H 5B9

DATE: 7/19/05
 REVIEWED BY: RER
 STRUCTURE FILE NUMBER: 2511452

DESIGNED BY: BTA
 CHECKED BY: NFF

DRAWN BY: BTA
 REVISED BY:

CAMBER AND DEFLECTIONS - SPANS 2 & 3
 BRIDGE NO. FRA-270-2583A
 RAMP P OVER IR-71

FRA-270-24.47
 PID 77320

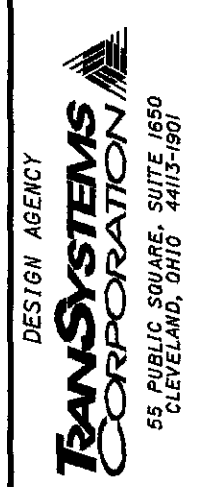
26 / 41
 794
 846

CAMBER AND DEFLECTIONS

GIRDER	POINT	SPAN 4												SPAN 5												Q BRG. F.A.
		Q PIER 3	0.1	0.2	F.S. 4	0.3	0.4	0.5	0.6	F.S. 5	0.7	0.8	0.9	Q PIER 4	0.1	0.2	F.S. 6	0.3	0.4	0.5	0.6	0.7	0.8	0.9		
GIRDER A	DEFLECTION DUE TO WEIGHT OF STEEL	0	3/16"	1/2"	9/16"	3/4"	7/8"	7/8"	3/4"	9/16"	1/2"	3/16"	1/16"	0	1/16"	1/8"	1/4"	1/4"	3/8"	7/16"	7/16"	3/8"	5/16"	3/16"	0	
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	9/16"	17/16"	13/4"	25/16"	23/4"	23/4"	23/16"	11/16"	13/8"	9/16"	1/16"	0	3/16"	9/16"	13/16"	5/16"	15/16"	19/16"	19/16"	17/16"	17/16"	9/16"	0	
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	23/4"	43/16"	53/16"	61/4"	71/16"	77/16"	73/16"	75/16"	61/4"	43/16"	25/8"	0	1/4"	1/8"	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/8"	5/16"	3/8"	1/2"	5/8"	5/8"	1/2"	3/8"	5/16"	1/8"	0	0	1/16"	1/8"	1/8"	3/16"	1/4"	5/16"	5/16"	1/4"	3/16"	1/8"	0	
	TOTAL CAMBER	0	31/16"	71/16"	77/8"	93/16"	113/8"	111/16"	105/8"	109/16"	83/8"	51/16"	23/4"	0	9/16"	15/16"	13/16"	17/16"	15/16"	21/4"	25/16"	21/16"	19/16"	7/8"	0	
	GIRDER B	DEFLECTION DUE TO WEIGHT OF STEEL	0	3/16"	3/8"	1/2"	5/8"	13/16"	13/16"	3/4"	5/8"	1/2"	5/16"	1/16"	0	0	1/16"	3/16"	3/16"	5/16"	3/8"	3/8"	3/8"	1/4"	1/8"	0
DEFLECTION DUE TO REMAINING DEAD LOAD		0	1/2"	11/4"	19/16"	2"	21/2"	29/16"	21/4"	113/16"	19/16"	13/16"	1/4"	0	1/16"	5/16"	5/8"	11/16"	1"	11/4"	15/16"	11/4"	1"	9/16"	0	
ADJUSTMENT REQUIRED FOR VERTICAL CURVE		0	23/8"	45/16"	43/16"	55/8"	61/2"	65/8"	63/8"	63/4"	55/8"	45/16"	23/8"	0	1/2"	5/8"	5/8"	5/8"	1/8"	0	0	0	0	0	0	
ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
ADJUSTMENT REQUIRED FOR HEAT CURVING		0	1/8"	1/4"	5/16"	7/16"	1/2"	1/2"	7/16"	3/8"	5/16"	3/16"	1/16"	0	0	1/16"	3/16"	3/16"	1/4"	3/8"	3/8"	3/8"	1/4"	1/8"	0	
TOTAL CAMBER		0	31/8"	61/4"	73/16"	81/16"	109/16"	109/16"	93/4"	91/2"	81/16"	59/16"	23/4"	0	5/8"	11/8"	19/16"	15/8"	11/16"	15/16"	21/8"	15/16"	11/2"	13/16"	0	
GIRDER C		DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/8"	1/2"	5/8"	3/4"	13/16"	3/4"	5/8"	9/16"	5/16"	1/8"	0	0	1/16"	1/8"	3/16"	1/4"	5/16"	3/8"	3/8"	1/4"	1/8"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	7/16"	11/16"	11/2"	17/8"	23/8"	21/2"	21/4"	17/8"	111/16"	15/16"	5/16"	0	0	1/4"	9/16"	11/16"	15/16"	13/16"	15/16"	11/4"	15/16"	1/2"	0	
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	21/16"	313/16"	45/16"	415/16"	53/4"	57/8"	53/4"	55/8"	51/16"	313/16"	23/16"	0	3/4"	11/16"	13/16"	11/16"	11/16"	1/2"	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/16"	3/16"	1/4"	5/16"	7/16"	7/16"	3/8"	5/16"	5/16"	3/16"	1/16"	0	0	1/16"	3/16"	1/4"	5/16"	3/8"	7/16"	7/16"	5/16"	3/16"	0	
	TOTAL CAMBER	0	21/16"	59/16"	69/16"	73/4"	95/16"	91/16"	91/8"	87/16"	79/16"	55/16"	21/16"	0	3/4"	17/16"	21/8"	23/16"	29/16"	23/8"	21/8"	2"	19/16"	7/8"	0	
	GIRDER D	DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/8"	1/2"	5/8"	3/4"	13/16"	3/4"	11/16"	5/8"	3/8"	1/8"	0	0	0	1/8"	1/4"	1/4"	3/8"	3/8"	3/8"	5/16"	3/16"	0
DEFLECTION DUE TO REMAINING DEAD LOAD		0	7/16"	11/16"	11/2"	17/8"	23/8"	25/8"	23/8"	2"	113/16"	11/8"	7/16"	0	-1/16"	1/8"	1/2"	13/16"	15/16"	11/4"	17/16"	13/8"	11/16"	9/16"	0	
ADJUSTMENT REQUIRED FOR VERTICAL CURVE		0	15/16"	31/2"	43/16"	49/16"	51/4"	53/8"	53/16"	45/16"	49/16"	31/2"	15/16"	0	13/16"	19/16"	111/16"	113/16"	111/16"	17/16"	13/16"	0	0	0	0	
ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
ADJUSTMENT REQUIRED FOR HEAT CURVING		0	1/16"	3/16"	3/16"	1/4"	3/8"	3/8"	5/16"	5/16"	1/4"	1/8"	1/16"	0	0	1/16"	3/16"	5/16"	5/16"	7/16"	1/2"	1/2"	3/8"	3/16"	0	
TOTAL CAMBER		0	29/16"	51/8"	63/8"	75/16"	813/16"	93/16"	85/8"	77/8"	71/4"	51/8"	29/16"	0	3/4"	13/4"	21/2"	31/8"	33/16"	31/2"	33/16"	21/4"	13/4"	15/16"	0	
GIRDER E		DEFLECTION DUE TO WEIGHT OF STEEL	0	1/8"	3/8"	1/2"	5/8"	13/16"	15/16"	7/8"	3/4"	11/16"	7/16"	3/16"	0	-1/16"	-1/16"	1/16"	1/4"	1/4"	3/8"	1/2"	1/2"	7/16"	1/4"	0
	DEFLECTION DUE TO REMAINING DEAD LOAD	0	3/8"	11/16"	11/2"	17/8"	29/16"	27/16"	211/16"	29/16"	21/8"	15/16"	9/16"	0	-3/16"	-1/16"	5/16"	13/16"	15/16"	111/16"	13/4"	17/16"	13/16"	0		
	ADJUSTMENT REQUIRED FOR VERTICAL CURVE	0	111/16"	31/4"	33/16"	43/16"	413/16"	415/16"	413/16"	45/16"	43/16"	31/8"	113/16"	0	13/16"	15/16"	23/8"	21/2"	21/2"	23/8"	113/16"	111/16"	0	0		
	ADJUSTMENT REQUIRED FOR HORIZONTAL CURVE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	ADJUSTMENT REQUIRED FOR HEAT CURVING	0	1/16"	1/8"	3/16"	3/16"	5/16"	5/16"	5/16"	1/4"	1/4"	1/8"	1/16"	0	-1/16"	0	1/16"	1/4"	1/4"	7/16"	9/16"	9/16"	7/16"	1/4"	0	
	TOTAL CAMBER	0	23/16"	43/4"	6"	67/8"	87/16"	9"	85/8"	75/8"	71/4"	51/16"	25/8"	0	7/8"	113/16"	21/8"	313/16"	4"	49/16"	41/2"	37/8"	25/16"	15/16"	0	

NOTES:

1. FOR CAMBER AND DEFLECTIONS, SPANS 1 AND SPANS 2 THRU 3, SEE SHEETS [25/41] AND [26/41].



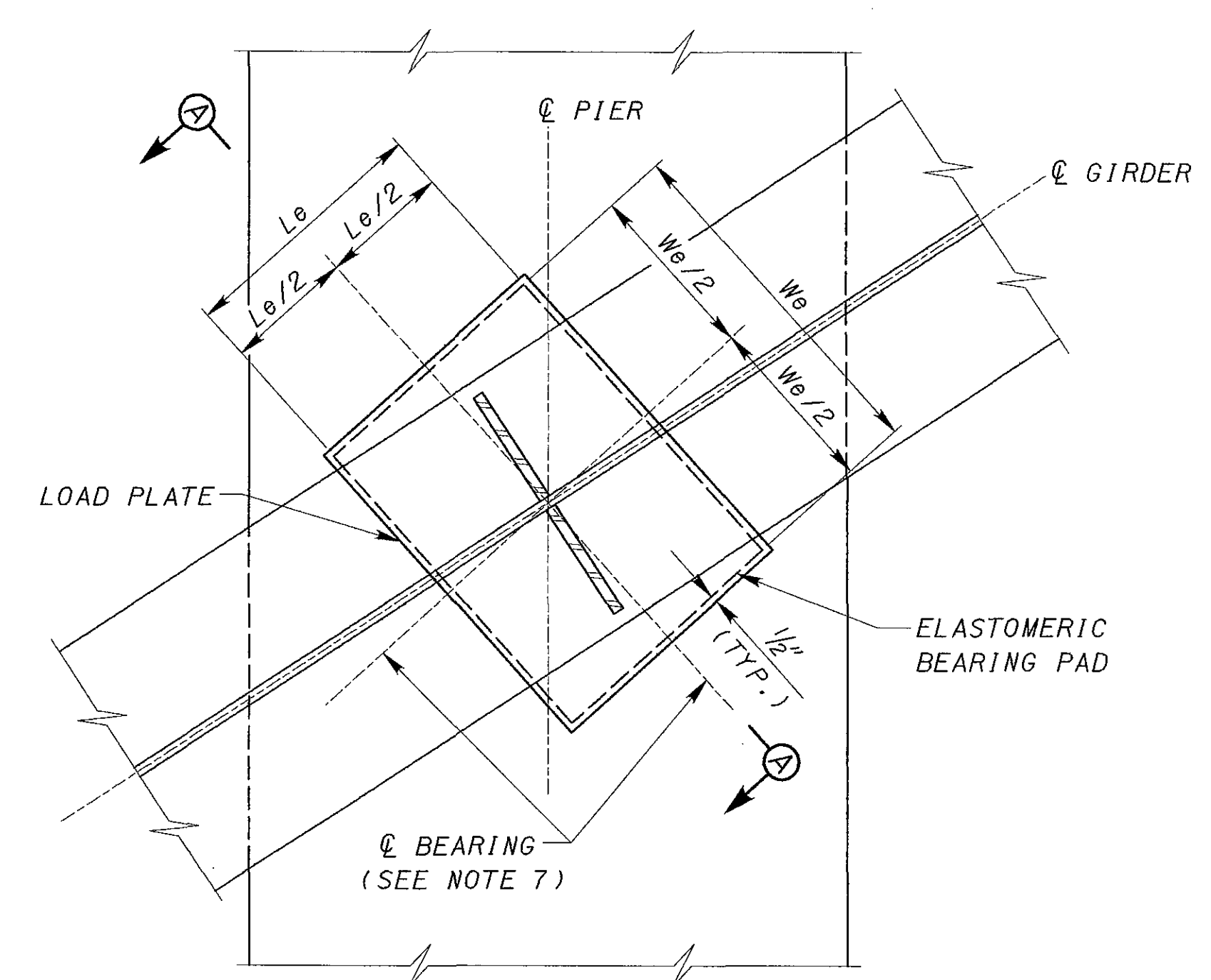
DESIGN AGENCY
TRANS SYSTEMS CORPORATION
58 PUBLIC SQUARE, SUITE 1650
CLEVELAND, OHIO 44115-9971

DATE 7/19/05
REVIEWED BY RER
STRUCTURE FILE NUMBER 2511452

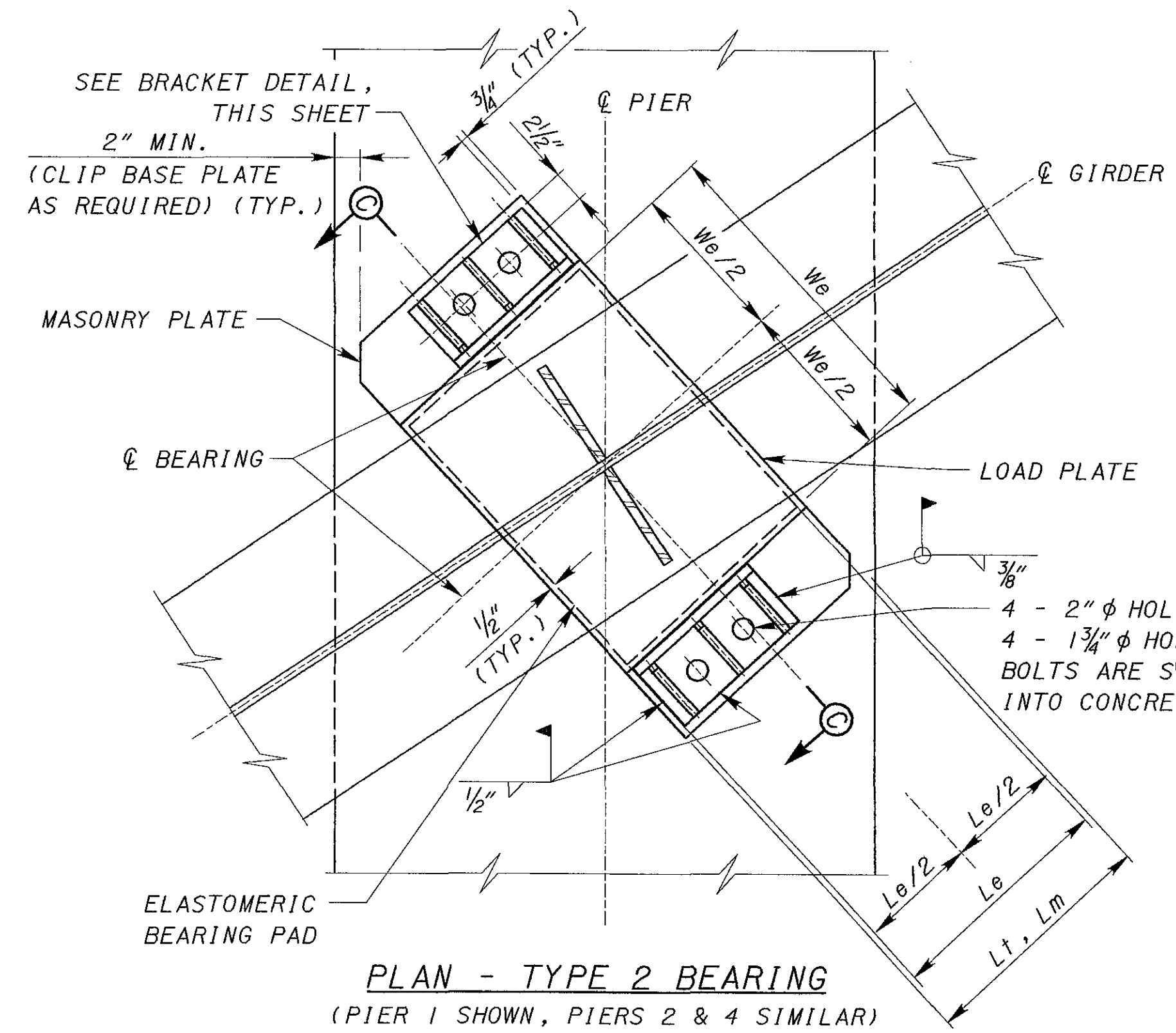
DRAWN BY BTA
CHECKED BY NFF

CAMBER AND DEFLECTIONS - SPANS 4 & 5
BRIDGE NO. FRA-270-2583A
RAMP P OVER IR-71

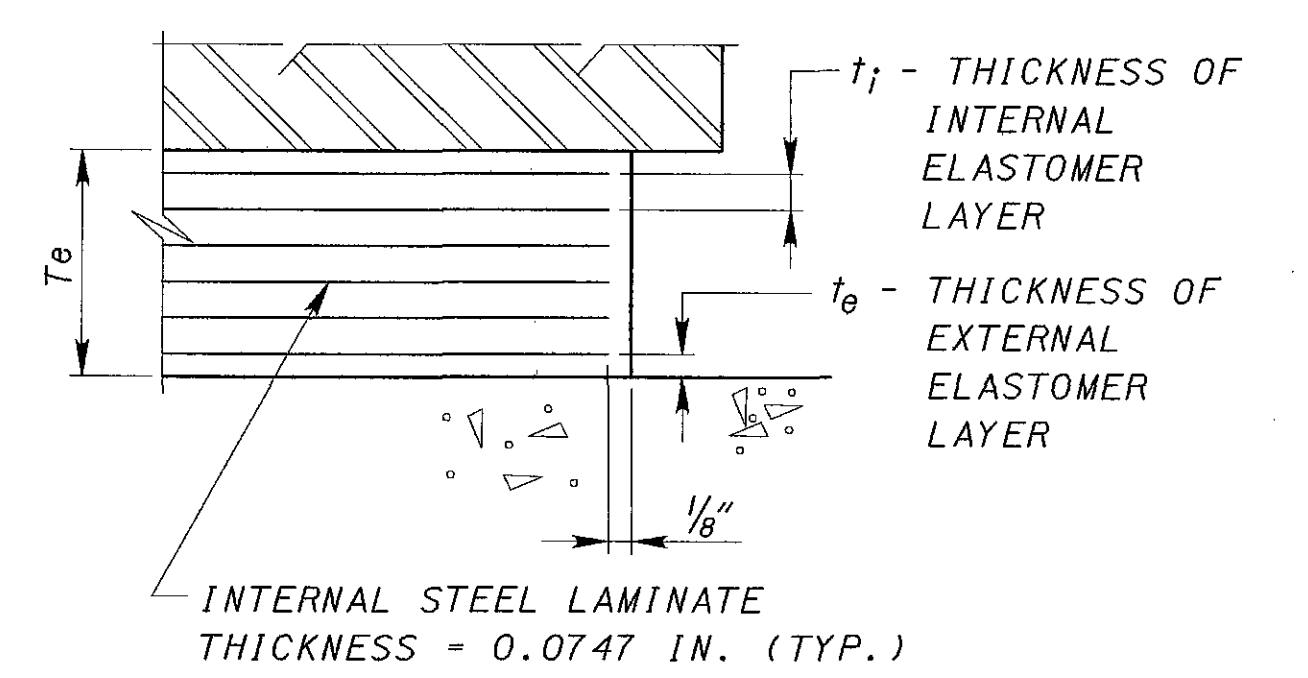
FRA-270-24.47
PID 77320



PLAN - TYPE 1 BEARING
 (PIER 1 SHOWN, PIERS 2 & 4 SIMILAR)



PLAN - TYPE 2 BEARING
 (PIER 1 SHOWN, PIERS 2 & 4 SIMILAR)



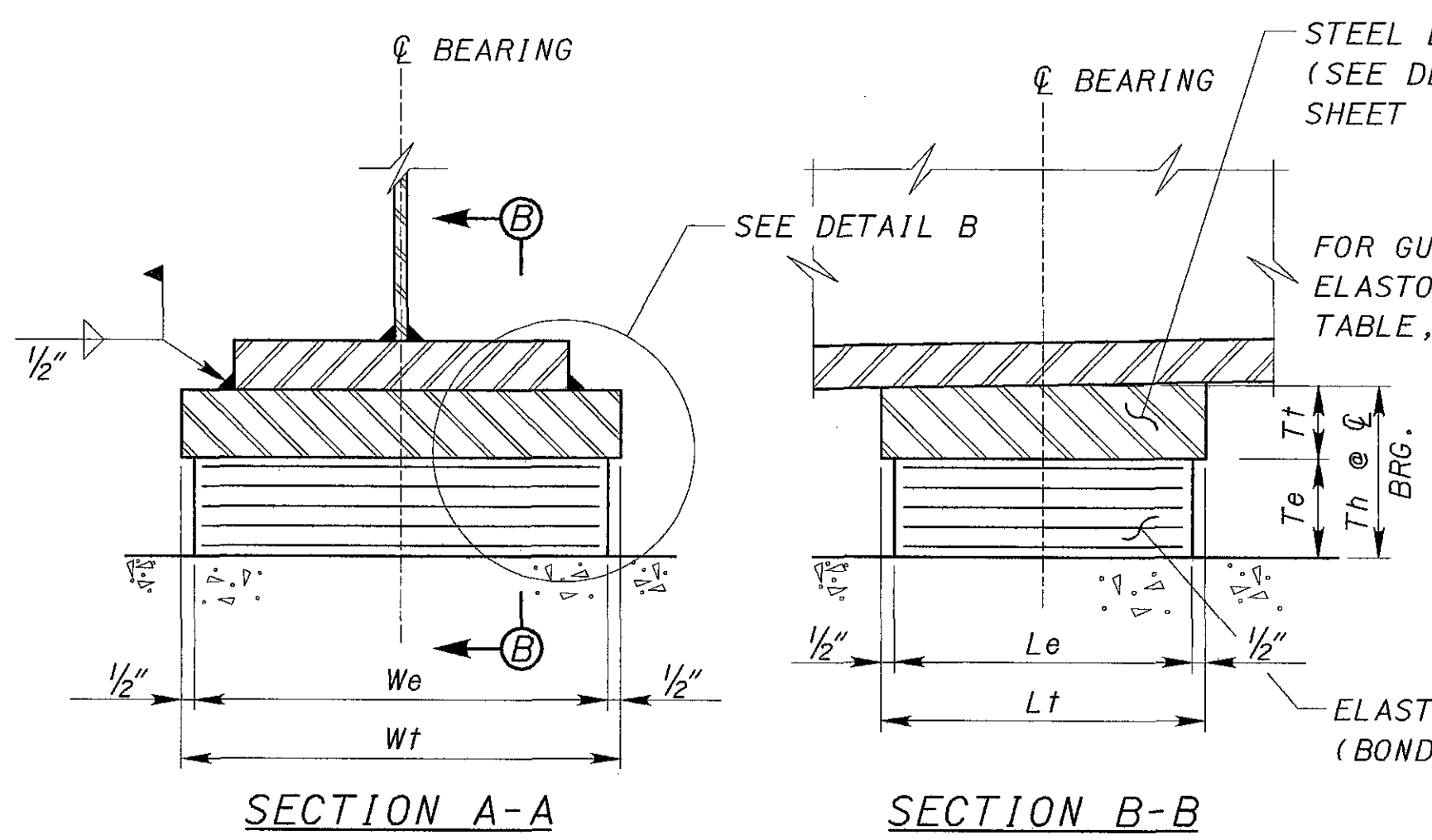
DETAIL B
 (MASONRY PLATE NOT SHOWN FOR CLARITY)

4 - 2" ϕ HOLES FOR 1/2" ϕ BOLTS (PIER 4)
 4 - 1 3/4" ϕ HOLES FOR 1 1/4" ϕ BOLTS (PIERS 1 & 2)
 BOLTS ARE SWEDGED ANCHOR BOLTS SET 1'-10" INTO CONCRETE (SEE NOTES 7 & 8)

NOTES:

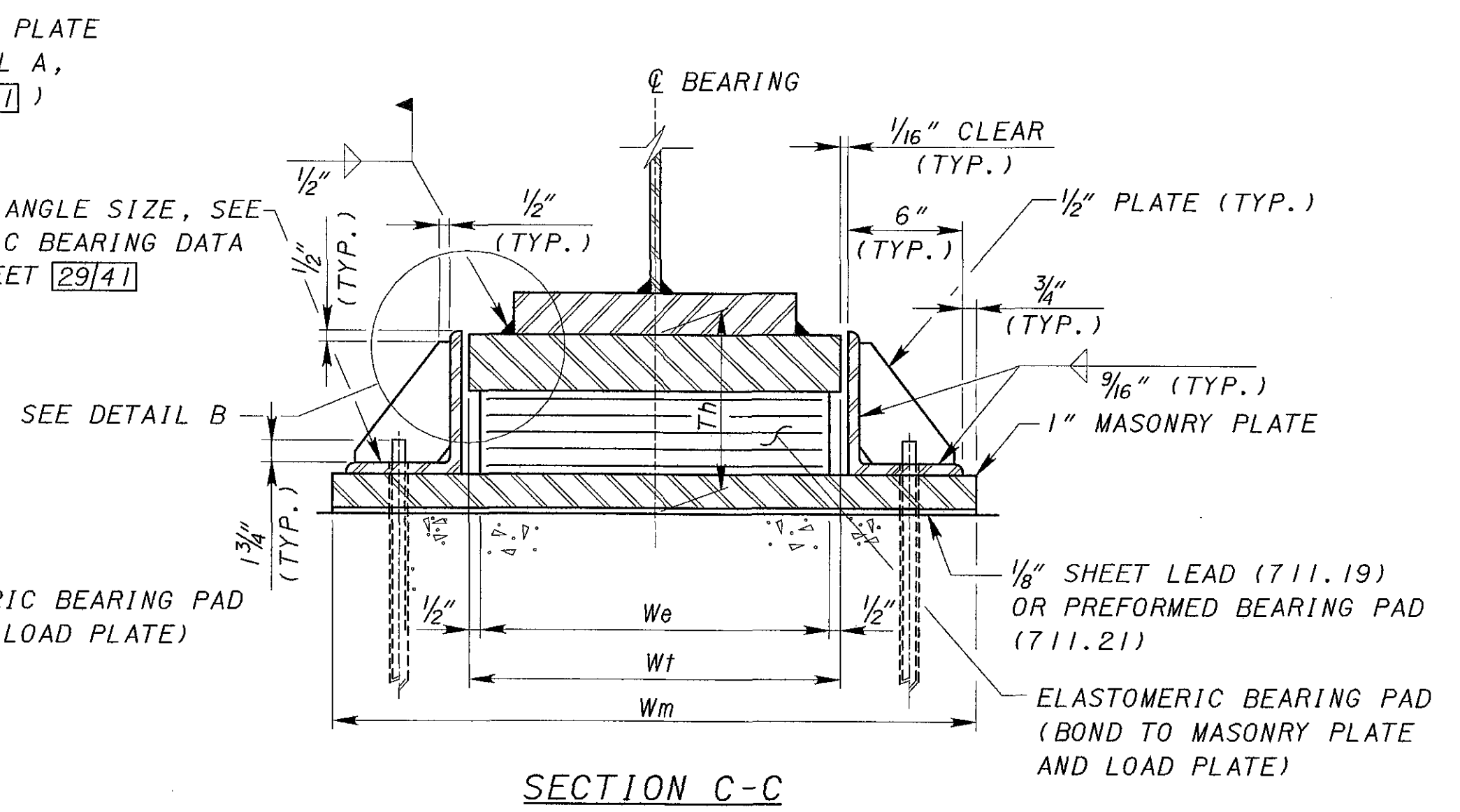
- ELASTOMERIC BEARINGS: THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION 1, SECTION 14.6.6 (METHOD A) OF THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.
- THE STEEL LOAD PLATES AND STEEL MASONRY PLATES (IF APPLICABLE) SHALL BE BONDED BY VULCANIZATION TO THE ELASTOMER. CONTROL WELDING OF LOAD PLATE TO THE SUPERSTRUCTURE SO THAT THE PLATE TEMPERATURE AT THE ELASTOMER BONDED SURFACE DOES NOT EXCEED 300°F AS DETERMINED BY THE USE OF PYROMETRIC STICKS OR OTHER TEMPERATURE MONITORING DEVICES.
- BEARING REPOSITIONING: IF THE STEEL IS ERECTED AT AN AMBIENT TEMPERATURE HIGHER THAN 80°F OR LOWER THAN 40°F AND THE BEARING SHEAR DEFLECTION EXCEEDS 1/6 OF THE BEARING HEIGHT AT 60°F \pm 10°F, RAISE THE GIRDERS TO ALLOW THE BEARINGS TO RETURN TO THEIR UNDERFORMED SHAPE AT 60°F \pm 10°F.
- BASIS OF PAYMENT - THE UNIT BID PRICE SHALL INCLUDE ALL MATERIALS LABOR, TESTING, PAINTING AND INCIDENTALS NECESSARY TO FURNISH AND INSTALL LAMINATED ELASTOMERIC BEARINGS. PAYMENT WILL BE MADE AT THE CONTRACT PRICE FOR ITEM 516, EACH, ELASTOMERIC BEARINGS WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE).
- BRIDGE SEAT ELEVATIONS HAVE BEEN ADJUSTED UPWARD TO COMPENSATE FOR THE VERTICAL DEFORMATIONS OF THE BEARINGS, AS FOLLOWS:

SEAT ADJUSTMENT (in.)					
GIRDER	A	B	C	D	E
PIER 1	1/8	3/16	1/8	3/16	1/8
PIER 2	-	1/8	1/8	1/8	-
PIER 3	-	-	-	-	-
PIER 4	3/16	-	-	-	3/16

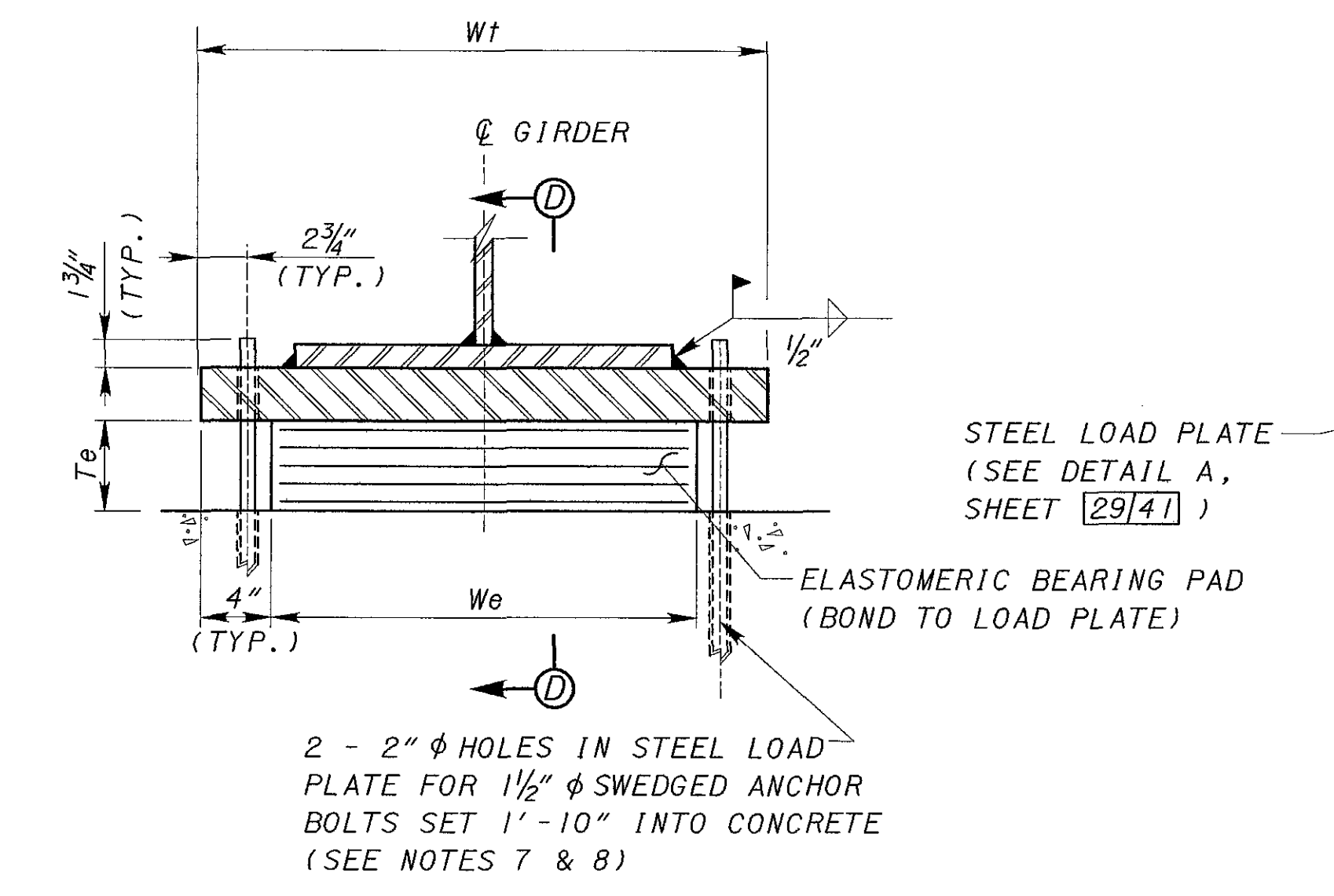


SECTION A-A

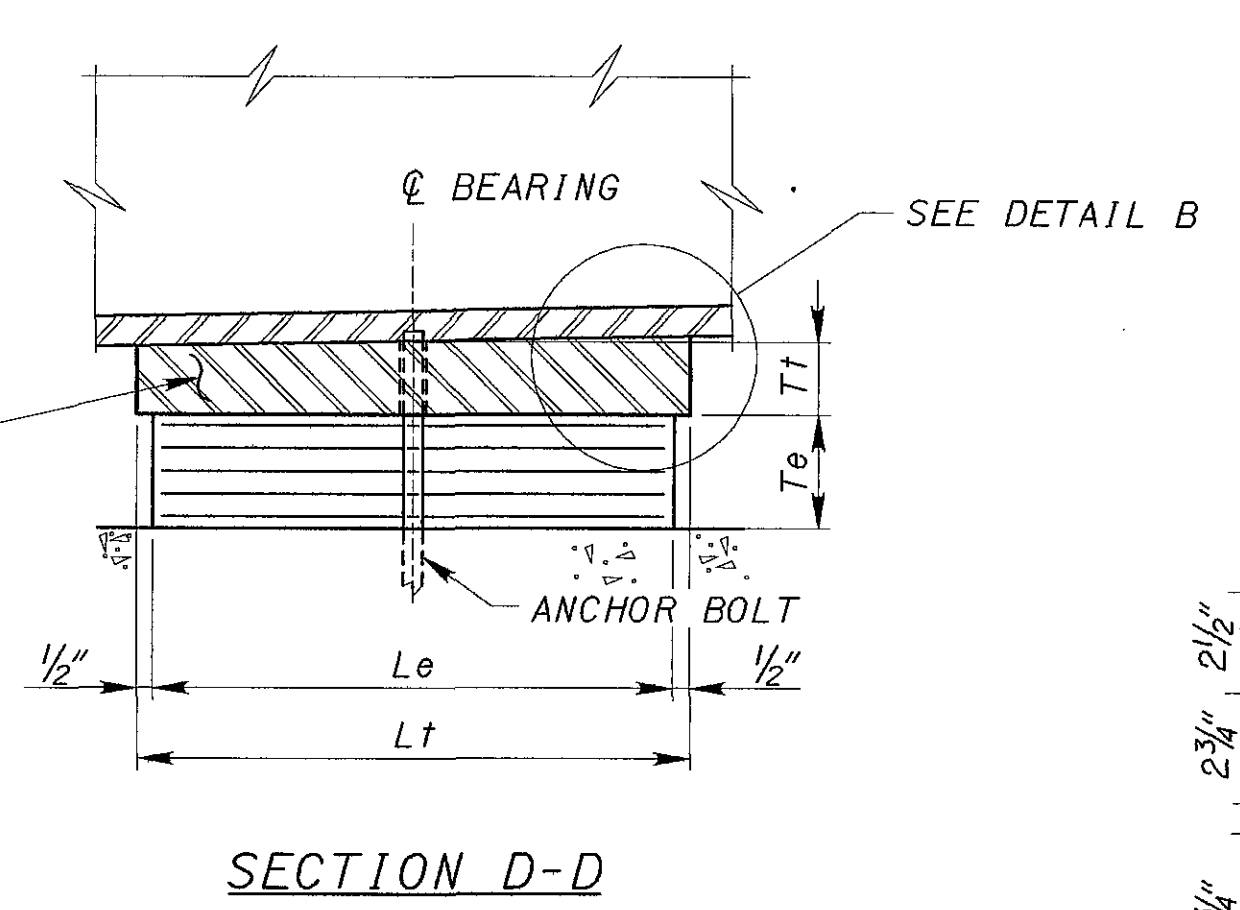
SECTION B-B



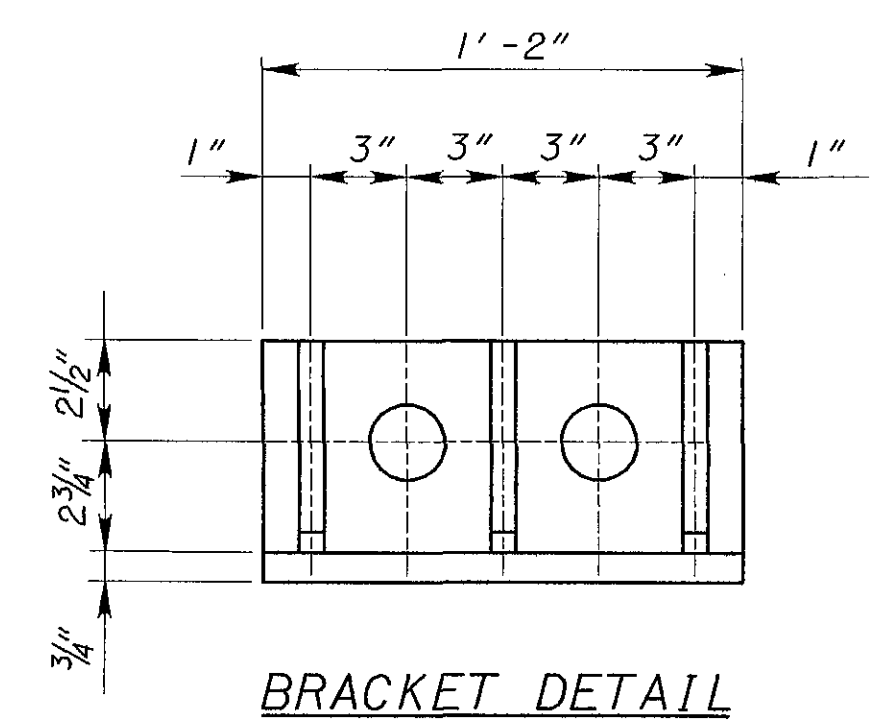
SECTION C-C



ELEVATION - TYPE 3 BEARING
 (PIER 3)

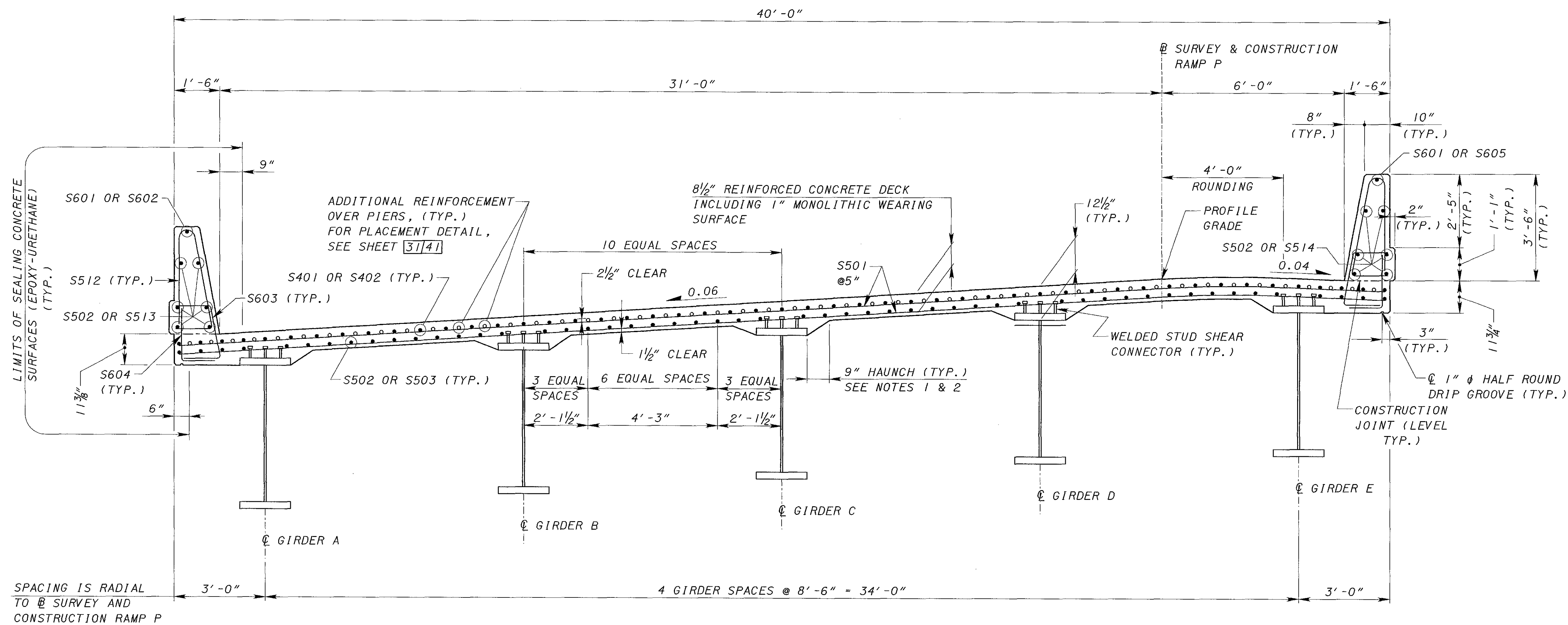


SECTION D-D



BRACKET DETAIL

- AT THE OPTION OF THE CONTRACTOR, THE BEARING ANCHOR RODS (OR FORMED HOLES), LOCATED AND SUPPORTED BY TEMPLATES, MAY BE CAST-IN-PLACE.
- FOR ORIENTATION AND DIRECTION OF BEARINGS AND ANCHOR ROD PLACEMENT DETAILS, SEE ABUTMENT DETAIL SHEETS [7/41] & [10/41] AND PIER DETAIL SHEETS [13/41] THRU [20/21].
- ANCHOR RODS SHALL BE INSTALLED PER ITEM 510 AND GALVANIZED PER ITEM 711.02. INCLUDE DOWEL HOLES AND ANCHOR RODS WITH ITEM 516 FOR PAYMENT.
- FOR STEEL LOAD PLATE DATA, SEE SHEET [29/41].
- FOR ADDITIONAL NOTES, SEE GENERAL NOTES SHEET [3/41].

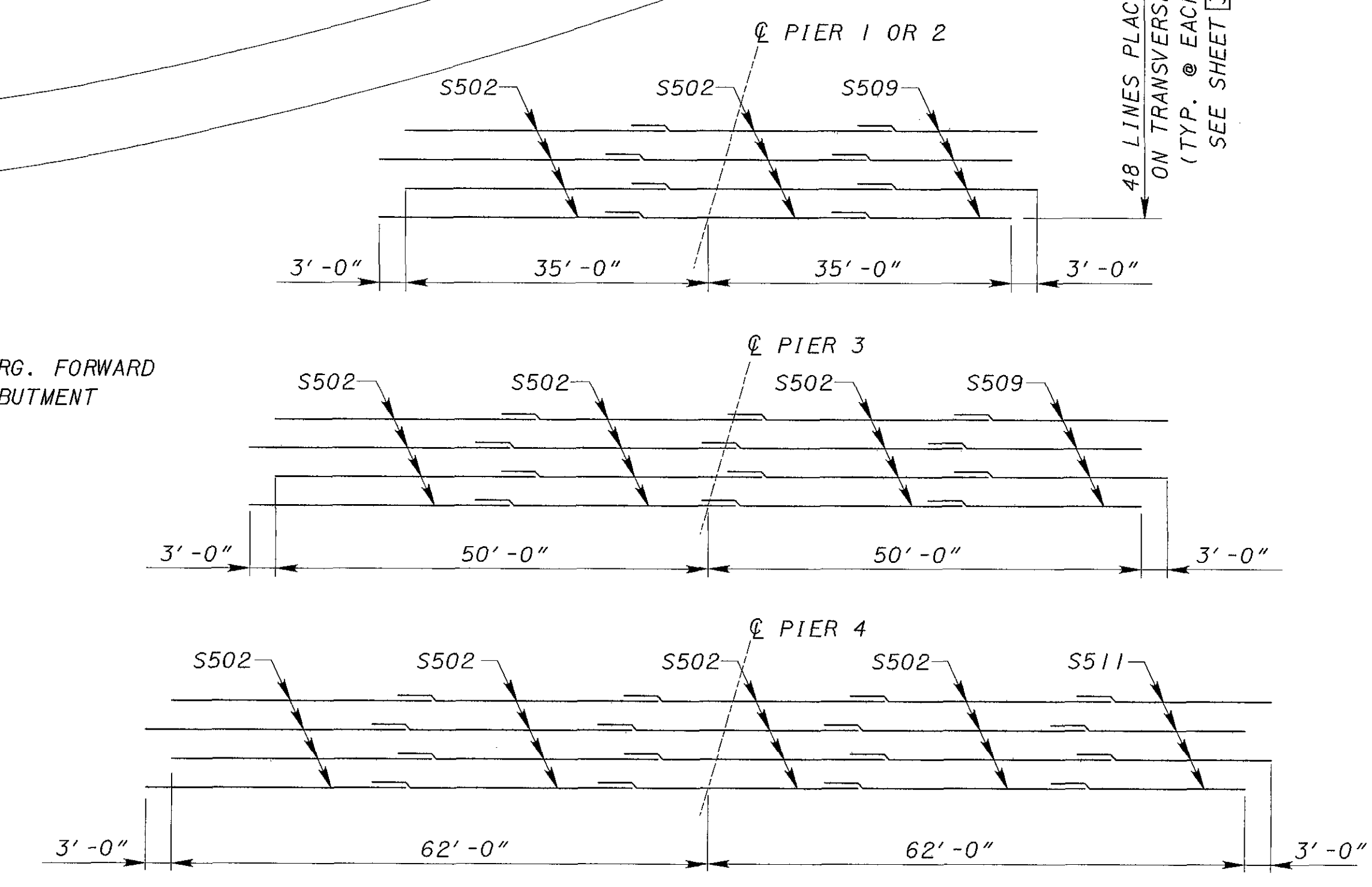
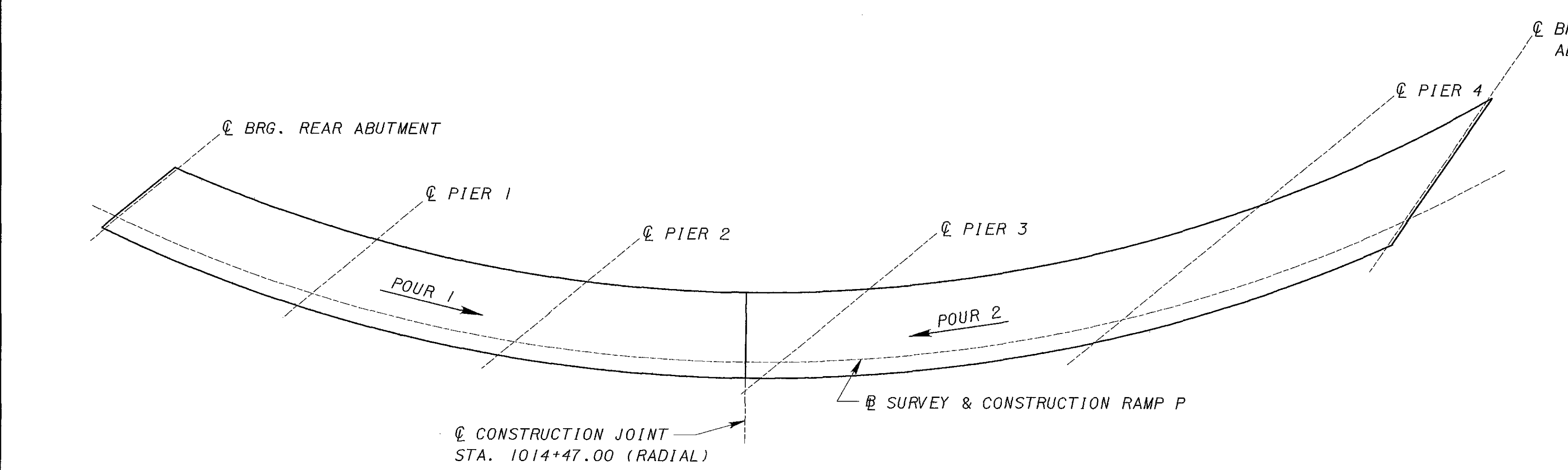
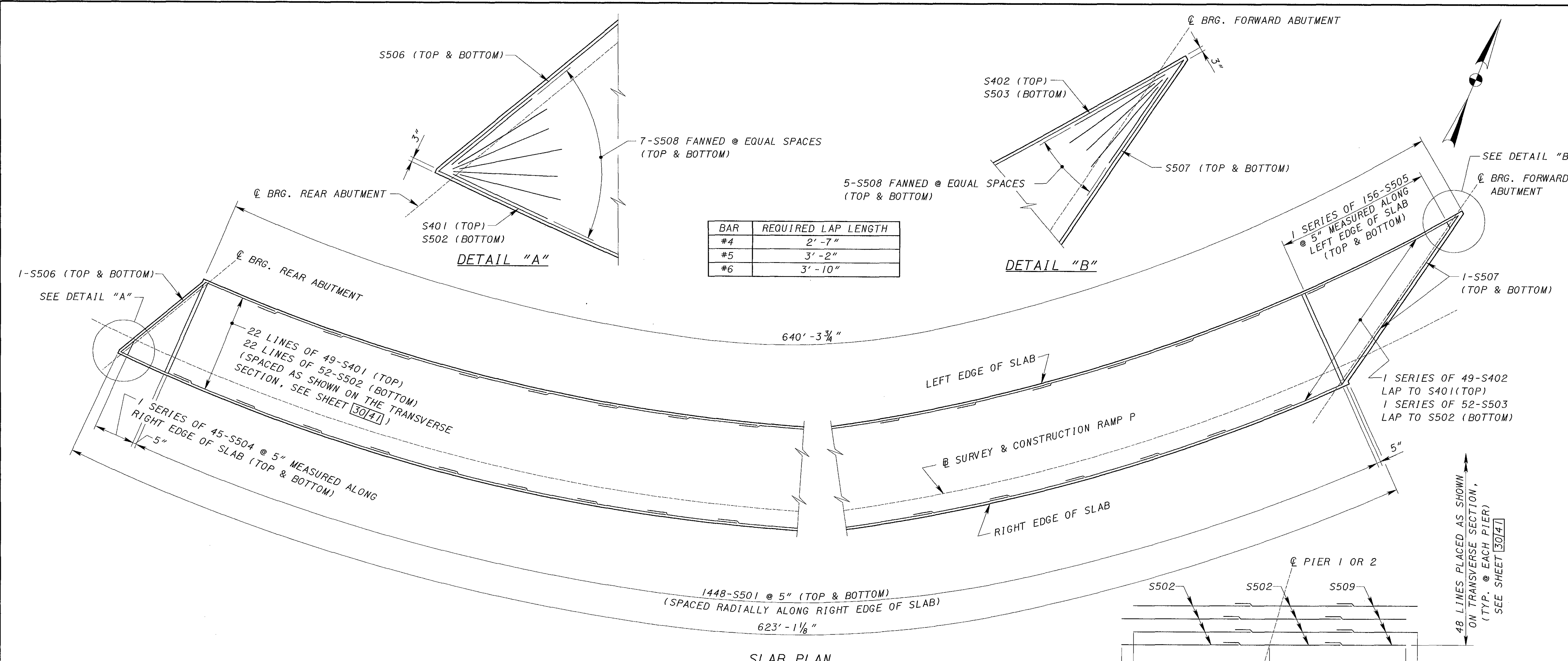


SPACING IS RADIAL TO @ SURVEY AND CONSTRUCTION RAMP P

TRANSVERSE SECTION

NOTES:

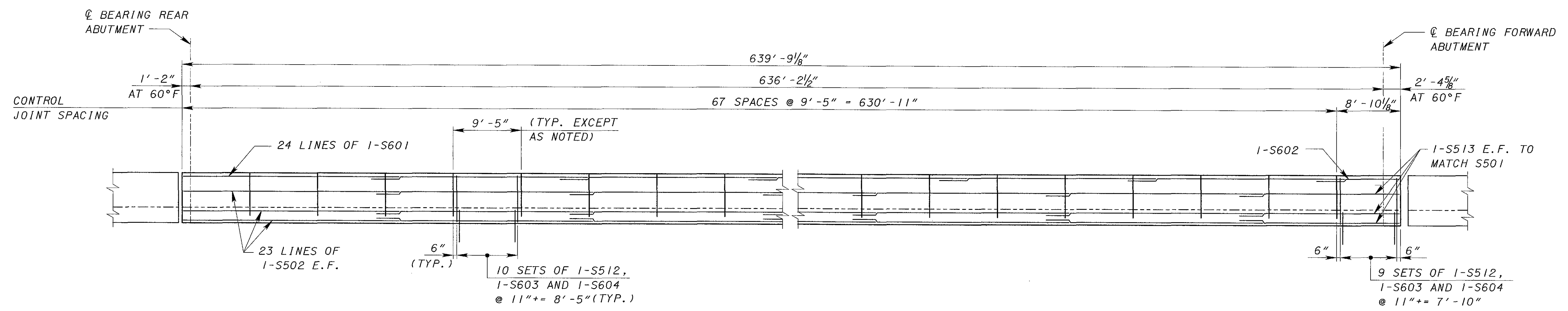
1. DECK SLAB CONCRETE QUANTITY: THE ESTIMATED QUANTITY OF DECK SLAB CONCRETE IS BASED ON THE CONSTANT DECK SLAB THICKNESS, AS SHOWN, PLUS THE QUANTITY OF CONCRETE THAT FORMS EACH GIRDER HAUNCH. THE ESTIMATE ASSUMES A CONSTANT HAUNCH THICKNESS OF 4 INCHES AND A CONSTANT HAUNCH WIDTH OUTSIDE THE EDGE OF EACH GIRDER FLANGE OF 9 INCHES. DEVIATE FROM THIS HAUNCH THICKNESS AS NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE. THE ALLOWABLE TOLERANCE FOR THE HAUNCH WIDTH OUTSIDE THE EDGE OF EACH GIRDER FLANGE IS ± 3 INCHES.
2. THE HAUNCH THICKNESS WAS MEASURED AT THE CENTERLINE OF THE GIRDER, FROM THE SURFACE OF THE DECK TO THE BOTTOM OF THE TOP FLANGE MINUS THE DECK SLAB THICKNESS. THE AREA OF ALL EMBEDDED STEEL PLATES HAS BEEN DEDUCTED FROM THE HAUNCH QUANTITY IN ACCORDANCE WITH 511.24.
3. THE TRANSVERSE DECK REINFORCING BARS SHALL BE FIELD BENT TO CONFORM TO THE ROADWAY CROWN. FIELD BENDING SHALL BE INCLUDED WITH ITEM 898 - QC/QA CONCRETE CLASS QSC2, SUPERSTRUCTURE FOR PAYMENT.
4. FOR PARAPET DETAILS, SEE SHEET 32/41.
5. FOR DECK PLAN SEE SHEET 31/41.
6. FOR SCREED ELEVATIONS, SEE SHEETS 33/41 THRU 35/41.
7. FOR STRIP SEAL EXPANSION JOINT DETAILS, SEE SHEET 36/41.
8. FOR REINFORCING SCHEDULE, SEE SHEET 41/41.



ADDITIONAL REINFORCEMENT OVER PIERS

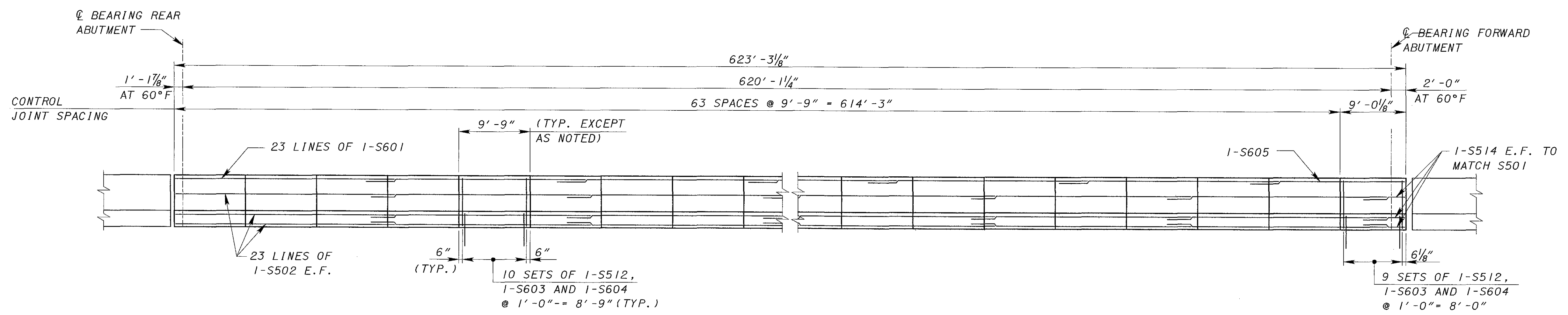
DECK POURING SEQUENCE
 POURING OF THE CONCRETE DECK SHALL PROCEED AS FOLLOWS:
 1. START POUR 1 AT THE REAR ABUTMENT AND PROCEED TO CONSTRUCTION JOINT
 2. START POUR 2 AT THE FORWARD ABUTMENT AND PROCEED TO CONSTRUCTION JOINT

- NOTES:**
- FOR TRANSVERSE SECTION, SEE SHEET [30/41].
 - FOR PARAPET DETAILS, SEE SHEET [32/41].
 - FOR REINFORCING STEEL LIST, SEE SHEET [41/41].



ELEVATION OF LEFT PARAPET

(ALL DIMENSIONS ARE MEASURED ALONG GUTTER LINE)



ELEVATION OF RIGHT PARAPET

(ALL DIMENSIONS ARE MEASURED ALONG GUTTER LINE)

LEGEND

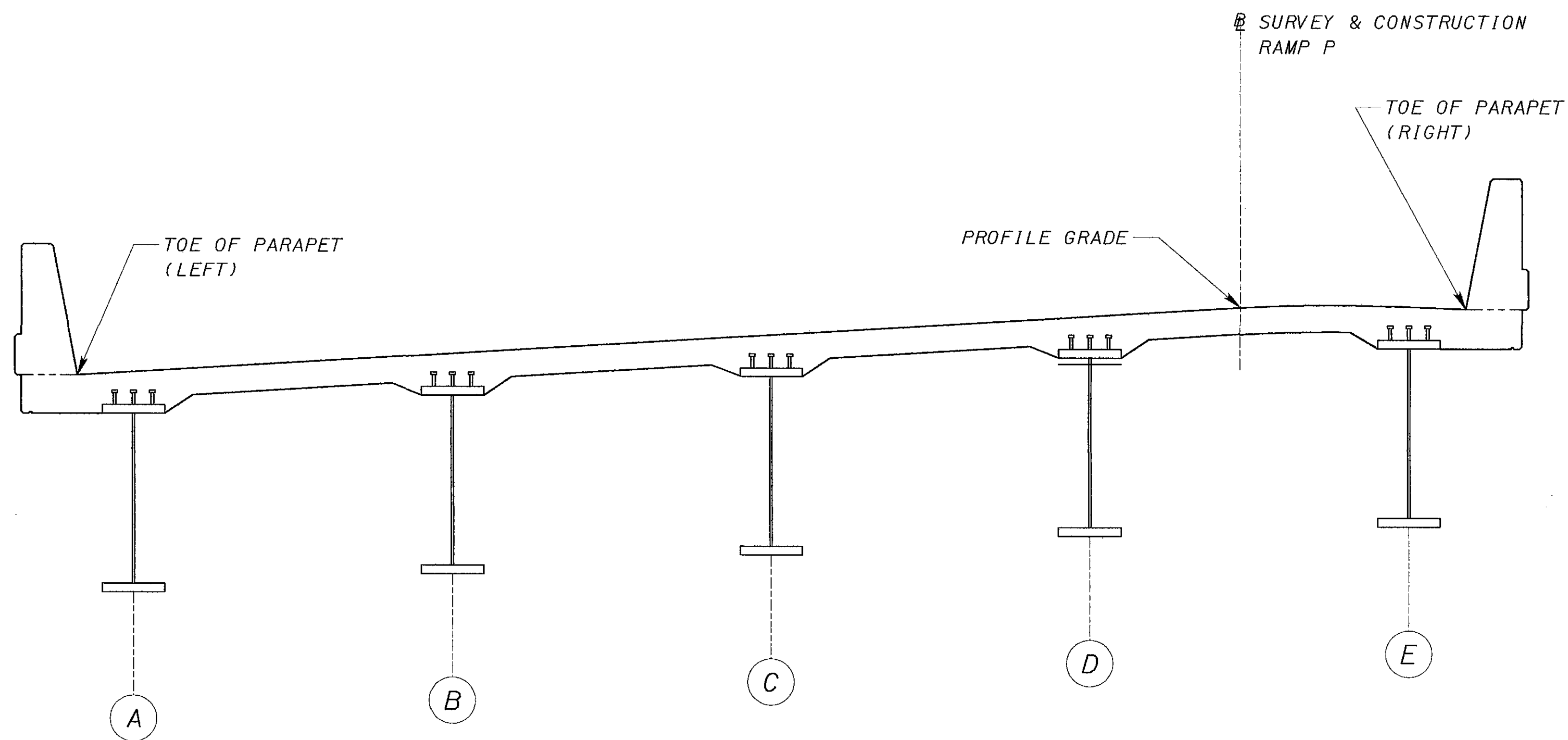
E.F. = EACH FACE
 TYP. = TYPICAL

NOTES:

1. FOR SLAB PLAN, SEE SHEET [31/41].
2. FOR REINFORCING STEEL LIST, SEE SHEET [41/41].
3. FOR ADDITIONAL PARAPET DETAILS AND NOTES, SEE ODOT DRAWING SBR-1-99.

SCREED ELEVATIONS - SPAN 1

LOCATION		CL BRG. REAR ABUT.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	CL PIER 1
TOE OF PARAPET (LEFT)	STATION	1011+91.39	1012+01.77	1012+12.15	1012+22.53	1012+32.91	1012+43.29	1012+53.66	1012+64.04	1012+74.42	1012+84.80	1012+95.18
	FINAL DECK ELEVATION	936.75	937.03	937.30	937.55	937.78	938.00	938.21	938.40	938.58	938.75	938.89
	SCREED ELEVATION	936.75	937.06	937.36	937.62	937.86	938.08	938.28	938.45	938.61	938.76	938.89
GIRDER A	STATION	1011+89.79	1012+00.11	1012+10.43	1012+20.74	1012+31.06	1012+41.38	1012+51.70	1012+62.02	1012+72.33	1012+82.65	1012+92.97
	FINAL DECK ELEVATION	936.80	937.08	937.34	937.60	937.83	938.06	938.26	938.46	938.64	938.80	938.95
	SCREED ELEVATION	936.80	937.11	937.40	937.67	937.91	938.14	938.33	938.51	938.67	938.81	938.95
GIRDER B	STATION	1011+85.37	1011+95.52	1012+05.66	1012+15.81	1012+25.95	1012+36.10	1012+46.25	1012+56.39	1012+66.54	1012+76.68	1012+86.83
	FINAL DECK ELEVATION	937.19	937.47	937.73	937.99	938.23	938.45	938.67	938.86	939.05	939.22	939.38
	SCREED ELEVATION	937.19	937.50	937.79	938.07	938.31	938.53	938.74	938.91	939.08	939.23	939.38
GIRDER C	STATION	1011+81.06	1011+91.04	1012+01.02	1012+11.00	1012+20.98	1012+30.97	1012+40.95	1012+50.93	1012+60.91	1012+70.89	1012+80.87
	FINAL DECK ELEVATION	937.57	937.85	938.12	938.38	938.62	938.85	939.07	939.27	939.46	939.63	939.80
	SCREED ELEVATION	937.57	937.88	938.18	938.46	938.71	938.94	939.14	939.33	939.50	939.65	939.80
GIRDER D	STATION	1011+76.86	1011+86.68	1011+96.51	1012+06.33	1012+16.15	1012+25.98	1012+35.80	1012+45.62	1012+55.44	1012+65.27	1012+75.09
	FINAL DECK ELEVATION	937.96	938.24	938.51	938.77	939.02	939.25	939.47	939.67	939.87	940.05	940.21
	SCREED ELEVATION	937.96	938.27	938.57	938.85	939.11	939.34	939.55	939.73	939.91	940.07	940.21
PROFILE GRADE	STATION	1011+74.93	1011+84.68	1011+94.43	1012+04.18	1012+13.93	1012+23.68	1012+33.43	1012+43.18	1012+52.93	1012+62.68	1012+72.43
	FINAL DECK ELEVATION	938.14	938.43	938.70	938.96	939.20	939.43	939.66	939.86	940.06	940.24	940.41
	SCREED ELEVATION	938.14	938.46	938.76	939.04	939.29	939.52	939.74	939.92	940.10	940.26	940.41
GIRDER E	STATION	1011+72.78	1011+82.45	1011+92.12	1012+01.79	1012+11.46	1012+21.13	1012+30.80	1012+40.47	1012+50.14	1012+59.81	1012+69.48
	FINAL DECK ELEVATION	938.10	938.38	938.65	938.91	939.16	939.40	939.62	939.83	940.02	940.21	940.38
	SCREED ELEVATION	938.10	938.42	938.72	939.00	939.26	939.50	939.70	939.89	940.06	940.23	940.38
TOE OF PARAPET (RIGHT)	STATION	1011+71.37	1011+80.99	1011+90.60	1012+00.22	1012+09.83	1012+19.45	1012+29.07	1012+38.68	1012+48.30	1012+57.91	1012+67.53
	FINAL DECK ELEVATION	938.00	938.28	938.55	938.81	939.06	939.30	939.52	939.73	939.93	940.11	940.29
	SCREED ELEVATION	938.00	938.32	938.62	938.90	939.16	939.40	939.60	939.79	939.97	940.13	940.29



TYPICAL CROSS SECTION

NOTES:

1. SCREED ELEVATIONS SHOWN ARE FOR THE DECK SLAB SURFACE PRIOR TO CONCRETE PLACEMENT. ALLOWANCE HAS BEEN MADE FOR ANTICIPATED CALCULATED DEAD LOAD DEFLECTION.
2. FOR SCREED ELEVATIONS, SPANS 2 THRU 5, SEE SHEETS [34/41] AND [35/41].

SCREED ELEVATIONS - SPAN 2

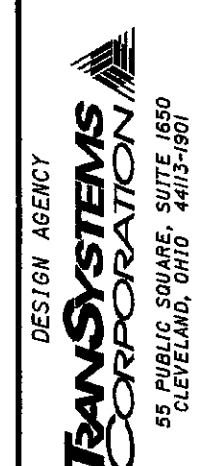
LOCATION		☉ PIER 1	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ PIER 2
TOE OF PARAPET (LEFT)	STATION	1012+95.18	1013+05.70	1013+16.22	1013+26.74	1013+37.26	1013+47.79	1013+58.31	1013+68.83	1013+79.35	1013+89.87	1014+00.39
	FINAL DECK ELEVATION	938.89	939.03	939.15	939.26	939.35	939.43	939.49	939.54	939.57	939.59	939.59
	SCREED ELEVATION	938.89	939.03	939.15	939.26	939.36	939.44	939.50	939.55	939.57	939.59	939.59
GIRDER A	STATION	1012+92.97	1013+03.41	1013+13.86	1013+24.30	1013+34.74	1013+45.19	1013+55.63	1013+66.07	1013+76.51	1013+86.96	1013+97.40
	FINAL DECK ELEVATION	938.95	939.09	939.22	939.33	939.42	939.50	939.56	939.62	939.65	939.67	939.68
	SCREED ELEVATION	938.95	939.09	939.22	939.33	939.43	939.51	939.57	939.63	939.65	939.67	939.68
GIRDER B	STATION	1012+86.83	1012+97.06	1013+07.29	1013+17.53	1013+27.76	1013+37.99	1013+48.22	1013+58.45	1013+68.69	1013+78.92	1013+89.15
	FINAL DECK ELEVATION	939.38	939.52	939.65	939.77	939.87	939.96	940.03	940.09	940.14	940.17	940.19
	SCREED ELEVATION	939.38	939.51	939.65	939.77	939.88	939.97	940.04	940.10	940.14	940.17	940.19
GIRDER C	STATION	1012+80.87	1012+90.90	1013+00.93	1013+10.96	1013+20.99	1013+31.02	1013+41.05	1013+51.08	1013+61.11	1013+71.14	1013+81.17
	FINAL DECK ELEVATION	939.80	939.94	940.08	940.20	940.31	940.41	940.49	940.56	940.61	940.65	940.68
	SCREED ELEVATION	939.80	939.93	940.07	940.20	940.31	940.42	940.50	940.56	940.61	940.65	940.68
GIRDER D	STATION	1012+75.09	1012+84.93	1012+94.77	1013+04.60	1013+14.44	1013+24.28	1013+34.12	1013+43.96	1013+53.79	1013+63.63	1013+73.47
	FINAL DECK ELEVATION	940.21	940.37	940.51	940.64	940.75	940.85	940.94	941.02	941.08	941.13	941.17
	SCREED ELEVATION	940.21	940.36	940.50	940.64	940.75	940.85	940.94	941.03	941.08	941.13	941.17
PROFILE GRADE	STATION	1012+72.43	1012+82.18	1012+91.93	1013+01.68	1013+11.43	1013+21.18	1013+30.93	1013+40.68	1013+50.43	1013+60.18	1013+69.93
	FINAL DECK ELEVATION	940.41	940.57	940.71	940.84	940.96	941.06	941.16	941.24	941.30	941.36	941.40
	SCREED ELEVATION	940.41	940.56	940.70	940.84	940.96	941.06	941.16	941.25	941.30	941.36	941.40
GIRDER E	STATION	1012+69.48	1012+79.13	1012+88.79	1012+98.44	1013+08.09	1013+17.75	1013+27.40	1013+37.05	1013+46.70	1013+56.36	1013+66.01
	FINAL DECK ELEVATION	940.38	940.54	940.68	940.82	940.94	941.05	941.14	941.23	941.30	941.36	941.41
	SCREED ELEVATION	940.38	940.53	940.67	940.81	940.94	941.05	941.14	941.23	941.30	941.36	941.41
TOE OF PARAPET (RIGHT)	STATION	1012+67.53	1012+77.12	1012+86.71	1012+96.30	1013+05.89	1013+15.49	1013+25.08	1013+34.67	1013+44.26	1013+53.85	1013+63.44
	FINAL DECK ELEVATION	940.29	940.45	940.59	940.73	940.85	940.96	941.06	941.15	941.22	941.28	941.33
	SCREED ELEVATION	940.29	940.44	940.58	940.72	940.85	940.96	941.06	941.15	941.22	941.28	941.33

SCREED ELEVATIONS - SPAN 3

LOCATION		☉ PIER 2	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ PIER 3
TOE OF PARAPET (LEFT)	STATION	1014+00.39	1014+14.26	1014+28.13	1014+42.00	1014+55.87	1014+69.75	1014+83.62	1014+97.49	1015+11.36	1015+25.23	1015+39.10
	FINAL DECK ELEVATION	939.59	939.57	939.52	939.45	939.36	939.23	939.09	938.91	938.71	938.49	938.23
	SCREED ELEVATION	939.59	939.58	939.55	939.50	939.41	939.27	939.12	938.92	938.70	938.48	938.23
GIRDER A	STATION	1013+97.40	1014+11.13	1014+24.86	1014+38.59	1014+52.32	1014+66.05	1014+79.77	1014+93.50	1015+07.23	1015+20.96	1015+34.69
	FINAL DECK ELEVATION	939.68	939.67	939.63	939.56	939.47	939.36	939.22	939.05	938.86	938.65	938.41
	SCREED ELEVATION	939.68	939.68	939.66	939.61	939.52	939.40	939.25	939.06	938.85	938.64	938.41
GIRDER B	STATION	1013+89.15	1014+02.49	1014+15.84	1014+29.18	1014+42.52	1014+55.87	1014+69.21	1014+82.55	1014+95.89	1015+09.24	1015+22.58
	FINAL DECK ELEVATION	940.19	940.19	940.17	940.12	940.05	939.96	939.84	939.70	939.53	939.34	939.13
	SCREED ELEVATION	940.19	940.20	940.20	940.16	940.10	940.00	939.87	939.71	939.53	939.33	939.13
GIRDER C	STATION	1013+81.17	1013+94.16	1014+07.14	1014+20.13	1014+33.11	1014+46.10	1014+59.08	1014+72.07	1014+85.05	1014+98.04	1015+11.02
	FINAL DECK ELEVATION	940.68	940.70	940.69	940.66	940.61	940.54	940.44	940.32	940.18	940.01	939.83
	SCREED ELEVATION	940.68	940.71	940.72	940.70	940.66	940.59	940.47	940.33	940.18	940.00	939.83
GIRDER D	STATION	1013+73.47	1013+86.12	1013+98.77	1014+11.42	1014+24.07	1014+36.72	1014+49.37	1014+62.02	1014+74.67	1014+87.32	1014+99.97
	FINAL DECK ELEVATION	941.17	941.20	941.21	941.20	941.16	941.10	941.03	940.93	940.80	940.66	940.50
	SCREED ELEVATION	941.17	941.21	941.24	941.24	941.21	941.15	941.06	940.95	940.80	940.65	940.50
PROFILE GRADE	STATION	1013+69.93	1013+82.43	1013+94.93	1014+07.43	1014+19.93	1014+32.43	1014+44.93	1014+57.43	1014+69.93	1014+82.43	1014+94.93
	FINAL DECK ELEVATION	941.40	941.44	941.45	941.44	941.41	941.37	941.30	941.20	941.09	940.96	940.81
	SCREED ELEVATION	941.40	941.45	941.48	941.48	941.46	941.42	941.33	941.22	941.09	940.95	940.81
GIRDER E	STATION	1013+66.01	1013+78.35	1013+90.68	1014+03.02	1014+15.36	1014+27.70	1014+40.03	1014+52.37	1014+64.71	1014+77.04	1014+89.38
	FINAL DECK ELEVATION	941.41	941.45	941.47	941.47	941.45	941.41	941.35	941.26	941.16	941.04	940.90
	SCREED ELEVATION	941.41	941.46	941.50	941.52	941.50	941.46	941.39	941.28	941.16	941.04	940.90
TOE OF PARAPET (RIGHT)	STATION	1013+63.44	1013+75.67	1013+87.90	1014+00.13	1014+12.36	1014+24.60	1014+36.83	1014+49.06	1014+61.29	1014+73.52	1014+85.75
	FINAL DECK ELEVATION	941.33	941.38	941.40	941.41	941.39	941.36	941.30	941.23	941.13	941.02	940.88
	SCREED ELEVATION	941.33	941.39	941.43	941.46	941.44	941.41	941.34	941.25	941.13	941.02	940.88

NOTES:

- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SPAN 1 SCREED ELEVATIONS, SEE SHEET [33/41].
- FOR SCREED ELEVATIONS, SPANS 4 AND 5, SEE SHEET [35/41].



DESIGNED BY
BTA
CHECKED
NFF

DATE
7/19/05

REVIEWED
RER

STRUCTURE FILE NUMBER
2511452

SCREED ELEVATIONS - SPANS 2 & 3
BRIDGE NO. FRA-270-2583A
RAMP P OVER IR-71

FRA-270-24.47
PID 77320

34/41

802
846

SCREED ELEVATIONS - SPAN 4

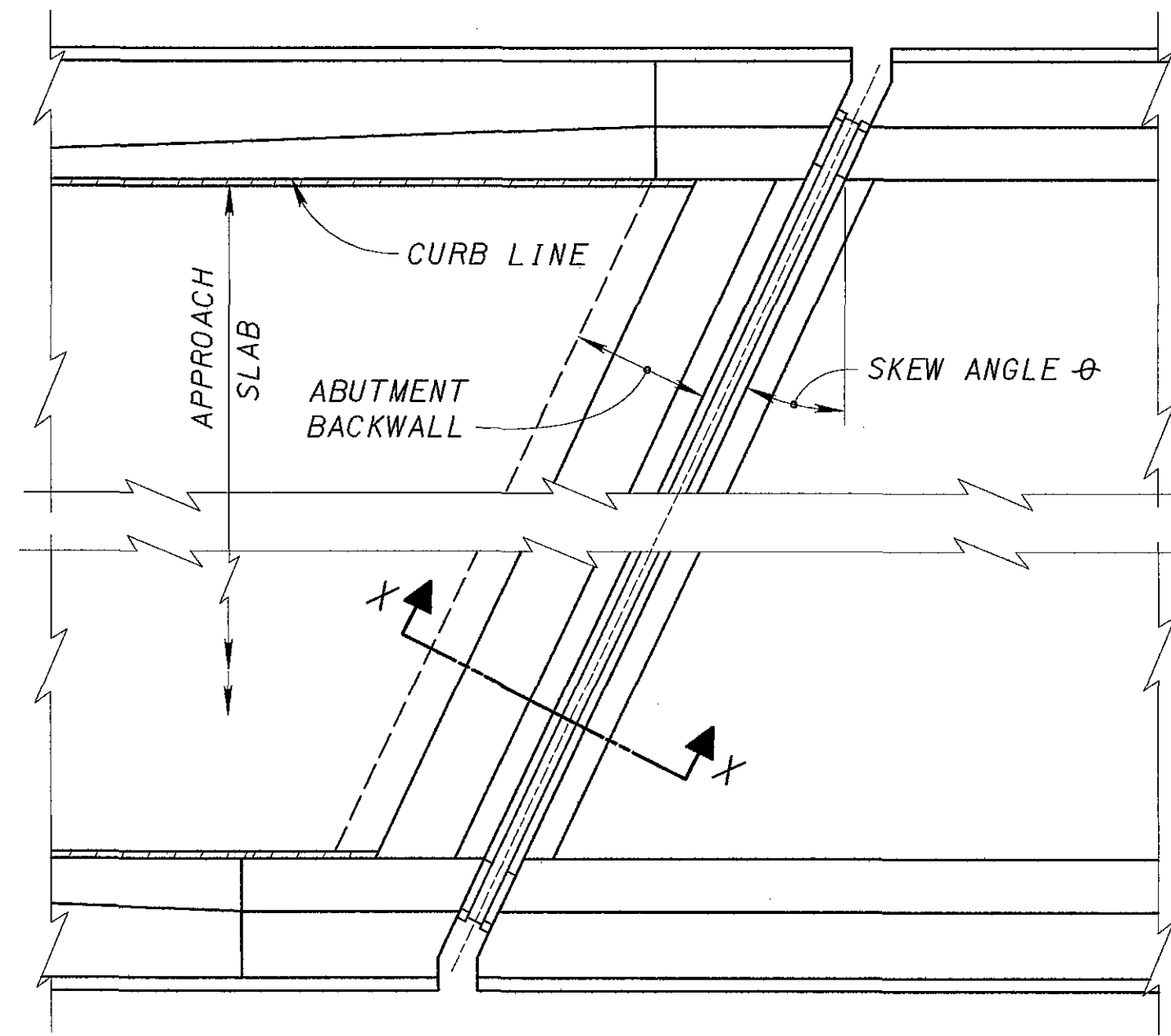
LOCATION		☉ PIER 3	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ PIER 4
TOE OF PARAPET (LEFT)	STATION	1015+39.10	1015+58.81	1015+78.51	1015+98.22	1016+17.92	1016+37.63	1016+57.34	1016+77.04	1016+96.75	1017+16.45	1017+36.16
	FINAL DECK ELEVATION	938.23	937.83	937.38	936.87	936.32	935.70	935.04	934.33	933.56	932.74	931.87
	SCREED ELEVATION	938.23	937.88	937.50	937.06	936.55	935.93	935.22	934.44	933.61	932.74	931.87
GIRDER A	STATION	1015+34.69	1015+53.97	1015+73.24	1015+92.52	1016+11.79	1016+31.07	1016+50.34	1016+69.62	1016+88.89	1017+08.17	1017+27.44
	FINAL DECK ELEVATION	938.41	938.03	937.60	937.11	936.58	936.00	935.37	934.69	933.96	933.18	932.35
	SCREED ELEVATION	938.41	938.08	937.72	937.30	936.81	936.23	935.55	934.80	934.01	933.18	932.35
GIRDER B	STATION	1015+22.58	1015+40.78	1015+58.97	1015+77.17	1015+95.36	1016+13.56	1016+31.76	1016+49.95	1016+68.15	1016+86.34	1017+04.54
	FINAL DECK ELEVATION	939.13	938.80	938.43	938.01	937.55	937.04	936.49	935.90	935.26	934.57	933.84
	SCREED ELEVATION	939.13	938.84	938.53	938.18	937.76	937.26	936.68	936.03	935.33	934.59	933.84
GIRDER C	STATION	1015+11.02	1015+28.30	1015+45.58	1015+62.86	1015+80.14	1015+97.43	1016+14.71	1016+31.99	1016+49.27	1016+66.55	1016+83.83
	FINAL DECK ELEVATION	939.83	939.54	939.22	938.85	938.45	938.00	937.52	937.00	936.43	935.83	935.18
	SCREED ELEVATION	939.83	939.58	939.32	939.01	938.65	938.21	937.71	937.14	936.51	935.86	935.18
GIRDER D	STATION	1014+99.97	1015+16.46	1015+32.95	1015+49.43	1015+65.92	1015+82.41	1015+98.90	1016+15.39	1016+31.87	1016+48.36	1016+64.85
	FINAL DECK ELEVATION	940.50	940.25	939.97	939.65	939.30	938.90	938.47	938.01	937.51	936.97	936.40
	SCREED ELEVATION	940.50	940.28	940.06	939.80	939.50	939.12	938.67	938.16	937.60	937.01	936.40
PROFILE GRADE	STATION	1014+94.93	1015+11.08	1015+27.23	1015+43.38	1015+59.53	1015+75.68	1015+91.83	1016+07.98	1016+24.13	1016+40.28	1016+56.43
	FINAL DECK ELEVATION	940.81	940.58	940.31	940.01	939.68	939.31	938.90	938.46	937.98	937.48	936.93
	SCREED ELEVATION	940.81	940.61	940.40	940.16	939.88	939.53	939.10	938.61	938.08	937.52	936.93
GIRDER E	STATION	1014+89.38	1015+05.17	1015+20.96	1015+36.76	1015+52.55	1015+68.34	1015+84.13	1015+99.92	1016+15.72	1016+31.51	1016+47.30
	FINAL DECK ELEVATION	940.90	940.68	940.44	940.16	939.85	939.50	939.12	938.71	938.26	937.78	937.27
	SCREED ELEVATION	940.90	940.71	940.53	940.32	940.06	939.74	939.34	938.89	938.37	937.83	937.27
TOE OF PARAPET (RIGHT)	STATION	1014+85.75	1015+01.32	1015+16.88	1015+32.45	1015+48.01	1015+63.58	1015+79.15	1015+94.71	1016+10.28	1016+25.84	1016+41.41
	FINAL DECK ELEVATION	940.88	940.68	940.45	940.18	939.88	939.55	939.18	938.79	938.36	937.90	937.40
	SCREED ELEVATION	940.88	940.71	940.54	940.34	940.09	939.79	939.40	938.97	938.47	937.95	937.40

SCREED ELEVATIONS - SPAN 5

LOCATION		☉ PIER 4	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ BRG. FWD. ABUT.
TOE OF PARAPET (LEFT)	STATION	1017+36.16	1017+48.43	1017+60.69	1017+72.96	1017+85.22	1017+97.49	1018+09.76	1018+22.02	1018+34.29	1018+46.55	1018+58.82
	FINAL DECK ELEVATION	931.87	931.30	930.73	930.13	929.53	928.93	928.34	927.74	927.14	926.54	925.95
	SCREED ELEVATION	931.87	931.32	930.78	930.21	929.64	929.06	928.47	927.86	927.23	926.59	925.95
GIRDER A	STATION	1017+27.44	1017+39.94	1017+52.43	1017+64.93	1017+77.42	1017+89.92	1018+02.41	1018+14.91	1018+27.40	1018+39.90	1018+52.39
	FINAL DECK ELEVATION	932.35	931.79	931.20	930.61	930.00	929.39	928.78	928.18	927.57	926.96	926.35
	SCREED ELEVATION	932.35	931.81	931.25	930.69	930.11	929.52	928.91	928.30	927.66	927.01	926.35
GIRDER B	STATION	1017+04.54	1017+17.59	1017+30.65	1017+43.70	1017+56.76	1017+69.81	1017+82.86	1017+95.92	1018+08.97	1018+22.03	1018+35.08
	FINAL DECK ELEVATION	933.84	933.29	932.72	932.13	931.51	930.88	930.25	929.61	928.98	928.34	927.70
	SCREED ELEVATION	933.84	933.30	932.75	932.19	931.59	930.98	930.36	929.71	929.06	928.38	927.70
GIRDER C	STATION	1016+83.83	1016+97.34	1017+10.85	1017+24.36	1017+37.87	1017+51.38	1017+64.88	1017+78.39	1017+91.90	1018+05.41	1018+18.92
	FINAL DECK ELEVATION	935.18	934.65	934.09	933.51	932.91	932.28	931.63	930.97	930.32	929.66	929.00
	SCREED ELEVATION	935.18	934.65	934.11	933.56	932.99	932.38	931.74	931.07	930.40	929.70	929.00
GIRDER D	STATION	1016+64.85	1016+78.74	1016+92.63	1017+06.52	1017+20.41	1017+34.31	1017+48.20	1017+62.09	1017+75.98	1017+89.87	1018+03.76
	FINAL DECK ELEVATION	936.40	935.88	935.35	934.78	934.19	933.58	932.94	932.28	931.60	930.93	930.25
	SCREED ELEVATION	936.40	935.87	935.36	934.82	934.27	933.68	933.06	932.39	931.69	930.98	930.25
PROFILE GRADE	STATION	1016+56.43	1016+70.48	1016+84.53	1016+98.58	1017+12.63	1017+26.68	1017+40.73	1017+54.78	1017+68.83	1017+82.88	1017+96.93
	FINAL DECK ELEVATION	936.93	936.43	935.90	935.35	934.77	934.16	933.52	932.86	932.19	931.51	930.82
	SCREED ELEVATION	936.93	936.42	935.91	935.39	934.85	934.26	933.64	932.97	932.28	931.56	930.82
GIRDER E	STATION	1016+47.30	1016+61.52	1016+75.73	1016+89.95	1017+04.17	1017+18.39	1017+32.60	1017+46.82	1017+61.04	1017+75.25	1017+89.47
	FINAL DECK ELEVATION	937.27	936.78	936.26	935.71	935.14	934.54	933.91	933.26	932.59	931.90	931.20
	SCREED ELEVATION	937.27	936.76	936.25	935.74	935.21	934.65	934.05	933.40	932.71	931.97	931.20
TOE OF PARAPET (RIGHT)	STATION	1016+41.41	1016+55.73	1016+70.05	1016+84.37	1016+98.69	1017+13.01	1017+27.33	1017+41.65	1017+55.97	1017+70.29	1017+84.61
	FINAL DECK ELEVATION	937.40	936.92	936.41	935.87	935.30	934.71	934.09	933.44	932.77	932.08	931.38
	SCREED ELEVATION	937.40	936.90	936.40	935.90	935.37	934.82	934.23	933.58	932.89	932.15	931.38

NOTES:

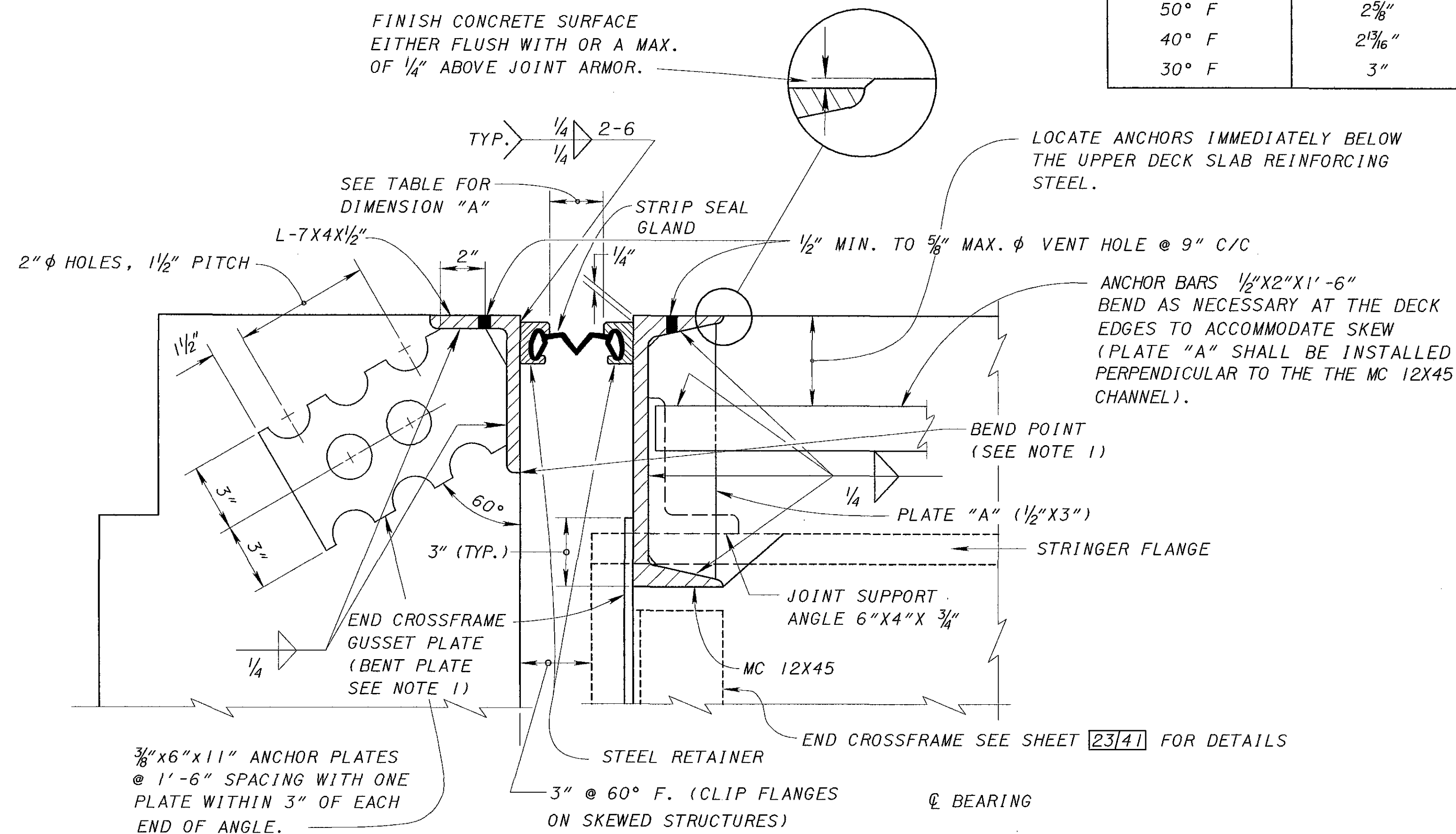
- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SPAN 1 SCREED ELEVATIONS, SEE SHEET [33/41].
- FOR SCREED ELEVATIONS, SPANS 2 AND 3, SEE SHEET [34/41].



PLAN AT ABUTMENT

5" STRIP SEAL JOINT WIDTH	
AMBIENT TEMPERATURE	DIMENSION "A"
90° F	1 7/8"
80° F	2 1/16"
70° F	2 1/4"
60° F	2 1/2"
50° F	2 5/8"
40° F	2 3/16"
30° F	3"

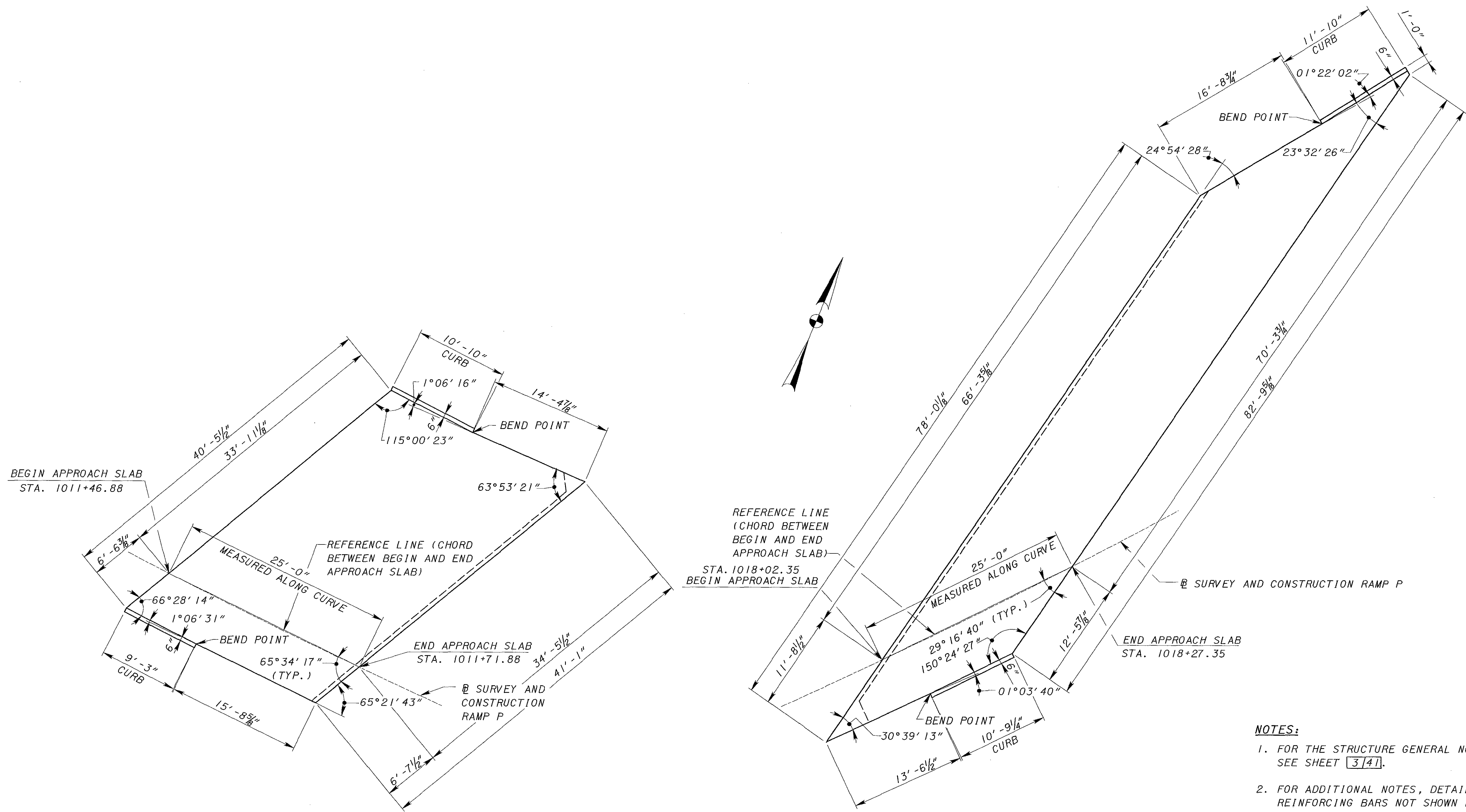
FINISH CONCRETE SURFACE EITHER FLUSH WITH OR A MAX. OF 1/4" ABOVE JOINT ARMOR.



SECTION X-X

NOTES:

1. THE ROADWAY GRADE AT THE ABUTMENTS IS GREATER THAN 2%, THEREFORE THE END CROSSFRAME GUSSET PLATE SHOULD BE BENT TO BE NORMAL TO THE ROADWAY GRADE. IN ADDITION, THE L-7x4x1/2" ANGLE IN THE BACKWALL SHALL BE ROTATED ABOUT THE BEND POINT TO PLACE THE JOINT ARMOR PERPENDICULAR TO THE ROADWAY GRADE.
2. FOR ADDITIONAL DETAILS AND NOTES NOT SHOWN, REFER TO ODOT STANDARD DRAWING EXJ-4-87 SHEETS 1 THRU 5.



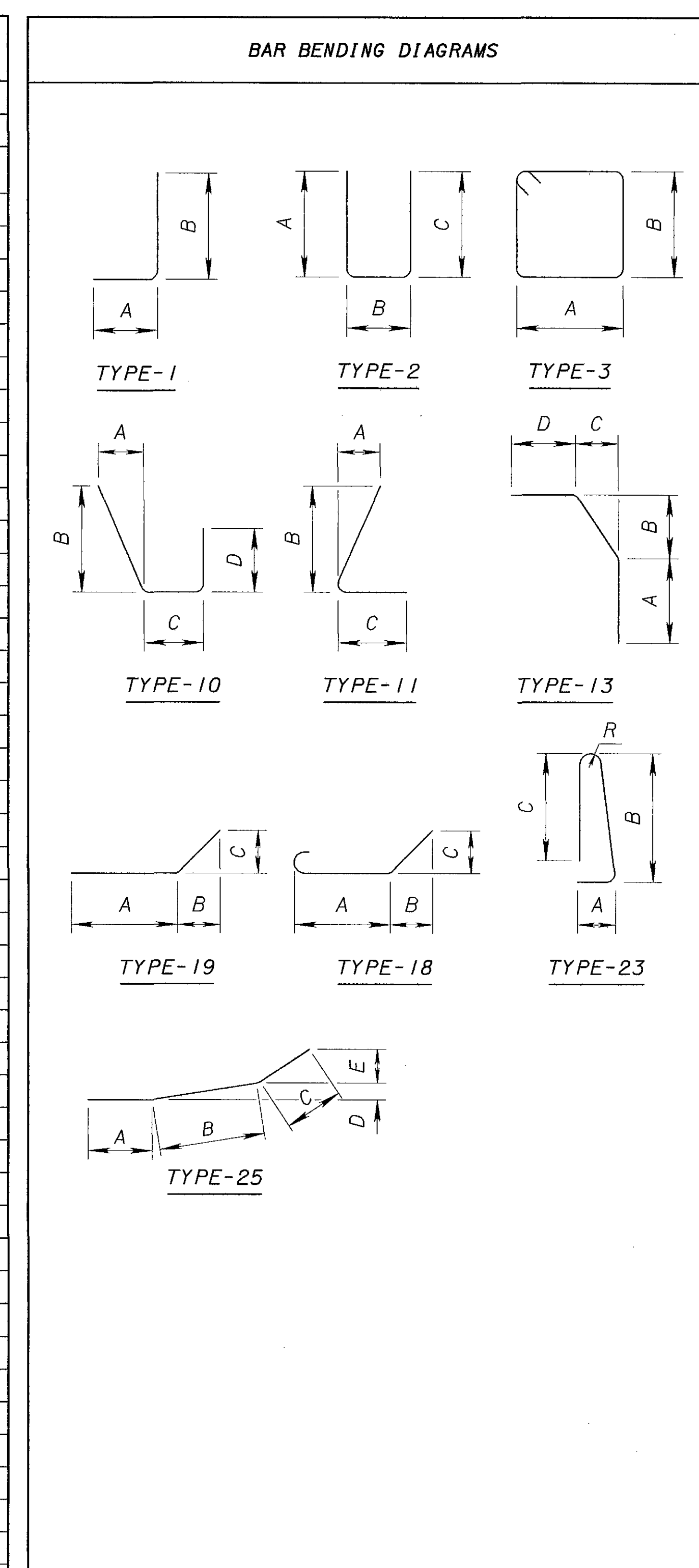
PLAN
REAR ABUTMENT
APPROACH SLAB

PLAN
FORWARD ABUTMENT
APPROACH SLAB

- NOTES:**
1. FOR THE STRUCTURE GENERAL NOTES, SEE SHEET 3747.
 2. FOR ADDITIONAL NOTES, DETAILS, SECTIONS AND REINFORCING BARS NOT SHOWN HERE SEE ODOT STANDARD BRIDGE DRAWING, REINFORCED CONCRETE APPROACH SLABS, AS-1-81, SHEET 1, 2 AND 3.
 3. APPROACH SLAB QUANTITIES TOTAL 216 SQ. YDS.
 4. USE CURB TYPE 4-A, AS SHOWN ON ODOT STANDARD DRAWING BP-5-1.

BAR SCHEDULE												
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS							
					A	B	C	D	E	R	INC.	
FORWARD ABUTMENT												
FA501	45	10'-5"	489	2	2'-0"	6'-8"	2'-0"					
FA502	18	8'-3"	155	1	10"	7'-7"						
FA503	50	8'-2"	426	2	2'-0"	4'-5"	2'-0"					
	1	6'-10"				3'-1"						
FA504	SERIES		52	2	2'-0"		2'-0"					
	OF 6	9'-11"				6'-2"						
	1	5'-9"				2'-0"						
FA505	SERIES		28	2	2'-0"		2'-0"					
	OF 4	7'-7"				3'-10"						
FA506	13	9'-7"	130	1	10"	8'-11"						
FA507	11	10'-11"	125	1	10"	10'-3"						
FA508	12	12'-2"	152	1	10"	11'-6"						
FA509	5	8'-6"	44	1	2'-0"	6'-8"						
FA510	1	6'-2"	6	1	2'-0"	4'-4"						
FA511	8	13'-1"	109	1	10"	12'-5"						
	1	3'-8"				1'-10"						
FA512	SERIES		28	1	2'-0"							
	OF 5	7'-1"				5'-3"						
	1	6'-11"				3'-2"						
FA513	SERIES		34	2	2'-0"		2'-0"					
	OF 4	9'-6"				5'-9"						
FA514	2	6'-11"	14	2	2'-7"	2'-0"	2'-7"					
FA515	22	8'-5"	193	1	2'-7"	6'-0"						
FA516	2	18'-8"	39	STR.								
FA517	10	12'-0"	125	3	3'-2"	2'-7"						
FA518	2	10'-4"	22	STR.								
FA519	38	30'-0"	1189	STR.								
FA520	4	28'-4"	118	STR.								
FA521	4	25'-0"	104	19	23'-8"	8"	1'-1"					
FA522	3	29'-10"	93	STR.								
FA523	5	21'-5"	112	1	2'-6"	19'-0"						
FA524	5	20'-8"	108	1	2'-6"	18'-4"						
FA525	5	20'-0"	104	1	2'-6"	17'-8"						
FA526	1	9'-7"	10	1	2'-3"	7'-6"						
FA527	1	9'-8"	10	1	2'-3"	7'-7"						
FA528	1	7'-5"	8	1	2'-3"	5'-4"						
FA529	1	6'-9"	7	1	2'-3"	4'-8"						
FA530	1	4'-8"	5	1	2'-3"	2'-7"						
FA531	1	13'-0"	14	19	12'-0"	6"	10"					
FA532	1	7'-2"	7	STR.								
FA533	1	26'-0"	27	19	25'-0"	6"	10"					
FA534	1	20'-0"	21	STR.								
FA535	1	9'-4"	10	19	8'-3"	6"	10"					
FA536	1	3'-5"	4	STR.								
FA537	10	30'-5"	317	STR.								
FA538	11	5'-6"	63	STR.								
FA539	2	9'-11"	21	19	7'-11"	2'-0"	4"					
FA540	8	16'-8"	139	STR.								
FA541	8	13'-11"	116	STR.								
FA542	1	10'-2"	11	STR.								
FA543	2	9'-10"	21	19	7'-9"	2'-1	7"					
FA544	1	16'-0"	17	STR.								
FA545	3	17'-11"	56	STR.								
FA546	7	7'-5"	54	23	1'-1"	3'-2"	3'-0"				3"	
FA547	1	5'-3"	5	19	2'-0"	3'-2"	7"					

BAR SCHEDULE												
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS							
					A	B	C	D	E	R	INC.	
FORWARD ABUTMENT												
FA548	6	7'-6"	47	STR.								
FA549	16	10'-0"	167	STR.								
FA550	6	5'-6"	34	25	1'-8"	2'-5"	1'-5"	1 1/2"	5"			
FA551	10	5'-6"	57	STR.								
FA552	6	6'-1"	38	STR.								
FA553	4	15'-3"	64	STR.								
FA554	6	8'-8"	54	STR.								
FA555	8	4'-0"	33	19	2'-0"	1'-10"	10"					
FA556	8	5'-0"	42	10	1'-0"	1'-9"	1'-2"	2'-0"				
FA557	4	3'-10"	16	11	1'-9"	1'-0"	2'-0"					
FA601	12	16'-6"	297	2	2'-7"	6'-8"	7'-7"					
	1	12'-11"				3'-1"						
FA602	SERIES		130	2	2'-7"		7'-7"					
	OF 6	16'-0"				6'-2"						
FA603	13	17'-10"	152	2	2'-7"	6'-8"	8'-11"					
FA604	11	19'-2"	316	2	2'-7"	6'-8"	10'-3"					
FA605	9	20'-5"	276	2	2'-7"	6'-8"	11'-6"					
FA606	5	9'-1"	68	1	2'-7"	6'-8"						
FA607	1	6'-9"	10	1	2'-7"	4'-4"						
	1	8'-0"				3'-2"						
FA608	SERIES		56	2	2'-7"		2'-7"					
	OF 4	10'-7"				5'-9"						
FA609	76	8'-9"	999	2	3'-10"	1'-5"	3'-10"					
FA610	76	9'-7"	1094	2	4'-3"	1'-5"	4'-3"					
FA611	76	6'-1"	694	2	2'-9"	11"	2'-9"					
FA612	2	12'-4"	37	1	1'-0"	11'-6"						
FA613	2	8'-0"	24	STR.								
FA614	7	22'-4"	235	2	10'-9"	1'-2"	10'-9"					
FA615	14	4'-11"	103	STR.								
	1	10'-8"			4'-11"		4'-11"					
FA616	SERIES		80	2		1'-2"						
	OF 6	7'-2"				3'-2"	3'-2"					
FA617	8	16'-8"	200	1	15'-8"	1'-2"						
FA618	16	5'-6"	132	STR.								
	1	11'-10"			5'-6"		5'-6"					
FA619	SERIES		86	2		1'-2"						
	OF 6	7'-2"				3'-2"	3'-2"					
FA620	7	3'-11"	41	13	2'-0"	11"	2"	1'-1"				
FA621	7	2'-11"	31	STR.								
FA622	60	5'-4"	481	STR.								
FA623	1	6'-5"	10	STR.								
FA624	1	7'-10"	12	STR.								
FA801	27	30'-0"	2163	STR.								
FA802	7	14'-4"	268	STR.								
FA803	2	5'-11"	32	19	4'-0"	11 3/4"	1'-7 3/4"					
FA804	4	18'-8"	199	STR.								
FA805	2	16'-8"	89	STR.								
FA806	2	14'-10"	79	STR.								
FA807	4	11'-7"	124	STR.								
FA808	2	23'-4"	125	19	19'-4"	3'-8"	1'-8"					
FA809	25	4'-10"	323	18	2'-7"	1'-0"	1'-0"					
TOTAL WEIGHT =			14,460	LBS								



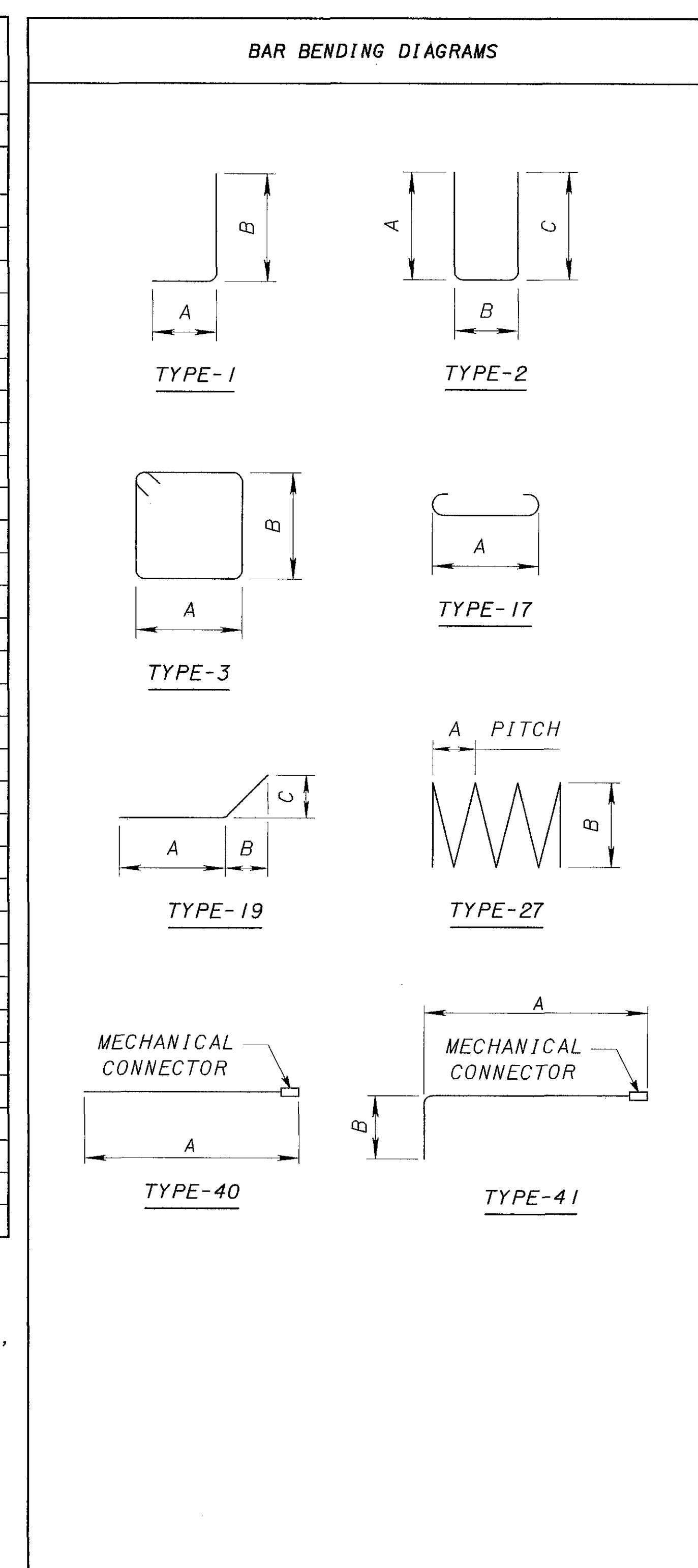
NOTES:
1. FOR NOTES SEE SHEET 40/41.

BAR SCHEDULE											
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS						
					A	B	C	D	E	R	INC.
PIER 1											
SP401	1	27'-3"	470	27	4 1/2"	3'-0"					
SP402	1	26'-6"	457	27	4 1/2"	3'-0"					
SP403	1	25'-10"	432	27	4 1/2"	3'-0"					
P501	64	12'-7"	840	3	2'-0"	4'-0"					
	4 SERIES	10'-7"			2'-0"	3'-0"					
P502	OF	TO	288	3							2 1/8"
	6 BARS	12'-5"			2'-0"	3'-11"					
P503	6	6'-11"	43	2	2'-0"	3'-2"	2'-0"				
P601	2	43'-2"	130	STR.							
P602	6	44'-8"	403	STR.							
P901	12	10'-7"	432	19	6'-5"	4'-1"	1'-0"				
P902	6	36'-2"	738	STR.							
P903	12	28'-10"	1176	41	27'-6"	1'-7"					
P904	12	18'-6"	755	1	1'-7"	17'-2"					
P1001	30	12'-4"	1592	17	9'-6"						
P1002	30	13'-4"	1721	17	10'-6"						
P1101	36	14'-3"	2726	1	2'-0"	12'-7"					
P1102	12	29'-11"	1907	STR.							
P1103	12	29'-3"	1865	STR.							
P1104	12	28'-6"	1817	STR.							
TOTAL WEIGHT PIER 1 = 17792 POUNDS											
PIER 2											
SP401	1	22'-10"	367	27	4 1/2"	3'-0"					
SP402	1	22'-0"	382	27	4 1/2"	3'-0"					
SP403	1	21'-1"	396	27	4 1/2"	3'-0"					
P501	88	12'-7"	1155	3	2'-0"	4'-0"					
	4 SERIES	10'-7"			2'-0"	3'-0"					
P502	OF	TO	336	3							1 3/4"
	7 BARS	12'-5"			2'-0"	3'-11"					
P503	6	6'-11"	43	2	2'-0"	3'-2"	2'-0"				
P601	2	47'-7"	143	STR.							
P602	6	49'-8"	448	STR.							
P1001	30	11'-4"	1463	17	8'-6"						
P1002	30	13'-4"	1721	17	10'-6"						
P1003	12	13'-4"	688	19	8'-2"	5'-1"	1'-0"				
P1004	6	39'-6"	1020	STR.							
P1005	12	32'-1"	1657	41	30'-7"	1'-10"					
P1006	12	20'-7"	1063	1	1'-10"	19'-1"					
P1101	27	14'-3"	2044	1	2'-0"	12'-7"					
P1102	9	25'-6"	1219	STR.							
P1103	9	24'-8"	1180	STR.							
P1104	9	23'-9"	1136	STR.							
TOTAL WEIGHT PIER 2 = 16461 POUNDS											

BAR SCHEDULE											
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS						
					A	B	C	D	E	R	INC.
PIER 3											
P401	48	8'-0"	257	STR.							
P402	40	10'-6"	281	STR.							
SP403	1	19'-11"	306	27	4 1/2"	3'-0"					
SP404	1	19'-1"	320	27	4 1/2"	3'-0"					
SP405	1	18'-3"	334	27	4 1/2"	3'-0"					
SP406	1	17'-5"	348	27	4 1/2"	3'-0"					
P501	102	12'-7"	1338	3	2'-0"	4'-0"					
	4 SERIES	10'-7"			2'-0"	3'-0"					
P502	OF	TO	480	3							1 1/4"
	10 BARS	12'-5"			2'-0"	3'-11"					
P503	6	6'-11"	43	2	2'-0"	3'-2"	2'-0"				
P601	4	30'-8"	184	STR.							
P602	12	31'-5"	566	STR.							
P901	14	36'-1"	1717	41	34'-9"	1'-7"					
P902	14	26'-3"	1250	1	1'-7"	24'-11"					
P1001	40	10'-10"	1864	17	8'-0"						
P1002	40	13'-4"	2294	17	10'-6"						
P1003	44	12'-3"	2319	1	1'-10"	10'-9"					
P1004	11	22'-7"	1069	STR.							
P1005	11	21'-10"	1033	STR.							
P1006	11	20'-11"	990	STR.							
P1007	11	20'-1"	951	STR.							
P1008	14	16'-3"	979	19	8'-2"	4'-1"	1'-0"				
P1009	21	21'-3"	1920	STR.							
TOTAL WEIGHT PIER 3 = 20843 POUNDS											

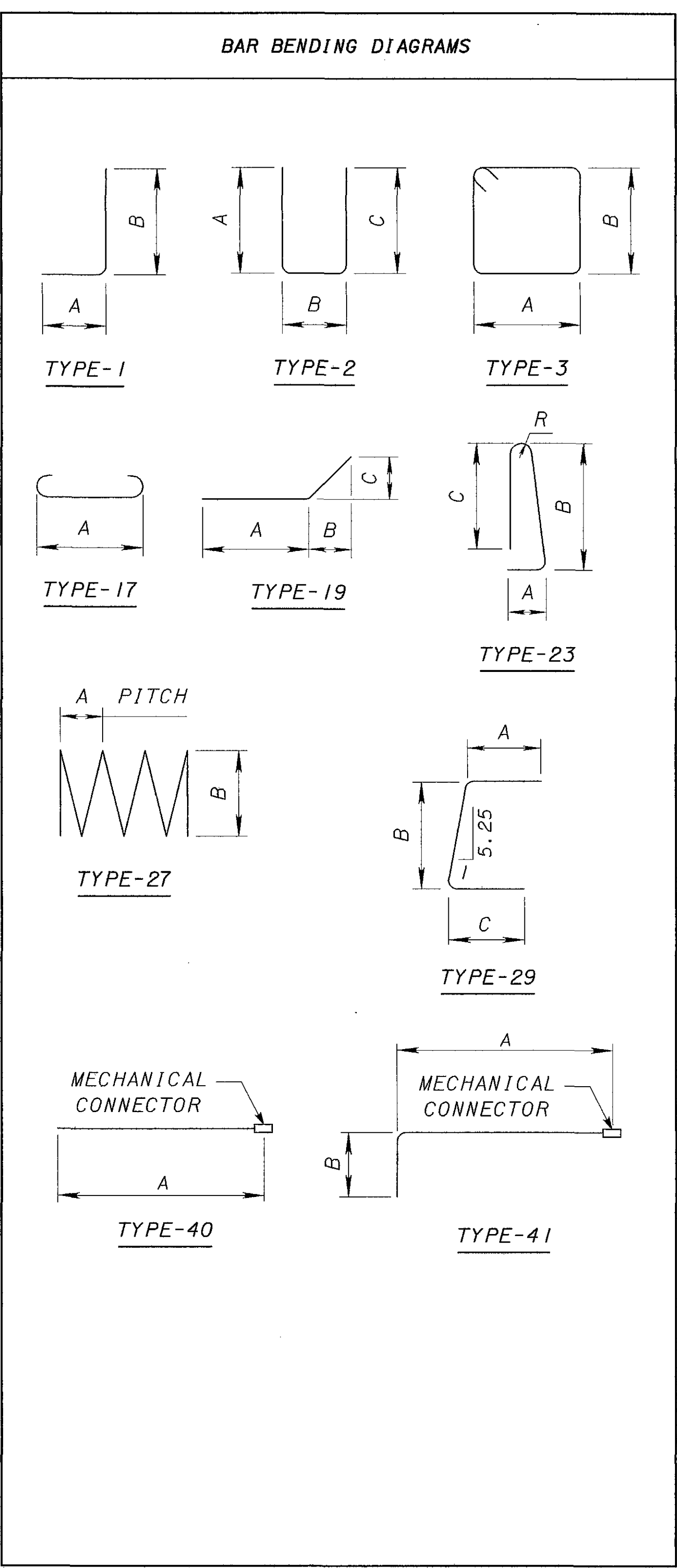
NOTES:

- THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN. THE FIRST DIGIT WHERE THREE DIGITS ARE USED, AND THE FIRST TWO DIGITS WHERE FOUR ARE USED, INDICATES THE BAR SIZE NUMBER. FOR EXAMPLE, RA601:
P: LOCATION OF THE BAR IN THE STRUCTURE (PIER)
6: BAR SIZE DESIGNATION NO. 6
01: SEQUENCE NUMBER
- BAR DIMENSIONS SHOWN ARE OUT-TO-OUT UNLESS OTHERWISE NOTED.
- ALL REINFORCING STEEL IS TO BE EPOXY COATED. STRAIGHT BARS ARE INDICATED BY "STR".
- CONCRETE SPACERS OR OTHER APPROVED NONCORROSIVE SPACING DEVICES SHALL BE USED AT SUFFICIENT INTERVALS (NEAR THE BOTTOM AND AT INTERVALS NOT EXCEEDING 10.0') TO INSURE CONCENTRIC SPACING FOR THE ENTIRE CAGE LENGTH. SPACERS SHALL BE CONSTRUCTED OF AN APPROVED MATERIAL EQUAL IN QUALITY AND DURABILITY TO THE CONCRETE SPECIFIED FOR THE COLUMN. THE SPACERS SHALL HAVE ADEQUATE DIMENSIONS TO ENSURE A MINIMUM 3" CLEAR SPACE BETWEEN THE OUTSIDE OF THE REINFORCING CAGE AND THE DESIGN DIMENSION OF THE COLUMN. CYLINDRICAL CONCRETE FEET (BOTTOM SUPPORTS) SHALL BE PROVIDED TO ENSURE THAT THE BOTTOM OF THE CAGE IS MAINTAINED AT THE PROPER DISTANCE ABOVE THE BASE.
- SPIRAL REINFORCING BARS: THE "LENGTH" SHOWN IN THE STEEL LIST FOR THE SPIRAL BARS IS THE LENGTH OF THE SPIRAL ALONG THE AXIS OF THE SPIRAL. ONE AND ONE-HALF CLOSED-COIL TURNS SHALL BE PROVIDED AT THE ENDS OF EACH SPIRAL UNIT.



BAR SCHEDULE											
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS						
					A	B	C	D	E	R	INC.
PIER 4											
SP401	1	18'-0"	316	27	4 1/2"	3'-0"					
SP402	1	16'-10"	297	27	4 1/2"	3'-0"					
SP403	1	15'-7"	276	27	4 1/2"	3'-0"					
SP404	1	14'-4"	256	27	4 1/2"	3'-0"					
SP405	1	13'-2"	236	27	4 1/2"	3'-0"					
P501	156	12'-7"	2047	3	2'-0"	4'-0"					
	4 SERIES	10'-7"			2'-0"	3'-0"					
P502	OF TO	336	3								1 3/4"
	7 BARS	12'-5"			2'-0"	3'-11"					
P503	6	6'-11"	43	2	2'-0"	3'-2"	2'-0"				
P701	6	31'-0"	368	STR.							
P702	18	31'-8"	1165	STR.							
P901	14	36'-6"	1737	41	35'-2"	1'-7"					
P902	14	37'-10"	1801	40	37'-10"						
P903	14	17'-6"	833	1	1'-7"	16'-2"					
P1001	100	12'-4"	5306	17	9'-6"						
P1002	55	12'-3"	2899	1	1'-10"	10'-9"					
P1003	11	20'-8"	978	STR.							
P1004	11	19'-6"	923	STR.							
P1005	11	18'-3"	864	STR.							
P1006	11	17'-2"	813	STR.							
P1007	11	15'-9"	746	STR.							
P1008	28	22'-8"	2731	STR.							
P1009	14	13'-2"	793	19	8'-2"	5'-0"	1'-0"				
TOTAL WEIGHT PIER 4 = 25764 POUNDS											

BAR SCHEDULE											
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS						
					A	B	C	D	E	R	INC.
SUPERSTRUCTURE											
S401	1078	30'-0"	21603	STR.							
	1 SERIES	22'-9"									
S402	OF TO	996	STR.								
	49 BARS	38'-1"									
S501	1448	39'-8"	59907	STR.							
S502	1852	30'-0"	57949	STR.							
	1 SERIES	5'-8"									
S503	OF TO	773	STR.								
	52 BARS	22'-10"									
S504	OF TO	1001	STR.								
	45 BARS	39'-6"									
	1 SERIES	3'-0"									
S505	OF TO	3437	STR.								
	156 BARS	39'-3"									
S506	2	44'-2"	92	STR.							
S507	4	43'-0"	180	STR.							
S508	24	8'-0"	200	STR.							
S509	96	19'-4"	1936	STR.							
S510	(NOT USED)										
S511	48	13'-4"	668	STR.							
S512	1318	7'-5"	10196	23	1'-1"	3'-2"	3'-0"				2 3/4"
S513	6	22'-3"	139	STR.							
S514	6	5'-9"	36	STR.							
S601	47	30'-0"	2118	STR.							
S602	1	11'-9"	18	STR.							
S603	1318	3'-11"	7754	29	1'-1"	1'-11"	1'-1"				
S604	1318	2'-10"	5609	1	1'-1"	1'-11"					
S605	1	21'-5"	32	STR.							
TOTAL WEIGHT SUPERSTRUCTURE = 174644 LBS											



NOTES:
 1. FOR NOTES SEE SHEET 40/41.

REINFORCING STEEL LIST

BRIDGE NO. FRA-270-2583A
 RAMP P OVER IR-71

DESIGNED BY: J.D.H. CHECKED BY: N.F.F.
 DRAWN BY: C.A.G. REVISED BY:

REVIEWED BY: DATE: 7/19/05
 STRUCTURE FILE NUMBER: 2511452

DESIGN AGENCY: **TRANS SYSTEMS CORPORATION**
 5000 W. 10TH AVE., DENVER, CO 80202

FRA-270-24.47
 PID 77320

41/41

809
846