Belmont 40 Slab Span, Slab Beam, and Box Beam Constructability Analysis



TRANSVERSE DECK SECTION THRU ARCH SPANS

BrR is self-computing slab beam LLDF's around 0.6 (Shear and Flexure), box beam LLDF's around 0.3. (Flexure) and 0.6 (shear) What should we be using? Initial thoughts was 0.75 for slab beams, reflecting the excavator turning and positioning slab beams. Might be too much (see results below). These ratings are governed by Service III. We need to be avoiding cracking during construction, so Service III and Strength I are the checks I'm doing. Results below are for Strength I (1.50 LL factor) and Service III assuming LLDF's automatically calculated for box beams and a 0.75 LLDF for slab beams.

System of units US customary OSI /	Lane/impact loading type Display form metric • As requested • Detailed Multiple rate	at: ing levels per row					
Bridge id	Structure	Vehicle	Inventory rating factor	Operating rating factor	Legal operating rating factor	Legal rating factor	ł
0701599-Stage 2 Co	BEL-40-23.38-Arch Span A, 16'-3	336+SU4				0.585	[
0701599-Stage 2 Co	BEL-40-23.38-Beam Span B, 45'-6	336+SU4				0.569	
0701599-Stage 2 Co	BEL-40-23.38-Arch Span D, 20'-3	336+SU4			111	0.521	
0701599-Stage 2 Co	BEL-40-23.38-Beam Span E, 37'-9	336+SU4		~	F. 11.	1.018	
0701599-Stage 2 Co	BEL-40-23.38-Beam Span F, 46'-0	336+SU4		. 110	,~ ,~	0.787	
0701599-Stage 2 Co	BEL-40-2338 (Arch Span B, 14'-0"	336+SU4		2^{\vee}		0.665	
0701599-Stage 2 Co	BEL-40-23.38 (Beam Span D, 33'-9	336+SU4	ck			1.327	
0701599-Stage 2 Co	BEL-40-23.38 (Beam Span G, 44'-6	336+SU4	5			0.857	
0701599-Stage 2 Co	BEL-40-23.38 (Beam Span C, 29'-9	336+SU4				1.559	

Structure Rating Results

System of units	Lane/impact loading type	Display format:				
● US customary ○ SI / metric	As requested O Detailed	Multiple rating levels per row	~			

	Bridge id	Structure	Vehicle	Inventory rating factor	Operating rating factor	Legal operating rating factor	Legal rating factor
▶ 0701	599-Stage 2 Co	BEL-40-23.38-Arch Span A, 16'-3	336+SU4				0.970
0701	599-Stage 2 Co	BEL-40-23.38-Beam Span B, 45'-6	336+SU4			•	0.982
0701	599-Stage 2 Co	BEL-40-23.38-Arch Span D, 20'-3	336+SU4			160	0.921
0701	599-Stage 2 Co	BEL-40-23.38-Beam Span E, 37'-9	336+SU4		C	11.	1.341
0701	599-Stage 2 Co	BEL-40-23.38-Beam Span F, 46'-0	336+SU4		~NC		1.055
0701	599-Stage 2 Co	BEL-40-2338 (Arch Span B, 14'-0"	336+SU4		ド		1.075
0701	599-Stage 2 Co	BEL-40-23.38 (Beam Span D, 33'-9	336+SU4				1.536
0701	599-Stage 2 Co	BEL-40-23.38 (Beam Span G, 44'-6	336+SU4	2.			1.090
0701	599-Stage 2 Co	BEL-40-23.38 (Beam Span C, 29'-9	336+SU4				1.679

Box Beam Chart in Plans for Identifying Strand Patterns

BOX BEAM SPAN	BEAM NUMBER	STRAND PATTERN	L	A	В	С	D
	B1	1F	46'-2"	OAND R BARS SPA. @ 4" MAX.	OAND R BARS SPA. @ 12" MAX.	Y BARS SPA @ 12" MAX	••••
	B2-B5	1	46'-2"				
В	<i>B6</i>	5	46'-2"	GAND E BARS SPA. @ 4" MAX	GAND E BARS SPA. @ 12" MAX	YY BARS SPA. @ 12" MAX.	7.00%
	<i>B7-B10</i>	1	46'-2"	O AND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
	<u>B11</u>	<u>1F</u>	46'-2"		<u></u>	Ç	
	B386	3F	23'-6"			ζ Ι	
	B387	3	24'-2"		OAND B BARS SPA @ 12" MAX	V RARE SRA @ 12" MAAY	
	B388	3	25-117/8	CANDR BARS SPA. @ 5 MAA.	O AND R BARS SPA. @ 12 MAA.	T BARS SPA. @ 12 WAA	
	B389	3	27-93/4			5	
C	B390 B201	<u> </u>	29-1 3/8	GANDEBARS SPA @ 3" MAX	GANDEBARS SPA @ 12" MAX	VY BARS SPA @ 12" MAX	Z 00%
Ŭ	B302	1	32' 07/8"	CANDE DANG OF A. @ S MAA	CAND E DANG OF A. W 12 MAA		7.00%
	B303	1	31'-7 3/1"			ج ا	
	B394	1	36'-5 5/8"	OAND R BARS SPA @ 3" MAX	OAND R BARS SPA @ 12" MAX	Y BARS SPA @ 12" MAX	
	B395	1	38'-3 1/2"				
	B396	1F	39'-0"			3	
	B397	3F	29'-3"			٤ - ٢	
	B398	3	29'-6 1/4"		O AND R BARS SPA. @ 12" MAX.	ξ [
	B399	3	30'-5 5/8"	OAND R BARS SPA. @ 3" MAX.		Y BARS SPA. @ 12" MAX	
	B400	3	31'-4 7/8"			- }	
	B401	3	32'-4 1/8"			۲ ک	
D	B402	6	33'-2"	GAND E BARS SPA. @ 3" MAX	GAND E BARS SPA. @ 12" MAX	YY BARS SPA. @ 12" MAX.	7.00%
	B403	2	34'-0"			Ç	
	B404	2	34'-11 1/4"		O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
	B405	2	35'-10 1/2"	OAND R BARS SPA. @ 3" MAX.			
	B406	2	36'-9 7/8"				
	B407	2F	37'-3"			Y	
	B408	4F	37'-3"	OAND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
	B409-B412	4	37'-13/4"				
E	B413	7	37'-13/4"	GAND E BARS SPA. @ 4" MAX	GAND E BARS SPA. @ 12" MAX	YY BARS SPA. @ 12" MAX.	7.00%
	B414-B417	4	37'-13/4"	O AND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
	B418	4F	37'-3"				
	B419	<u>2F</u>	45'-6"	O AND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
	B420-B423	2	45'-4 3/4"	CANDERARS SPA @ 4" MAX	CANDERARS SPA @ 12" MAX	VV PARS SPA @ 12" MAA	7 000/
	B424	5	45-4 3/4"	GANDE BARS SPA. @ 4 MAX	GANDEBARS SPA. @ 12 MAA	TT BARS SPA. @ 12 MAA.	7.00%
	B423-B428	2	45-43/4"	O AND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
	B429	<u>2F</u>	43-0				
	B431_B424	2	44-2	O AND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	
G	BA35	6	11/2"	GANDEBARS SPA @ 4" MAX	GANDEBARSSPA @ 12" MAX	YY BARS SPA @ 12" MAX	7.00%
	B436-B439	2	44-11/8"				1.0070
	B440	 2F	44'-2"	O AND R BARS SPA. @ 4" MAX.	O AND R BARS SPA. @ 12" MAX.	Y BARS SPA. @ 12" MAX	

For reference only, live load definitions from arch rib and spandrel column constructability analysis. Loads will be the same for the BrR models

-Assume excavator weight is 81k + slab beam pick (20'x4'x1' = 12k) and has an axle spacing of 13.25'. Total axle load is 46.5k. SU4 truck is 54k (including slab beam loads) and follows the traditional AASHTO axle weights and spacings. Keep a minimum distance of 10' between excavator and SU4 truck



Assume Excavator is confined to the center of the crane mats, for design purposes assume it moves within a 13' width at the center of the crane mat. What is worst case LLDF? Still consider 75% of the load on a single tread. Tie rods installed before steel beams and crane mat placed. No different than a noncomposite slab beam/box beam bridge from a LLDF perspective.

Dimensions				
All dimensions are approximate and may vary depending on bucket se	election.			
				5
				Ł
	2			
Boom Option		Reac 6.5 m	h Boom (21'4")	
Stick Options		Reac	h Stick	
• • • •	R3.9DB (1	12'10")	R3.2DB (10'6")
1 Machine Height:				
Cab Height	3170 mm	10.4 ft	3170 mm	10.4 ft
FOGS Height	3310 mm	10.9 ft	3310 mm	10.9 ft
Handrails Height	3160 mm	10.4 ft	3160 mm	10.4 ft
With Boom/Stick/Bucket Installed	3670 mm	12.0 ft	3490 mm	11.5 ft
With Boom/Stick Installed	3560 mm	11.7 ft	3330 mm	10.9 ft
With Boom Installed	2880 mm	9.4 ft	2880 mm	9.4 ft
With Boom/Stick/Bucket Installed (with auxiliary lines)	3720 mm	12.2 ft	3530 mm	11.6 ft
With Boom/Stick Installed (with auxiliary lines)	3620 mm	11.9 ft	3400 mm	11.2 ft
With Boom Installed (with auxiliary lines)	2970 mm	9.7 ft	2970 mm	9.7 ft
2 Machine Length:				
With Boom/Stick/Bucket Installed	11 200 mm	36.7 ft	11 180 mm	36.7 ft
With Boom/Stick Installed	11 180 mm	36.7 ft	11 140 mm	36.5 ft
With Boom Installed	9960 mm	32.7 ft	9960 mm	32.7 ft
With Boom/Stick/Bucket Installed (with auxiliary lines)	11 200 mm	36.7 ft	11 180 mm	36.7 ft
With Boom/Stick Installed (with auxiliary lines)	11 180 mm	36.7 ft	11 140 mm	36.5 ft
With Boom Installed (with auxiliary lines)	10 010 mm	32.8 ft	10 010 mm	32.8 ft
3 Upperframe Width without Walkways	3030 mm	9.9 ft	3030 mm	9.9 ft
4 Tail Swing Radius	3530 mm	11.6 ft	3530 mm	11.6 ft
5 Counterweight Clearance	1250 mm	4.1 ft	1250 mm	4.1 ft
6 Ground Clearance	510 mm	1.7 ft	510 mm	1.7 ft
Length to Center of Rollers Track Locate	4040 mm	13.3 ft	4040 mm	13.3 ft
8 Track Length	5030 mm	16.511	5030 mm	16.5 ft
9 Track Gauge - Extended	2590 mm	8.5 ft	2590 mm	8.5 ft
 Track Width/Undercarriage Width (with steps/without steps): C00 mm (24%) Shoes 	2100	10.5.0	2100 mm	10.5.0
200 mm (20°) Shoes	3190 mm	10.5 It	2200 mm	10.5 11
700 mm (20) Shoes	3290 mm	10.8 II	3290 mm	10.8 II
oor min (51-) Shoes	5390 mm	11.1.1	2440 mm	11.1 11
250 mm (22*) Shoes	2440			
850 mm (33") Shoes	3440 mm	11.3 ft	JANO IIIII	mon
850 mm (33") Shoes Bucket Type Bucket Canacity	3440 mm HD	2.46 w ¹²	1.88 m ³	246 m

BrR Settings for Running 336 Excavator + SU4 Truck



Assumptions:

-Crane Mat is Timber, u = 0.35, MOE = 1.5x10^3 ksi

-Take 0.75k and divide it along length of span to represent position and load of more heavily loaded tread. Use 0.25k for the other tread. Results in klf tread unit load.

-No tying of the beams together is modeled, even though the beams will have tie rods installed before the excavator drives over them. -Compression only spring support defined in vertical direction to avoid potential bearing uplift. Generic stiffness assigned

Conclusion: The 0.61 LLDFs in the BrR model, used for the 48" wide slab beams, are conservative.





Case 2: Excavator Tread Centered on Beam. Max LLDF's Shown





The maximum shift within the 13' allowable width is analyzed considering 75% of the excavator + slab beam pick on the left tread centered over the 48" wide slab beam left adjacent to the center 36" wide slab beam. A case where the right tread has 75% of the load seems less likely since the excavator would likely not be reaching out as far from this position with the excavator shifted to the left.

Dead loads during this construction stage consist of the self weight of the beams, HP sections, and crane mats. From previous constructability checks, the crane mats plus HP sections can be assumed to weigh 70psf. This is applied as a DW load in the model under Member Loads.

The load combination for this construction check, at the Strength Limit State, is:

1.25(Permanent Dead Load) + 1.50(Construction Dead Load) + 1.50(Construction Live Load)

An impact factor of 10% is applied to the live load.

From these two load placements, the worst case LLDF is 0.54, leave the previously calculated 0.61 LLDF in BrR to be conservative. To compare with AASHTO calculated LLDF, slab beam Moment and Shear distribution factors for a single lane loaded are all 0.600. If we divide out the multiple presence factor of 1.20, the AASHTO LLDF for these slab beam spans would be 0.500. So using the maximum LLDF of 0.61 from the Midas analysis is not undercutting the AASHTO LLDF.

Looking at the results from running the above described LLDF's, all slab beam spans are failing and three of the box beam spans are also failing. Since the box beam spans are using LLDF's for single lane including multiple presence factors, divide their BrR computed LLDF's by 1.20 and see if they pass. Cannot divide slab beam LLDF's by 1.2 since those are generated by Midas instead of BrR.

It is worth noting that that the AASHTO LLDF equations are not applicable for the slab beams due to beam span length. BrR is defaulting to lever rule because of this, resulting in the 0.5 axle going to a beam. We can use the prevoously estimated 0.61 LLDF (vs 0.75) since 0.61 is still greater than the max distribution going into the slab beams as a result of load distribution through the timber crane mat and steel beams. Thus, the 0.61 is still the result of using the lever rule for the slab beam.

Structure Rating Results

	System of units US customary OSI /	metric Lane/impact loading type Display format: Multiple rating level	s per row				
	Bridge id	Structure	Vehicle	Inventory rating factor	Operating rating factor	Legal operating rating factor	Legal rating factor
	0701599-Stage 2 Co	BEL-40-23.38-Arch Span A, 16'-3	336+SU4				0.731
I	0701599-Stage 2 Co	BEL-40-23.38-Beam Span B, 45'-6	336+SU4				0.569
Ι	0701599-Stage 2 Co	BEL-40-23.38-Arch Span D, 20'-3	336+SU4				0.651
	0701599-Stage 2 Co	BEL-40-23.38-Beam Span E, 37'-9	336+SU4				1.018
	0701599-Stage 2 Co	BEL-40-23.38-Beam Span F, 46'-0	336+SU4				0.787
	0701599-Stage 2 Co	BEL-40-2338 (Arch Span B, 14'-0"	336+SU4				0.831
	0701599-Stage 2 Co	BEL-40-23.38 (Beam Span D, 33'-9	336+SU4				1.327
	0701599-Stage 2 Co	BEL-40-23.38 (Beam Span G, 44'-6	336+SU4				0.857
	0701599-Stage 2 Co	BEL-40-23.38 (Beam Span C, 29'-9	336+SU4				1.559

slab beam ratings

box beam ratings

After dividing out multiple presence factors from the single lane LLDF's, it was also decided to set the multi lane LLDF's to a low number so that those do not govern the load rating. Additionally, looking at the excavator track gauge width, the middle 36" wide beam can never see the 75% loading that the outer 48" wide beams will see. Therefore, the middle beam's distribution factors were set to 0.5 for Moment and Shear, which reflects the AASHTO-calculated LLDF divided by 1.20 to remove the multiple presence factor.

In discussions with Chris Cummings and Jeff Broadwater, options included using a smaller excavator or adding strands to the beams. From a couple quick iterations, smaller excavators improved ratings but not enough to get all beams to pass. Therefore it was decided to add strands to the beams to get them to pass. Goal is to add as few strands as possible so that changes to the beam designs (and camber, etc.) are minimal. Strands added as shown below; screen shots taken from plans and new strands shown in magenta.

The only box beams that required adjusted strand patterns were in Box Beam Span B, Patterns 1 and 5. All three slab spans required additional strands.

Box Beam Span B Strand Adjustments



Arch Span A 16'-3" & Arch Span B 14'-0" Strand Adjustments









Arch Span D 20'-3" Strand Adjustments







Box Beam and Slab Span Results after Additional Strands Added

Structure Rating Results

(System of units US customary OSI /	metric As requested Detailed Mu	lay format:					
	Bridge id	Structure	Vehicle	Inventory rating factor	Operating rating factor	Legal operating rating factor	Legal rating factor	F
Þ	0701599-Stage 2 Co	BEL-40-23.38-Arch Span A, 16'-3	336+SU4				1.171	
	0701599-Stage 2 Co	BEL-40-23.38-Beam Span B, 45'-6	336+SU4				1.172	
	0701599-Stage 2 Co	BEL-40-23.38-Arch Span D, 20'-3	336+SU4				1.092	
	0701599-Stage 2 Co	BEL-40-23.38-Beam Span E, 37'-9	336+SU4				1.410	
	0701599-Stage 2 Co	BEL-40-23.38-Beam Span F, 46'-0	336+SU4				1.159	
	0701599-Stage 2 Co	BEL-40-2338 (Arch Span B, 14'-0"	336+SU4				1.323	
	0701599-Stage 2 Co	BEL-40-23.38 (Beam Span D, 33'-9	336+SU4				1.710	Γ
	0701599-Stage 2 Co	BEL-40-23.38 (Beam Span G, 44'-6	336+SU4				1.205	
	0701599-Stage 2 Co	BEL-40-23.38 (Beam Span C, 29'-9	336+SU4				2.014	



Below is the BrR QC Checkprint. Everything needs checked; material properties can be found in the plans Bluebeam pdf. Remember that Impact is 10% and has been overriden in each member definition.



































