Belmont 40 Arch Bridge Superstructure Demolition QC



ETB: I will apply the construction dead load to the cap beams as element beam loads and the construction live load as pressure loads to the deck elements. There are 5 different cap beam load magnitudes for each construction live load and dead load based upon tributary spacing of cap beams. Grouped in the model using the groups shown below. They are:

- Group A Caps, 15.125' say 11.34k/42.5' = 0.267klf for construction dead load
- Group B Caps, 14' say 10.5k/42.5' = 0.247klf for construction dead load
- Group C Caps, 14.125' say 10.59k/42.5' = 0.249klf for construction dead load
- Group D Caps, 15.75' say 11.81k/42.5' = 0.278klf for construction dead load
- Group E Caps, 15.625' say 11.72k/42.5' = 0.275klf for construction dead load

-Models created for various stages of construction, with both columns and arches investigated.

-Assume excavator weight is 81k + slab beam pick (20'x4'x1.5' = 18k) and has an axle spacing of 13.25'. Total axle load is 49.5k. Wheel loader weight is 57kips + 18k (slab beam) for a total axle load of 37.5k and spaced 11.25' apart. Keep a minimum distance of 20' between excavator and wheel loader. -Assume the excavator/loader is primarily confined to the CL of the bridge. The excavator will pick each slab beam and travel to the loader, which will travel longitudinally along the bridge with each pick to place it off the bridge. No other heavy construction vehicle will be on the bridge with the excavator and wheel loader. -Braking force, BR, will be 10% of the weight of the excavator + wheel loader: 0.1*(49.5k*2+37.5k*2)/42.5' = 0.41klf applied to the cap elements. Assume this load is distributed to only one cap for conservatism. All stages of construction were analyzed for column loads without braking force, and then the two cases with the highest D/C ratios were re-run with the braking force included.

-Previous demo iterations have shown that the arch is sensitive to unbalanced load conditions. Therefore the outer three slab beams, along with the parapets and sidewalk, will be removed in their entirety before any other interior slab beams are removed. The outer slab beams will be removed evenly. See below for the arch code check's superstructure removal. These correspond to the sketches on the next page. Hatched regions are those where the slab beams and any overlay/sidewalk/deck/parapet above the slab beam has also been removed. Red hatch indicates portions of superstructure removed during current stage. Blue hatch indicates portions of superstructure previously removed. -For Arches A and B, the demolition steps have been furthest discretized. Since Arches C and D bridge decks will be deconstructed most similarly to Arch B, only a couple Arch C and D stages will be modeled to save time. These stages will be chosen based upon those governing Arch B stages.



Information Provided By DBW

Information Provided By DBW

APPLY CONSTRUCTION DEAD LOAD (CDL) OF 15 psf x 50' WIDE = 750PLF TO ACCOUNT FOR PROTECTIVE DECKING / DEBRIS CONTAINMENT.

APPLY CONSTRUCTION LIVE LOAD (CR) OF CAT 336 EXCAVATOR: 81,000 lb (ON 2 TRACKS APPROX. 13' LONG) AND JOHN DEERE 744 WHEEL LOADER 57,000LB (ON TWO AXLES APPROX. 11'-3"). SPACE THE EXCAVATOR AND WHEEL LOADER AXLES 20' APART.



Transverse Section Showing Bridge Demolition **Initial Sequence and Excavator Placement**

TU)	IGNORE FOR INITIAL PROOF OF CONCEPT CHECKS
) + 1.0×(TU) + 1.0×(C₩) J) <mark>U)_</mark> _	
7) CRCE EFFECT DURING CONSTRUCTION ("INAC" ("ACTIVE" WIND OF 20 MPH) DIRECTION	TIVE" WIND



Calculated: ETB 3/5/25 Checked: DBW 3/15/25

APPLY CONSTRUCTION DEAD LOAD (CDL) OF 50psf x 19' WIDE TO ACCOUNT FOR CRANE MATS ON CENTER 5 SLAB BEAMS

APPLY CONSTRUCTION LIVE LOAD (CLL) OF 25 psf x 50' WIDE = 1,250 PLF TO ACCOUNT FOR PERSONNEL AND MISC. EQUIPMENT.

TRANSVERSE DECK SECTION THRU ARCH SPANS

336 Hydraulic Excavator Specifications



Stick Options		Reac	Mass Stick			
	R3.9DB ((12'10'')	R3.2D B (10'6")		M2.55TB (8'4")	
1 Machine Height:						
Cab height	3180 mm	10'5"	3180 mm	10'5"	3180 mm	10'5"
OPG height	3330 mm	10'11"	3330 mm	10'11"	3330 mm	10'11"
Guardrails/Handrails Height	3180 mm	10'5"	3180 mm	10'5"	3180 mm	10'5"
With Boom/Stick/Bucket Installed	3660 mm	12'0"	3480 mm	11'5"	3610 mm	11'10"
With Boom/Stick Installed	3560 mm	11'8"	3330 mm	10'11"	3410 mm	11'2"
With Boom Installed	2880 mm	9'5"	2880 mm	9'5"	2830 mm	9'3"
With Boom/Stick/Bucket Installed (with auxiliary lines)	3670 mm	12'0"	3530 mm	11'7"	3620 mm	11'11"
With Boom/Stick Installed (with auxiliary lines)	3620 mm	11'11"	3410 mm	11'2"	3420 mm	11'3"
With Boom Installed (with auxiliary lines)	2970 mm	9'9"	2970 mm	9'9"	2900 mm	9'6"
2 Machine Length:						
With Boom/Stick/Bucket Installed	11 180 mm	36'8"	11 160 mm	36'7"	10 870 mm	35'8"
With Boom/Stick Installed	11 170 mm	36'8"	11 120 mm	36'6"	10 830 mm	35'6"
With Boom Installed	9960 mm	32'8"	9960 mm	32'8"	9640 mm	31'8"
With Boom/Stick/Bucket Installed (with auxiliary lines)	11 180 mm	36'8"	11 160 mm	36'7"	10 870 mm	35'8"
With Boom/Stick Installed (with auxiliary lines)	11 170 mm	36'8"	11 120 mm	36'6"	10 830 mm	35'6"
With Boom Installed (with auxiliary lines)	10 010 mm	32'10"	10 010 mm	32'10"	9640 mm	31'8"
3 Upperframe Width without Walkways	2970 mm	9'9"	2970 mm	9'9"	2970 mm	9'9"
4 Tail Swing Radius	3530 mm	11'7"	3530 mm	11'7"	3530 mm	11'7"
5 Counterweight Clearance	1260 mm	4'2"	1260 mm	4'2"	1260 mm	4'2"
6 Ground Clearance	510 mm	1'8"	510 mm	1'8"	510 mm	1'8"
7 Track Length – Length to Center of Rollers	4040 mm	13'3"	4040 mm	13'3"	4040 mm	13'3"
8 Track Length	5030 mm	16'6"	5030 mm	16'6"	5030 mm	166"
9 Track Gauge – Extended	2740 mm	9'0"	2740 mm	9'0"	2740 mm	9'0"
10 Track Width/Undercarriage Width (with steps):						
700 mm (28") Shoes	3440 mm	11'3"	3440 mm	11'3"	3440 mm	11'3"
800 mm (31") Shoes	3540 mm	11'7"	3540 mm	11'7"	3540 mm	11'7"
850 mm (33") Shoes	3590 mm	11'9"	3590 mm	11'9"	3590 mm	11'9"
Bucket Type	H	D	H	D	SD	W
Bucket Capacity	2.00 m ³	2.61 yd3	2.00 m ³	2.61 yd3	2.41 m ³	3.15 yd3
Bucket Tip Radius	1790 mm	5'9"	1790 mm	5'9"	1910 mm	6.3 ft

336 Hydraulic Excavator Specifications

Engine	
Engine Model	Cat [®] C7.
Net Power	
ISO 9249	223.5 kV
ISO 9249 (DIN)	304 hp (1
Engine Power	
ISO 14396	225 kW
ISO 14396 (DIN)	306 hp (i
Bore	105 mm
Stroke	135 mm
Displacement	7.01 L
 Meets U.S. EPA Tier 4 Final, emission standards. Recommended for use up to 4 power derate above 3000 m (9). Advertised power is tested per at the time of manufacture. Net power is the power available equipped with fan, air intakes swith engine speed at 2,000 rpm. All Cat nonroad U.S. EPA Tie Korea Stage V, India CEV Sta diesel engines are required to twith 15 ppm of sulfur or less) lower-carbon intensity fuels** ✓ 20% biodiesel FAME (fa ✓ 100% renewable diesel, F GTL (gas-to-liquid) fuel Refer to guidelines for success Cat dealer or "Caterpillar Ma (SEBU6250) for details. 	EU Stage V, and 500 m (14,760 ft) a ,840 ft). r the specified star ole at the flywheel system, exhaust system, ex 4 Final, EU Stag ge V, and China N use ULSD (ultra-k or ULSD blended or ULSD blended or ULSD blended star tty acid methyl es IVO (hydrotreated s sful application. P achine Fluids Reco
**Tailpipe greenhouse gas emis are essentially the same as t	ssions from lower-c raditional fuels.
Swing Mechanism	
Swing Speed	8.84 rpn
Maximum Swing Torque	143 kN-
Weights	
Operating Weight	36 800 k
 Long Wide Undercarriage, R HD 2.12 m³ (2.77 yd³) Bucket, 3 6.8 mt (15,000 lb) Counterwe 	each Boom, R3.9 850 mm (33") Tripl ight.

Track

ptional Track Shoes Width	850 mm
ptional Track Shoes Width	800 mm
ptional Track Shoes Width	700 mm
lumber of Shoes (each side)	49
umber of Track Rollers (each side)	8
lumber of Carrier Rollers (each side)	2

ASSUME EXCAVATOR IS CENTERED IN 14' SPAN AND REACHES TO ADJACENT 14' SPAN PICK RADIUS = 7' + 7' = 14' (SAY 15')

ASSUME EXCAVATOR IS CENTERED IN 14' SPAN AND REACHES TO ADJACENT 20'-3" SPAN PICK RADIUS = 7' + 10' - 1.5'' = 17' - 1.5'' (SAY 20')

LIFT CAPACITY IS GREATER THAN 20,000 lb.

		Drive		
Cat® C7.1 TT	ГА	Maximum Gradeability	35°/70%	
		Maximum Travel Speed	4.7 km/h	2.9 mph
23.5 kW	300 hp	Maximum Drawbar Pull	302.5 kN	68,005 lbf
04 hp (metr	ic)	Hudsenlis Costem		
		Hydraulic System		
25 kW	302 hp	Main System - Maximum Flow	560 L/min	148 gal/min
06 hp (metr	ic)	(Implement)	(280 ×	(74 ×
05 mm	4 in		2 pumps)	2 pumps)
35 mm	5 in	Maximum Pressure – Equipment –	35 000 kPa	5,076 psi
.01 L	428 in ³	Maximum Pressure – Equipment –	38.000 kPa	5.511 nsi
V, and Japa	n 2014	Lift Mode	50 000 RI U	2,211 [231
760 ft) altitu	de with engine	Maximum Pressure - Travel	35 000 kPa	5,076 psi
/00 11/ 11/14	de finiti engine	Maximum Pressure - Swing	29 400 kPa	4,264 psi
ied standard	d in effect	Boom Cylinder – Bore	150 mm	6 in
		Boom Cylinder - Stroke	1440 mm	57 in
wheel when	and alternator	Stick Cylinder - Bore	170 mm	7 in
aust system	, and anerhator	Stick Cylinder - Stroke	1738 mm	68 in
EU Stage V,	Japan 2014,	DB Bucket Cylinder - Bore	150 mm	6 in
China Nonro	oad Stage IV	DB Bucket Cylinder - Stroke	1151 mm	45 in
(ultra-low su	alfur diesel	TB Bucket Cylinder - Bore	160 mm	6 in
biended with	i the following	TB Bucket Cylinder - Stroke	1356 mm	53 in
ethyl ester)* otreated veg	etable oil) and	Service Refill Capacities		
d Die		Fuel Tank Capacity	600 L	158.5 gal
ds Recomm	endations"	Cooling System	39 L	10.2 gal
	Circuitorio	Engine Oil (with filter)	25 L	6.6 gal
can use hig	her blends,	Swing Drive	18 L	4.8 gal
L	- Interneting Confe	Final Drive (each)	8 L	2.1 gal
tower-carbo fuels	n intensity fueis	Hydraulic System (including tank)	373 L	98.5 gal
		Hydraulic Tank (including suction pipe)	161 L	42.5 gal
		Diesel Exhaust Fluid (DEF) Tank	50 L	13.2 gal
.84 rpm		Standards		
43 kN-m	105,250 lbf-ft	Brakes	ISO 10265:2	008
		Cab/Operator Protective	ISO 10262:1	998 Level II
		Guards (OPG) (optional)		
6 800 kg	81,100 lb	Cab/Rollover Protective	ISO 12117-2	:2008
n, R3.9DB (12'10") Stick,	Structure (ROPS)		
") Inple Gr	ouser Shoes,	Sound Performance		
		ISO 6395:2008 (external)	105 dB(A)	
		ISO 6396:2008 (inside cab)	72 dB(A)	
50 mm	33 in	Hearing protection may be needed when	en operating	with an open
00 mm	31 in	operator station and cab (when not pr	operly maints	ained or door
00 mm	28 in	windows open) for extended periods of	or in a noisy e	nvironment.
9		Also Caralitization Contact		
		Air Conditioning System		

The air conditioning system on this machine contains the fluorinated greenhouse gas refrigerant R134a (Global Warming Potential = 1430). The system contains 1.00 kg of refrigerant, which has a CO2 equivalent of 1.430 metric tonnes.

REPRESENTATIVE TRUCK DEFINED IN MIDAS FOR EXCAVATOR PLUS WHEEL LOADER



Dimensions With Forks A Height to Top of Cab B Hood Height C Ground Clearance D Length From Centerline to Front Axle E Wheelbase F Overall Length, Forks on Ground G Height to Hinge Pin, Fully Raised H Reach, Fully Raised I Fork Height, Fully Raised J Maximum Reach, Fork Level K Fork Height, Maximum Reach L Reach, Ground Level M Depth Below Ground N Tine Length O Load Position, 50% Tine Length Specifications With Forks Tipping Load, Straight, No Tire Deflection EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Tipping Load, Straight, With Tire Deflection EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Tipping Load, 37-deg. Partial Turn, No Tire Deflection EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Tipping Load, 40-deg. Full Turn, No Tire Deflection EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Tipping Load, 40-deg. Full Turn, With Tire Deflection EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Rated Operating Load, 50% Full-Turn Tipping Load, With Tire Deflection (conforms to ISO 14397-1)* EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Rated Operating Load, Rough Terrain, 60% Full-Turn Tipping Load, With Tire Deflection (conforms to EN474-3)* EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Rated Operating Load, Firm and Level Ground, 80% Full-T Tipping Load, With Tire Deflection (conforms to EN474-3 EPA FT4/EU Stage V EPA Tier 3/EU Stage IIIA Operating Weight EPA FT4/EU Stage V

EPA Tier 3/EU Stage IIIA Loader operating information is based on machine with identified linkage and standard equipment, JD9 PSS 6090 (EPA FT4/EU Stage V) and JD9 6090H (EPA Tier 3/EU Stage IIIA) engines, ROPS cab, rear cast bumper/counterweight, transmission side-frame guards, bottom guards, standard tires, full fuel tank, and 79-kg (175 lb.) operator. This information is affected by changes in tires, ballast, and different attachments.

*Rated operating capacity based on Deere attachments only.



	3.2 m (10'6") 6.5 m (21'4") ← 700 mm (28") Triple Grouser Track Shoes								4040	mm (13'3'')				
											Æ			
	п	3.ZUB	_ i			E								
1						2	740 mm (9'0")					5030	mm (16°6°')	
5-1		3000 mr	n (10'0")	4500 mr	(500 mm (15'0") 6000 mm (20'0") 7500 mm (25'0")		7500 mm (25'0") 9000 mm (n (30'0")					
	<u>.</u>	R.	đ	R.	d I	Ð	d II	Ę.	Ċ,	Ð	d T	Ð	đ	mm ft/in
7500 mm	kg							*8750	7800			*7350	*7350	7700
25'0'								10000	7750			*16,300	*16,300	24'11"
5000 mm 20'0"	Kg Ib							*8900 *19,450	16,650			*/150 *15,800	13,700	8580 27'11"
4 <u>500 mm</u>	kg			*13 500	*13 500	*10 900	10 600	*9550	7500	8200	5600	*7200	5500	9130
15'0"	lb					*23,550	22,800	*20,800	16,150			*15,850	12,100	29'10"
3000 mm	kg			*17 150	15 250	*12 600	10 000	*10 450	7250	8050	5500	*7500	5100	9410
10.0.	Ib			*36,800	32,900	*27,250	21,600	*22,650	15,550	17,250	11,800	*16,500	11,250	30'10'
1500 mm	kg			*19 /50	14 300	*14 100	9500	10 350	6950	/900	5350	/350	5000	9440
0	10 ka			*20,000	30,800	-30,500	20,900	10,150	14,990	7000	E2E0	7500	TU,990	30 11
0'0"	Kg Ib			*44 600	29 900	20 / 150	9200 19 750	21 000	14 550	16 750	5250 11 200	16 550	11 150	9220 20'2''
-1500 mm	ka	*14 050	*14 050	*20 200	13 800	14 000	9050	10 000	6650	10,730	עענקוו	8100	5450	8750
-5'0"	lb	*31,800	*31,800	*43,800	29,700	30,100	19,450	21,550	14,350			17,800	11,950	28'7"
-3000 mm	kg	*22 200	*22 200	*18 750	13 950	14 050	9050	10 050	6700			9300	6200	7960
-10'0"	IĎ	*50,200	*50,200	*40,600	29,950	30,200	19,550	21,700	14,450			20,550	13,750	25'11"
-4500 mm	kg	*21 100	*21 100	*15 900	14 250	*12 050	9300					*10 200	7950	6750
-15'0"	lb	*45,450	*45,450	*34,150	30,650	*25,650	20,050					*22,400	17,800	21'10"
	* I ISO 10567:2007													

* Indicates that the load is limited by hydraulic lifting capacity rather than tipping load. The above loads are in compliance with hydraulic excavator lift capacity standard ISO 10567:2007. They do not exceed 87% of hydraulic lifting capacity or 75% of tipping load. Weight of all lifting accessories must be deducted from the above lifting capacities. Lifting capacities are based on the machine standing on a firm, uniform supporting surface. The use of a work tool attachment point to handle/lift objects, could affect the machine lift performance.

Lift capacity stays with ±5% for all available track shoes.

Always refer to the appropriate Operation and Maintenance Manual for specific product information.

While general information, pictures, and descriptions are provided, some illustrations and text may include product options and accessories NOT AVAILABLE in all regions, and in some countries products and accessories may require modifications or additions to ensure compliance with the local regulations of those countries.

Dimensions and Specifications With Coupler and Hook-On Construction Forks

336 Hydraulic Excavator Specifications

Reach Boom Lift Capacities – Counterweight: 6.8 mt (15,000 lb) – without Bucket, Heavy Lift: On

744 P-TIER



744 P-TIER STANDARD-LIFT AND HIGH-LIFT LOADERS WITH COUPLER AND HOOK-ON CONSTRUCTION FORKS

	514.5× 004	
EPA F 14/EU Stoge V / EPA Tier 3/E	U Stage IIIA	11 1 1 1 N
Standard-Lift	Standard-Lift	High-Lift
1.83-m (/2 in.) tine length	2.44-m (96 in.) tine length	1.83-m (/2 in.) tine length
3.5/ m (II ft. 9 in.)	3.5/ m (11 ft. 9 in.)	3.5/ m (II ft. 9 in.)
2./3 m (8 ft. 11 in.)	2./3 m (8 ft. 11 in.)	2./3 m (8 ft. 11 in.)
458 mm (18 in.)	458 mm (18 in.)	458 mm (18 in.)
1./0 m (5 ft. / in.)	L/U m (5ft. / in.)	L/U m (5 ft. / in.)
3.46 m (11 ft. 4 in.)	3.46 m (11 ft. 4 in.)	3.46 m (1) ft. 4 in.)
9.82 m (32 ft. 3 in.)	10.43 m (34 ft. 3 in.)	10.49 m (34 ft. 5 in.)
4.27 m (14 ft. 0 in.)	4.27 m (14 ft. 0 in.)	4.84 m (15 ft. 11 in.)
0.90 m (35 in.)	0.90 m (35 in.)	1.05 m (3 ft. 5 in.)
3.22 m (10 ft. 7 in.)	3.22 m (10 ft. 7 in.)	379 m (12 ft. 5 in.)
l.77 m (5 ft. 10 in.)	177 m (5 ft. 10 in.)	2.29 m (7 ft. 6 in.)
l.29 m (4 ft. 3 in.)	1.29 m (4 ft. 3 in.)	1.29 m (4 ft. 3 in.)
1.00 m (3 ft. 3 in.)	1.00 m (3 ft. 3 in.)	1.67 m (5 ft. 6 in.)
56 mm (2.2 in.)	56 mm (2.2 in.)	59 mm (2.3 in.)
1.83 m (72 in.)	2.44 m (96 in.)	1.83 m (72 in.)
0.92 m (36 in.)	1.22 m (48 in.)	0.92 m (36 in.)
13 078 kg (28,832 lb.)	11 859 kg (26,144 lb.)	10 941 kg (24,120 lb.)
12 712 kg (28,025 lb.)	11 521 kg (25,399 lb.)	10 621 kg (23,415 lb.)
-	-	-
12 666 kg (27.923 lb.)	11 S02 kg (25.357 lb.)	10 611 kg (23.393 lb.)
12 318 kg (27,157 lb.)	11 178 kg (24.643 lb.)	10 308 kg (22,725 lb.)
11 526 kg (25,410 lb.)	10 428 kg (22,989 lb.)	9586 kg (21.133 lb.)
11 206 kg (24 705 lb.)	10 134 kg (22 342 lb.)	9307 kg (20,518 lb.)
11 275 kg (24.857 lb.)	10 197 kg (22,480 lb.)	9367 kg (20.650 lb.)
10 963 kg (24 169 lb.)	9909 kg (21846 lb.)	9094 kg (20.049 lb.)
10 710 kg (23 611 lb.)	9702 kg (21389 lb.)	8907 kg (19 636 lb)
10.425 kg (22,001 lb.)	9/38 kg (20 807 lb)	8661 kg (13,030 kL)
10 425 kg (22,505 lb.)	5456 kg (20,607 lb.)	0001 kg (15,054 lb.)
5355 kg (1) 906 lb)	6951 kg (10,695 lb)	4454 kg (9 819 lb)
5222 kg (11,000 ld.) 5212 kg (11,000 ld.)	4001 kg (10,000 l0.) 6719 kg (10,604 l6.)	4434 kg (3,013 lb.)
5215 kg (11,455 lb.)	4713 kg (10,404 lb.)	4331 kg (3,346 kb.)
6476 kg (14167 lb.)	5921 kg (12 922 lb 1	536.6 kg (11.791 lb.)
6355 kg (13700 lb.)	5663 kg (12,033 l0.)	5107 kg (1) /67 (k.)
-	5005 Ng (12,405 IU.)	sishing (investing)
um *		
8568 kg (18,889 lb.)	7762 kg (17.112 lb.)	7126 kp (15.710 lb.)
8340 kg (18.387 lb.)	7550 kg (16.645 lb.)	6929 kg (15,276 lb.)
	the second se	and the set
25 022 kg (55.)64 lb)	25 194 kg (55 543 lb)	25 562 kg (56 354 lb)
24 713 kg (54 483 lb)	24.885 kg (54.862 lb)	25 253 kg (55 673 lb)
name and distribution and	the new region, out that	a. a. a. a. reg (a. a. a. (a.)

PROPOSED LOCATIONS OF TEMPORARY SHORING TOWERS UNDER FLOORBEAMS

159.5



say 165k is factored load that each temporary shoring support needs to handle



BEL 40 Iteration 3 Demo Analysis Arch Model First Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Second Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Third Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Fourth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Fifth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Sixth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Seventh Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Eighth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Ninth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Tenth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Eleventh Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Twelfth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Thirteenth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Fourteenth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Fifteenth Stage.mcb



BEL 40 Iteration 3 Demo Analysis Arch Model Sixteenth Stage.mcb

Tables Works Group Report	Ba	Base The second s				
🗐 Works						
🖃 😭 Analysis Control Data	🚰 MIDA	AS/Text Editor - [BEL 40 Iteration 3 Demo Analysis Arch Model Fourth Stage.lcp]				
	Ella Edita Viena Wiedena Hala					
Construction Stage Analysis [Stage=Last]	File	Edit View Window Help				
Structures	🗅 😅 🛛	🖬 曇 🐧 🖽 羔 ங 💼 झ 🛤 🛱 ♀ ♀ झ 🥠 洙 洙 泳 🔊 ∞• A 🕂 🔁 伊 🗣 🖽 🚍 💡				
Nodes : 6570	00001					
Elements : 6548	00002	++				
Properties	00003	MIDAS(Modeling, Integrated Design & Analysis Software)				
E Section : 367	00004	MIDAS/Civil - Load Combinations				
⊕ I Section Stiffness Scale Factor	00005	(C) SINCE 1989				
	00007	I MIDES Information Technology Co. Ltd. (MIDES IT)				
Boundaries	80000	MIDAS/Civil Version 9.4.0				
Encode Supports : 32 Previously Defined Loads	00009	+========++++++++++++++++++++++++++++++				
Elastic Link : 1536 whose assigments change	00010					
Plate End Release : 1575 based upon stage of	00011 -					
Static Loads demolition	00012 1	DESIGN TYPE : General				
	00013 -					
I Static Load Case 2 [Deflector Type Parapet ; Deflector Type Parap	00015]	LIST OF LOAD COMBINATIONS				
EII Static Load Case 3 [Sidewalk Type Farapet, Sidewalk Type Farap	00016 =					
Static Load Case 5 [Deck + Box Beam : Deck + Box Beam]	00017	NUM NAME ACTIVE TYPE				
Pressure Loads : 2350	00018	LOADCASE (FACTOR) + LOADCASE (FACTOR) + LOADCASE (FACTOR)				
⊡ 📊 Static Load Case 6 [Overlay ; Overlay]	00019 =					
Pressure Loads : 1765	00021	I DC_C ADDIVE AND Self Weint(1 000) + Deck + Box Beam(1 000) + Overlav(1 000)				
□ I Static Load Case 7 [Construction Dead Load ; Construction Dead Lo.	00022 -					
Element Beam Loads : 528	00023 2	2 DW_C Active Add				
Static Load Case 8 [Construction Live Load ; Construction Live Load]	00024	Construction Dead Lo(1.000) + Construction Live Lo(1.000) + Braking Force(1.000)				
Pressure Loads : 2350	00025 -					
I Static Load Case 9 [Braking Force ; Braking Force] I Static Load Case 9 [Braking Force ; Braking Force]	00026	3 FactoredDLDeflection Active Add				
Element Beam Loads : 16	00028	+ Construction Dead Lo (1.500)				
Moving Load Analysis New Static Load	00029 -					
Traffic Surface Lanes : 2						
Traffic Surface Lane 1 [Center A] New Lane		New results Combinations				
Traffic Surface Lane 2 [Center B] Definition						
🚊 🖙 😓 Vehicles : 7						
···· ↔ Vehicles 2 [HL-93TDM ; Standard] Definition						
Vehicles 3 [HL-93TRK ; Standard]						
Wendes 5 [505 , User Defined]						
Concest of SUT : User Defined 1						
B Moving Load Cases : 2						
Moving Load Case 1 [Arch A Excavator] New Moving		8				
Moving Load Case 2 [Arch B Excavator] Load Cases						
	Ready					









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- (EBridge 21-3" 14'-0" -Beam symmetrical about & except 10-61/2" for scupper openings 1-6" -Fill voids with Class S concrete; high-early-strength; Payment included in superstructure precast transverse beams.(typ.)** **€**C - E Beam <<u>C</u> **_** ** The contractor shall verify that the void dimensions shown are adequate to accept existing column reinforcing steel before the beams are constructed. PLAN-BEAM A Hooked Stirrup Hooked Stirrup 4-8801 Corner @ bot tom Corner@top Slope | Level 7-9" 3/16" per ft. 1'-6" 2-8601 00 2-B602 2-B603 -Level 4 206 33 206 4" 206" 40 812"=2-10" 4 spa@ / 4" = 5'-4" 3@ 1-1"= 3'-3" 4-B1102 B503 *B504* B505 17:0" * Dimensions shown at beam Q SECTION A-A (Along Beam ()

remain

NOTES(con't) SUPPORT locations during storage and transportation shall be under the lifting inserts. Ship, store upright BEAMS shall be precast CONCRETE, Class S



42'-6" N MICROFILMED · 7-3" JAN 22 1986 4-91/2" 241" 1-5" 1-5" τω, N (0)**≜**E - & Exist. Arch Rib and Column Lifting insert location _____ (See Sheet [5] 72] for note)

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1 _ Bridge 21-3" -Beam symmetrical 14-0" about q 10-01/2" 1-6" -Fill voids with class S concrete; high-early-strength (Typ.)** See Sheet [15/72] for note. *c***↑** B - 2 Beam B ¢↓ f EPayment for concrete to fill voids shall be included in superstructure precast transverse beams. PLAN-BEAM B Hooked stirrup Hooked stirrup Corner @ bottom Corner @top <u>4-8801</u> Slope | Level 7-9" 3/16" per ft. 1'-6" <u>2-B601</u> Δ D. D 0.4 <u>2-B602</u> Level 2-B603 5 spa. @ 1-4" 4 spa.@6" 2@8" 2@11" /-2" 4-B1103 B505 *B503* B504 17-0" * Dimensions shown at beam Q

SECTION A-A (Along Beam (2)

remain





(Each Col. Face)

Cut exist. #8 at top of Col.

Exist. # 8's to remain



SECTION B-B



	Pier#1 Room	Span"R"= 45-6"		-E Pier #2		ARCH SP	AN 'A" = 145-6"	•
	. 23'-3"	E Diaphi	ragm	16-3"	14'-0"	14'-0"	14-0"	14:0"
			B1	<u>E Beam Span</u> BID	B23	B34	B45	B56
JAN 221980	 		 B2				B1C	B.57
							D46	20,
			<u>B3</u>	B14	B25	B36	B4?	B 5 8
			<u>B4</u>	B15	B26	B37	B48	B59
- & Bridge			<u>B5</u>	BIG	B27	B38	B49	BGC
.9.			BG		B28	B39	<i>B50</i>	BG/
			B7	BIB	B29	B40	B51	B62
0			BB	B19	B30	B41	B52	B63
+85.0			B9	B20	B31	B42	B53	B 64
a. 106			B10		B32	B43	B54	BG
+S			B1/		B33	B44	B55	BGG
	Expansion Joint	L		1-0° Q Tie Rods (typ.); All 12" Box Beams				
	ADCH SDA	NIA"- IAFI E"	& Pier #3	La Friday Loint				
IVIATE TI IINE	ARCA SPAN	NA = 143 - 6	20:3"	- E EXPANSION UDINF 14-0"	ARCH SPAN "E	<u>3"=132-6"</u> 14-0"	14'-0"	14'-0"
AF	14'-0"	1443" 1443"	20-3"	-4-0" 	ARCH SPAN "E 14'-0"	3 ¹ /32 ² 6" /4 ² 0"	14'-0"	14'-0"
A F	14'-0" BIOO	14-3" BIII	20'3" B	122 B133	ARCH SPAN "E 14'-0" BI44	3"≟/32'6" 4'0" BI55	14'-0" B166	14'-0" B17
A	14'-0" BIOO BIOI	14-3" BIII BIII BII2	20'3" B B	122 B133 123 B134	ARCH SPAN "E 14'-0" B144 B145	3"=132-6" 14-0" B155 B156	14'-0" B166 B167	14'-0" B17 B17
	14'-0" BIOO BIOI BIO2	14-3" 4-3" BIII BII2 BII3	20'3" B B B	122 B133 123 B134 124 B135	ARCH SPAN "E 14'-0" B144 B145 B146	3"=132-6" 14-0" B155 B156 B157	14'-0" B166 B167 B168	14'-0" B17 B17 B17
	14'-0" BIOO BIOI BIO2 BIO3	14-3" BIII BII2 BII3 BII4	20'3" B B B B	122 B133 123 B134 124 B135 125 B136	ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147	3"=132-6" 14-0" B155 B156 B157 B158	14'-0" B166 B167 B168 B169	14'-0" B17 B17 B17 B18
	14'-0" BIOO BIOI BIO2 BIO2 BIO3 BIO4	14-3" BIII BII2 BII2 BII3 BII4 BII5	20'3" B B B B B B	Image: Participant opinit 122 123 123 124 125 126	ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147 B148	3"= /32-6" /4-0" B155 B156 B157 B158 B159	14'-0" B166 B167 B168 B169 B170	14'-0" B17 B17 B17 B18 B18
E Bridgez	14'-0" BIOO BIOI BIO2 BIO2 BIO3 BIO4 BIO5	IVA = 143 - 6 14'-3" BIII BII2 BII2 BII3 BII4 BII5 BII6	20'3" B B B B B B B B	122 14'-0" 122 8133 123 8134 124 8135 125 8136 126 8137 127 8138	ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147 B148 B149	3"= /32-6" /4-0" B155 B156 B157 B158 B159 B160	14'-0" B166 B167 B168 B169 B170 B171	14'-0" B17 B17 B17 B18 B18 B18
E Bridgez	14'-0" BIOD BIOJ BIOZ BIOZ BIO3 BIO3 BIO5 BIO5	1/4-3" BIII BIIZ	20'3" B B B B B B B B B B B		ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147 B148 B149 B149 B150	3"= /32'-6" 14'-0" B155 B156 B157 B158 B159 B160 B161	14'-0" B166 B167 B168 B169 B170 B171 B172	14'-0" B17 B17 B17 B18 B18 B18
E Bridgez	14'-0" BIOO BIOI BIO2 BIO2 BIO3 BIO3 BIO4 BIO5 BIO5 BIO5	IVA = 143 - 6 14'-3" BIII BII2 BII2 BII3 BII4 BII5 BII6 BII7 BII8	20'3" B B B B B B B B B B B B B B B B B B B	Image: Participan contract I22 Image: Participan contract I22 B133 I23 B134 I23 B134 I24 B135 I25 B136 I26 B137 I27 B138 I28 B139 I29 B140	ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147 B148 B149 B149 B150 B151	3"= /32-6" /4-0" B155 B156 B157 B158 B159 B160 B161 B162	14'-0" B166 B167 B168 B169 B170 B171 B172 B173	14'-0'' B17 B17 B17 B18 B18 B18 B18 B18
E Bridgez	Id'-O" BIOD BIOJ BIOZ BIOZ BIO3 BIO3 BIO4 BIO5 BIO5 BIO5 BIO5	14-3" BIII BIII BII2 BII3 BII4 BII5 BII6 BII7 BI18 BI19	20'-3" B B B B B B B B B B B B B B B B B B B	Image: Participant contract Image: Participant contract I22 B133 I22 B133 I23 B134 I23 B134 I24 B135 I25 B136 I26 B137 I27 B138 I28 B139 I29 B140 I30 B141	ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147 B148 B148 B149 B149 B150 B151 B152	3"±132±6" 14±0" B155 B156 B157 B158 B159 B160 B161 B162 B163	14'-0" B166 B167 B168 B169 B170 B171 B172 B172 B173	14'-0'' B17 B17 B17 B18 B18 B18 B18 B18
E Bridgez	IACH SPAN IA'-O" BIOO BIOO BIOI BIO2 BIO3 BIO3 BIO4 BIO5 BIO7 BIO8 BIO9	14-3" BIII BII2 BII2 BII3 BII4 BII5 BII6 BII7 BI18 BI19 BI20	20-3" B B B B B B B B B B B B B B B B B B B	Image: Participar Contract I22 Image: Participar Contract I22 B133 I23 B133 I23 B134 I23 B134 I24 B135 I25 B136 I25 B136 I26 B137 I27 B138 I28 B139 I29 B140 I30 B141 I31 B142	ARCH SPAN "E 14'-0" B144 B144 B145 B146 B147 B148 B148 B149 B149 B150 B151 B152 B152 B153	3"± /32 ¹ 6" /4 ¹ 0" B155 B156 B157 B158 B159 B160 B160 B162 B163 B164	14'-0" B166 B167 B168 B169 B170 B170 B171 B172 B173 B174 B175	14'-0'' B17 B17 B17 B18 B18 B18 B18 B18 B18
E Bridgez	IACH SPA IA'-O" BIOO BIOO BIOI BIO2 BIO3 BIO3 BIO4 BIO5 BIO5 BIO6 BIO7 BIO8 BIO9 BIO9	1/4-3" BIII BIIZ BIIZ BIIZ BIIS BIIA BIIA	20'3" B B B B B B B B B B B B B B B B B B B	Image: Part of the second state in	ARCH SPAN "E 14'-0" B144 B145 B145 B146 B147 B148 B148 B148 B149 B150 B151 B152 B153 B154	3"± /32 ¹ 6" /4 ² 0" B155 B156 B157 B158 B159 B160 B160 B162 B163 B164 B165	14'-0" B166 B167 B168 B169 B170 B170 B171 B172 B173 B174 B175 B176	14'-0" BIT BIT BIT BIT BIT BIT BIT BIT BIT BIT

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PRESTRESSED CONCRETE BEAM PLAN

	BE	L-40-23.38	FHWA	STATE	PROJECT	
	14'-0"	14'-0"	5	оню		
				1		
6	B 6 7	878		/2	B89 ·	
7	BGB	B79			B90	
3	B69	880			B91	().
	B 70	B81			B92	
	B 71	B82			B93	Щ
	B 72				B94	
2	B73	B84			B95	
3	B 74	B85		•	B96	
7	B 75	B86			B97	
5	B 76	B87		an na an a	B98	
6	B77	B88	·		B99	SMatch line A
		ansverse beams<		.		
	14:0"		- Singer			
7	B188	4 4				
78	B189			•		
79	B190		•		•	1 1
30	B191					
81	B192		•		NOTES:	
82	B/93	Fo	or Ba	ox Bea	m detail.	s see sheet
83	B194	F	or su	uperst	ructure a	letails see
34	B195	 [4]	heets 16 72	5 <u>22/72</u> +hru[50/72	3/72 and
85	B196	F	or Tre heet[ansver 4 72	se Beam	plan see
86	B197				STATE OF OHIO	18 72
87	B198	Match line B; see sheet			DE ALA	
Ba	NOTES ox beams shall ome identification	(conit) have the on marks	Br	UX E RIDGE ER TH AND V	NO. BEL E B.¢O. VHEELIN	- 40-2338 RAILROAD G CREEK
0n +h	the shop drawin	ngs as on	I. A. M.	urawn tr J. <i>A.M.</i>	ACED CHECKED R.L.D. N	111 12-1-80

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 \sim Q PIER #4 ARCH SPAN"B"= 132-6" MICROFILMED 14-0" 14-3" 20'-3" JAN 221986 Match line B; See sheet • B199 B210 BZZI 18 72 B200 BZII B222 B201 B212 B223 B213 B202 B224 CE Bridge ! B203 B214 B225 B204 B215 B226 ie. マ B205 B216 B227 B206 B217 B228 B207 B218 B229 BZOB B219 B230 B209 B220 B231 -----E Expansion Joint 3-0" and Transverse E Pier #5 beam-ARCH 20'-3" 14'-0" 14'-0" Match B309 B320 line C-B331 B310 B321 B332 · B3/1 B322 B333 B312 B323 B334 B3/3 B324 B335 re Bridge B314 B325 B336 B315 B326 B337 B316 B327 B338 B317 *B*328 B339 B318 B329 B340 ·B319 *B330* B341 3-0"

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A STREET STREET

PRESTRESSED CONCRETE BEAM PLAN

	ARCH	H SPAN "C" = 118'-6	1 **				FHWA REGION STATE PROJECT
14'-0"	14'-0"	14'-0"		- 14'-0"	14-0"	14'-3"	5 OHIO 74
B232	. B243	B254	B265	B276	B287	B298	$\frac{1}{1}$
B233	B244	B255	B266	B277	B288	B299	CE Pier # 5
B234	B245	B256	B267	B278	B289	B300	
B235	B246	B257	B268	B279	B290	B30/	
B236	B247	B258	B269	B280	B291	B302	Ті
B237	B248	B259	B2 70	B281	B292	B303	
B238	B249	B260	B271	B282	B293	B304	
B239	B250	B261	B272	B283	B294	B305	
B240	B251	B262	B273	B284	B295	B306	- 4 ING
B241	B252	B263	B274	B285	B296	<i>B307</i>	-Mati
B242	B253	B264	B275	B286	B297	<i>B308</i>	
LI . CDAN "D" = 10346		1, the	& Pier #G	E Precast	Trans. Beam(typ.)	Joint & Trans. Be	n 3:0"
14'-0"	14-0"	14'-0"	15-3"	1-0"			
B342	B353	B364	B375	Match line		* *	
B343	B 354	B365	B376	20/72			
B344	B355	B366	B377				
B345	B356	B367	B378				
B346	B357	B368	B379				
<u>B347</u>	<u>B358</u>	B369	B380			•	
B348	B359	B370	B381				
B349	B360	B371	B382				
B350	B361	B372	B383			FC	DR notes see sheet 18 72
B351	B362	B 3 7 3	B384				STATE OF OHIO DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGES AND STRUCTURAL DESIGN
B352	B363	B374	B385				BOX BEAM LAYOUT

Expansion Joint

BRIDGE NO. BEL-40-2338 OVER THE B.¢O. RAILROAD AND WHEELING CREEK TRACED CHECKED REVIEWED DATE REVISED R.L.D. WJJ 12-1-80 DESIGNED DRAWN J.A. M. J. A.M.

Michorita & Pier #6 re Beam Splice Beam Span"C"= 29-9" 00 海鮒 1-0" 14-10% 1-0" 16-10/2" 1 - e Pier #7,MICROFILMED B 386 ______B391_ JAN 221986 B387 Match line D; See sheet B388 -----------B389 <u>B390</u> , [12°-50'] +24°-30' E Bridge -B391 B392 <u>B393</u> B394 <u>B395</u> B396 -----+ E Diaphragm Beam Span"F"= 46'-0" E Pier #9 23'0" B 419 <u>B420</u> Match line E -34°-39' 34°-39'-E Bridge -E Expansion Joint

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	d)		•	•			· •	
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	<u>\</u>			E	DEPAR BUREAU OF	BRIDGES AND STRUC	FURAL DESIGN	
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		•		OVE	R TH	E B, É O. R	All RCA	\vec{D}
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. •				ULSIGNED DRA U.A.M. J.A.	M. TRAC	R.L.D. W	112-1 12-1	
						<u> </u>		







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Beam	Bor Size	NO. Regid	Lieng
CB 21-48 (16 Strands)	#4	7	9'-6'
CB 21-48 (10 Strands)	# 4	Э	6'-0
B 21-36 (12 Strands)	# <i>5</i> "	S	8'-6'
B 21-36 (8 Strands)	#5	1	6'-6

43'-6" - & Bridge 21-9" 20'-3" SE401@ 1-4"42 From the 2011 rehabilitation. an overlay has been placed See attached dead load calculations -SE402@9"% -Profile Grade 3/16" per ft. 3/16 per ft. 0.0 5.5 و. د 12" × 36" Box Beam; Level Erect first -----5-12" x 48" Box Beams = 4 5-12"x48" Box Beams = 1-6" 1-6" * ------20'0" + fit-up 14'-0" 20'-0" + fit - up 14'-0" 21-3" 21-3" 42-6"

TRANSVERSE DECK SECTION THRU ARCH SPANS

20'-3" 1-6 (to g of Bridge) PE502 P.E 503 SE 508@trans. beams or SE512@pier#7 C<u>onstr. Ut.</u> Level SE501 Constr. Ut. SE 502 SE503 Varies

SOUTH SIDE-SECTION

NOTES:

*This is the nominal dimension. The pay quantity of that portion of the deck concrete over the beams shall be based on the average of this dimension and the depth at beam bearings even though deviation from this average may occur because the top of the beam may not have the camber anticipated in the design; i.e., 3/4". The camber of beams shall be measured in the field before the deck is placed. The actual depth at mid-span shall be the nominal dimension plus or minus the difference between actual and anticipated camber. FHWA
REGIONSTATEPROJECT2550HIO74BEL-40-23.38

NOTES:

For slab steel plan see sh	neet 22/72
For parapet steel spacing see	2
sheet 28/72 thru 35/72	
Scupper location on sheet	14 72.
Scupper details on sheet	48 72
Field cut reinforcing steel	at the
scuppers.	

For Beam Plan see sheets 18/72; 19/72

Calculated camber for all 12" box beams = 3/4"

For additional notes see sheet 23/72

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	STATE OF OHIO DEPARTMENT OF TRANSPORTATION BUREAU OF BRIDGES AND STRUCTURAL DESIGN							
•	SUPERSTRUCTURE							
	BRIDGE NO. BEL-40-2338 OVER THE B.¢O. RAILROAD AND WHEELING CREEK							
	DESIGNED J. A. M.	drawn J.A.M.	TRACED	checked R.L.D.	REVIEWED WJJ 12-	date 1-80	REVISED	

STANPAT PRODUCTS INC. PORT BASHINGTON & I

TASK : Load Rating Supporting Calculations PROJECT NO : 195987 SUBJECT : Dead Load Calculations INTERNATIONAL CALCULATED BY : FTB DATE : 6/5/2023 CHECKED BY : JTB DATE : 7/12/2023 DEAD LOADS CALCULATIONS DEAD LOADS CALCULATIONS DESCRIPTION: -Dead load computations include a 5% self weight increase factor No concrete is subtracted for the formliners REFERENCES: PEL 40 2233 Original Plans 1932.pdf -BEL 40 2233 Driginal Plans 1932.pdf -BEL 40 2233 Original Plans 1932.pdf -BEL 40 2230 Original Plans 1932.pdf -BEL 40 2230 Origin Plans	PROJECT : Belmont 40				Michael Baker	
SUBJECT : Dead Load Calculations INTERNATIONAL CALCULATED BY : ETB DATE : 6/5/2023 CHECKED BY : JTB DATE : 7/12/2023 DEAD LOADS CALCULATIONS DESCRIPTION: -Dead load computations include a 5% self weight increase factor -No concrete is subtracted for the formliners REFERENCES: -BEL 40 2283_Original Plans 1932.pdf -DED LOADS COMPUTATIONS Concrete density O.15 DETE TOR TYPE PARAPET Cross sectional area Base width DEMAKK PARAPET Cross sectional area Base width DIEMALK Height MEL 40 238 [n] OLD 000 [n ² Sectional area Base width DIEMALK <td cols<="" td=""><td>TASK : Load Rating Supportin</td><td>g Calculations</td><td>PROJECT NO</td><td>: 195987</td><td></td></td>	<td>TASK : Load Rating Supportin</td> <td>g Calculations</td> <td>PROJECT NO</td> <td>: 195987</td> <td></td>	TASK : Load Rating Supportin	g Calculations	PROJECT NO	: 195987	
CALCULATED BY : ETB DATE : 6/5/2023 CHECKED BY : JTB DATE : 7/12/2023 DEAD LOADS CALCULATIONS OPEAD LOADS CALCULATIONS REFERENCES: BEL 40 2283_Original Plans 1932.pdf BEL 40 2283_Original Plans 1932.pdf DEAD LOADS COMPUTATIONS Concrete density OLS kcf DEFLECTOR TYPE PARAPET Cross sectional area Base width DEMAK PARAPET Cross sectional area Base width Sidewalk parapet pressure load OLS kef SUEWALK Height USE NEL ORD COMPUTATIONS Construct density OLS kcf SUEWALK PARAPET Cross sectional area Sign (n) <td>SUBJECT : Dead Load Calculo</td> <td>ations</td> <td></td> <td></td> <td>INTERNATIONAL</td>	SUBJECT : Dead Load Calculo	ations			INTERNATIONAL	
DEAD LOADS CALCULATIONS DESCRIPTION: -Dead load computations include a 5% self weight increase factor -No concrete is subtracted for the formliners REFERENCES: -BEL 40 2283_Original Plans 1932.pdf -BEL 40 2281-981.000.02.338-2010-00.pdf DEAD LOAD COMPUTATIONS Concrete density DEFLECTOR TYPE PARAPET Cross sectional area Base width DEMALK PARAPET Cross sectional area Base width DEMALK PARAPET Cross sectional area Base width Sold.00 Sold.00 Sold.00 In Cross sectional area Base width Sold.00 Sold.00 Sold.00 Sold.00 In Cross sectional area Base width Sold.00 Sold.00 Sold.00 </td <td>CALCULATED BY : ETB</td> <td>DATE : 6/5/2023</td> <td>CHECKED BY</td> <td>JTB</td> <td>DATE : 7/12/2023</td>	CALCULATED BY : ETB	DATE : 6/5/2023	CHECKED BY	JTB	DATE : 7/12/2023	
DEAD LOADS CALCULATIONS DESCRIPTION: -Dead load computations include a 5% self weight increase factor -No concrete is subtracted for the formiliners REFERENCES: -BEL 40 2283 Original Plans 1932.pdf -DEL 40 2283 Original Plans 1932.pdf DEL 40 2283 Original Plans 1932.pdf -DEL 40 2283 Original Plans 1932.pdf DEL 40 2283 Original Plans 1932.pdf DED 400 COMPUTATIONS Concrete density DIS Kef DEFLECTOR TYPE PARAPET Cross sectional area Sold to 100 ft Sold to 50 fs Sold to 100 ft Sold to 100 ft						
DESCRIPTION: - Dead load computations include a 5% self weight increase factor - No concrete is subtracted for the formliners REFERENCES: -BEL 40 2283_Original Plans 1932.pdf -BEL 40 2283_Original Plans 1932.pdf -BEL 40 2283_Determine Plans 1932.pdf -D1-22813-BEL-00040-23.38-2010-00.pdf DEAD LOAD COMPUTATIONS Concrete density Deflector type parapet Cross sectional area Base width Deflector type parapet pressure load SIDEWALK PARAPET Cross sectional area Base width Diffector type parapet pressure load SIDEWALK PARAPET Cross sectional area Base width Sidewalk parapet pressure load O.55 ksf SIDEWALK Height Width Sidewalk pressure load O.14 ksf Deck Thickness Boor Beam Height Deck + box Beam pressure load Out as the pressure load Net as pressure load Out as the pressure load Deck + box beam pressure load Overlay Thick					DEAD LOADS CALCULATIONS	
DESCRIPTION: -Dead load computations include a 5% self weight increase factor -No concrete is subtracted for the formiliners REFERENCES: -BEL 40 2283_Original Plans 1932.pdf -Dit -22815-BEL-00040-23.38-2010-00.pdf DEAD LOAD COMPUTATIONS Concrete density 0.15 kcf DEFLECTOR TYPE PARAPET Cross sectional area 429.00 in² Base width 1.50 ft Deflector type parapet pressure load 0.31 ksf SIDEWALK PARAPET 504.00 in² Cross sectional area 504.00 in² Base width 1.00 ft Sidewalk parapet pressure load 0.55 ksf SIDEWALK 4.13 ft Height 0.14 ksf Deck Thickness 5.38 in Box Beam Height 0.23 ksf Deck + box beam pressure load 0.23 ksf						
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